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# The Aerodynamic and Dynamic Loading of a Slender Structure by an Impacting Tornado-Like Vortex: The Influence of Relative Vortex-to-Structure Size on Structural Loading

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The Aerodynamic and Dynamic Loading of a Slender Structure by an Impacting Tornado-  
Like Vortex: The Influence of Relative Vortex-to-Structure Size on Structural Loading

A dissertation submitted in partial fulfillment  
of the requirements for the degree of  
Doctor of Philosophy in Civil Engineering

by

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## ABSTRACT

Structural loading produced by an impacting vortex is a hazardous phenomenon that is encountered in numerous applications ranging from the destruction of residences by tornados to the chopping of tip vortices by rotors. Adequate design of structures to resist vortex-induced structural loading necessitates study of the phenomenon that control the structural loading produced by an impacting vortex. This body of work extends the current knowledge base of vortex-structure interaction by evaluating the influence of the relative vortex-to-structure size on the structural loading that the vortex produces. A computer model is utilized to directly simulate the two-dimensional impact of an impinging vortex with a slender, cylindrical structure. The vortex's tangential velocity profile (TVP) is defined by a normalization of the Vatisstas analytical (TVP) which realistically replicates the documented spectrum of measured vortex TVPs. The impinging vortex's maximum tangential velocity is fixed, and the vortex's critical radius is incremented from one to one-hundred times the structure's diameter. When the impinging vortex is small, it interacts with vortices produced on the structure by the free stream, and maximum force coefficient amplitudes vary by more than 400% when the impinging vortex impacts the structure at different times. Maximum drag and lift force coefficient amplitudes reach asymptotic values as the impinging vortex's size increases that are respectively 94.77% and 10.66% less than maximum force coefficients produced by an equivalent maximum velocity free stream. The vortex produces maximum structural loading when its path is shifted above the structure's centerline, and maximum drag and lift force coefficients are respectively up to 4.80% and 34.07% greater than maximum force coefficients produced by an equivalent-velocity free stream. Finally, the dynamic load factor (DLF) concept is used to develop a generalized methodology to assess the dynamic amplification of a structure's response to vortex loading and to assess the

dynamic loading threat that tornados pose. Typical civil and residential structures will not experience significant response amplification, but responses of very flexible structures may be amplified by up to 2.88 times.

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## NOMENCLATURE

### English

A	Projected cylinder area per unit length, $D \cdot L$ ( $m^2$ )
$A_F$	Forcing function amplitude (N)
$A_s$	Structure response amplitude (m)
C	Damping constant (kg/s)
$C_d$	Drag force coefficient, $2 \cdot F_d / A \cdot \rho \cdot U_\infty^2$ (dim)
$C_d'(+)$	Maximum positive drag coefficient amplitude (dim)
$C_d'(-)$	Maximum negative drag coefficient amplitude (dim)
$C_F$	Any force coefficient (dim)
$C_l$	Lift force coefficient, $2 \cdot F_l / A \cdot \rho \cdot U_\infty^2$ (dim)
$C_l'(+)$	Maximum positive lift coefficient amplitude (dim)
$C_l'(-)$	Maximum negative lift coefficient amplitude (dim)
$C_N$	Normal force coefficient, analogous to $C_l$ for free stream (dim)
$C_S$	Stream-wise force coefficient, analogous to $C_d$ for free stream (dim)
$C_S'$	Amplitude of stream-wise force coefficient (dim)
c	Airfoil chord length (m)
D	Diameter of the cylinder (m)
$F_d$	Drag force (N)
$F_l$	Lift force (N)
$F_o$	Maximum forcing amplitude (N or dim)
$f_{Cl}$	Frequency of vortex shedding (Hz)
$f_n$	Natural frequency of structure, $\omega_n / 2 \cdot \pi$ (Hz)

$f_v$	Vortex loading frequency (Hz)
$K$	Structural stiffness (N/m)
$k_{1-4}$	Kutta coefficients for Runge-Kutta solution (dim)
$L$	Reference structure length (m)
$M$	Structure mass (kg)
$N$	Exponent for Vatis' vortex model (dim)
$P_\infty$	Ambient/Reference Pressure (N/m <sup>2</sup> )
$P^*$	Dimensionless pressure, $P/\rho \cdot U_\infty^2$ (dim)
$Q$	Axial volumetric flow rate through vortex (m <sup>3</sup> /s)
$q_1$	State variables for position in the response model (m)
$q_2$	State variables for velocity in the response model (m/s)
$R$	Radial coordinate of the vortex (m)
$Re$	Reynolds number, $D \cdot U_\infty/\nu$ (dim)
$Re_c$	Chord Reynolds number, $c \cdot U_\infty/\nu$ (dim)
$Re_v$	Vortex Reynolds number, $\Gamma_\infty/\nu$ (dim)
$R_m$	Dimensionless mass term for response model, $\rho \cdot D \cdot A/M$ (dim)
$r_c$	Critical radius for the vortex (m)
$r_p'$	Radial distance between the vortex center and the boundary node (dim)
$S$	Vortex swirl ratio, $0.5 \cdot \Gamma_\infty \cdot r_c/Q$ (dim)
$St$	Strouhal number, dimensionless vortex shedding frequency, $f_{Cl} \cdot D/U_\infty$ (dim)
$T_d$	Dynamic load application period (s)
$T_{lag}$	Time required for the vortex and cylinder centers to align, $X_o/U_\infty$ (dim)
$T_n$	Fundamental structure period (s)

$T_v$	Loading period of impacting vortex (s)
$T_v^*$	Dimensionless vortex shedding period (dim)
$T^*$	Period of vortex shedding for $Re = 150$ , $1/St$ (dim)
$t$	Time (s)
$t^*$	Dimensionless time ( $t \cdot U_\infty / D$ )
$U$	Horizontal velocity (m/s)
$U_\infty$	Bulk velocity of the fluid stream (m/s)
$U_\infty^*$	Dimensionless bulk velocity for response model, $U_\infty / \omega_n \cdot D$ (dim)
$U^*$	Dimensionless horizontal velocity, $U / U_\infty$ (dim)
$V$	Vertical velocity (m/s)
$V_\theta$	Tangential velocity of the vortex (m/s)
$V^*$	Dimensionless vertical velocity, $V / U_\infty$ (dim)
$V_R$	Dimensionless Magnitude of velocity incident on the structure (dim)
$X$	Horizontal coordinate (m)
$X_o$	Starting location of the vortex (dim)
$X^*$	Dimensionless horizontal coordinate, $X/D$ (dim)
$X'$	Horizontal coordinate of translating reference frame attached to vortex (dim)
$x$	Structure position or displacement (m)
$\dot{x}$	Structure velocity (m/s)
$\ddot{x}$	Structure acceleration ( $m/s^2$ )
$x^*$	Dimensionless structure position or displacement, $x/D$ (dim)
$\dot{x}^*$	Dimensionless structure velocity, $U_\infty \cdot \dot{x}^*$ (dim)
$\ddot{x}^*$	Dimensionless structure acceleration, $U_\infty^2 / D \cdot \ddot{x}^*$ (dim)

$x_{o,st}$	Maximum displacement from static application of forcing, $F_o/K$ (m or dim)
$x_p$	Horizontal coordinate of boundary node with respect to cylinder center (dim)
$x_p'$	Horizontal coordinate of boundary node with respect to vortex center (dim)
$Y$	Vertical coordinate (m)
$Y^*$	Dimensionless vertical coordinate, $Y/D$ (dim)
$Y'$	Vertical ordinate of translating reference frame attached to vortex (dim)
$y_p$	Vertical coordinate of boundary node with respect to cylinder center (dim)
$y_p'$	Vertical coordinate of boundary node with respect to vortex center (dim)

### **Greek**

$\alpha$	Dimensionless angular velocity of the vortex (dim)
$\alpha_o$	Oseen vortex constant, 1.25643 (dim)
$\beta$	Sullivan vortex constant, 6.238 (dim)
$\delta t^*$	Solution time step (dim)
$\zeta$	Damping ratio (dim)
$\Delta r$	Radial node spacing (dim)
$\rho$	Density of the fluid ( $\text{kg}/\text{m}^3$ )
$\nu$	Kinematic viscosity of the fluid ( $\text{m}^2/\text{s}$ )
$\kappa, \eta, \lambda$	Coefficients for W-W vortex model (dim)
$\theta$	Incidence angle of stream's velocity (deg)
$\Gamma_\infty$	Infinite or total circulation, $2 \cdot \pi \cdot r_c \cdot V_{\theta,max}$ ( $\text{m}^2/\text{s}$ )
$\omega_d$	Damped natural frequency (rad/s)
$\omega_f$	Forcing frequency (rad/s)
$\omega_n$	Natural frequency (rad/s)

## **Acronyms**

BVI	Body Vortex Interaction
CW	Clockwise
CCW	Counter Clockwise
EF	Enhanced Fujita Scale
L-O	Lamb-Oseen
NSD	Mean Normalized Standard Deviation
ODE	Ordinary Differential Equation
RCVM	Rankine Combined Vortex Model
RK4	Fourth Order Runge-Kutta
SDOF	Single Degree of Freedom
S-K	Scully-Kaufmann
SS	Saffir-Simpson Scale
TOI	Time of Impact
TVP	Tangential Velocity Profile
UP	Under Prediction
VSC	Vortex Shedding Cycle
W-W	Wood and White

## CHAPTER 1: INTRODUCTION AND OBJECTIVES

### 1.1 Introduction

On May 22, 2011, the devastating tornado depicted in Figure 1.1 struck the city of Joplin, MO. Despite the adequate advanced warning provided by the National Weather Service (NWS), 158 people lost their lives and an additional 1,000 residents sustained injuries (NWS, 2011). It is unlikely that loss of life due to severe tornados can be eliminated. However, the numbers of fatalities and injuries can certainly be reduced by adequately designing structures to resist tornado wind loads.



**Figure 1.1:** (a) The May 2011 Joplin, MO tornado and (b) damage track through a subdivision in Joplin, MO (Magill, 2012).

Structural loading by impacting vortices is a hazardous phenomenon encountered in numerous applications of engineering significance. Severe atmospheric vortices, specifically tornados and hurricanes, cause 122 deaths (NWS, 2014) and \$5.5 billion in financial losses (NWS, 2012) in the United States and its territories each year. Rotor-tip vortices produced by rotorcraft propellers (See Figure 1.2) are chopped by propeller blades, producing impulsive noise, aircraft vibration, and accelerated fatigue of the propeller blades (Bagai and Leishman, (1993), Ramasamy and Leishman, (2006), and Vatistas, (2006)). Wing-tip vortices produced by fixed-wing aircraft (See Figure 1.3) form hazardous, turbulent wakes that limit the capacity of

large airports (Bhagwat and Leishman, 2002). Vortices shed within clusters of tall structures (See Figure 1.4) impact downstream structures producing uncomfortable noise and vibration in the downstream structures (Rockwell, 1998). These few examples demonstrate that structural loading by vortices is a hazardous phenomenon that is relevant to much of society.



(a)



(b)

**Figure 1.2:** Rotor tip vortices produced by (a) de Havilland Canada DHC-5 Buffalo turbo-prop plane and (b) Cobra attack helicopter (Diaz, 2010).



(a)



(b)

**Figure 1.3:** Wing tip vortices behind (a) CF 18 Hornet (Chandler, 2005) and (b) Boeing 727 (uiowa, 1999).



(a)

(b)

**Figure 1.4:** Vortex shedding from mountains on islands near Chile (NASA, 2013) and (b) cluster of tall buildings in Dubai (Cheong, 2015).

It is imperative that the amplitude and character of structural loading produced by vortices be understood so that adequate design methodology may be adopted to resist the loading.

The aforementioned scenarios of structural loading by vortices collectively illustrate that the relative size of the impinging vortex to the impacted structure vary greatly depending upon the impact scenario. Tornado and hurricane vortices may have critical radii greater than 2 km (NOAA, 2013b) and 45 km (NOAA, 1980) respectively, meaning that they are many times larger than the structures that they load. Wing tip vortices and vortices produced within clusters of tall structures are similar-sized to the structures that they impact. Finally, critical radii of rotor tip vortices are several times smaller than the chord length of the rotors that chop them. This summary highlights the fact that structure loading by vortices occurs at practically all relative vortex-to-structure size scales. The question that immediately arises is “How does the relative vortex-to-structure size influence the character and amplitude of the resulting structure loading?”

Three methodologies are used to study structure loading by vortices: post-storm damage investigation, physical experiment, and computer simulation.

Post-storm damage investigation is premised on the fact that vortex induced loading is no different than loading produced by equivalent-velocity straight-line wind. Pre-determined

structural damage levels are used to estimate maximum wind speeds occurring in tornados via the Enhanced Fujita or “EF” scale (McDonald et al., 2009) and in hurricanes via the Saffir-Simpson or “SS” scale (Simpson and Saffir, 1974). Post-storm damage investigation affords an effective method to qualitatively document severe storm occurrence but is subject to bias (Such as: similar damage levels appearing differently, failure to document the most severe local damage, etc) and is based on the fallacious premise that vortex- and straight-line wind loading are equivalent. Post-storm damage investigation provides no direct information regarding the forces produced by atmospheric vortices, hence alternative methodology is needed to assess the loading that an impacting vortex produces on a structure.

Physical experiments have replicated impact of convecting vortices with air foils (Thom and Duraisamy, 2010) as well as the impact of sustained, tornado-like vortices with prismatic structures (Haan et al., 2008, 2010, and 2014). However physical experiments are costly and difficult to control. Convecting vortices are distorted by turbulence and high wind speeds within wind tunnels (Horner et al., 1996) in addition to being attenuated by interaction with side walls (Seath et al., 1989). Haan et al.’s (2008, 2010, and 2014) sustained vortices have unrealistic properties, raising question as to whether or not realistic tornado-like vortices can be reproduced in laboratory settings. In short, it is difficult to physically simulate impact of structures by vortices and therefore physical experimentation is not currently a viable approach to develop correlations between vortex parameters and the loading that vortices produce.

The inherent uncertainty, physical complexity, and high cost of alternative assessment methodologies encourages that vortex-structure interaction be studied via computer simulation. Upon developing an accurate computer model, practically all vortex parameters are accessible variables, and meaningful correlations can be developed between vortex parameters and the

resulting structural loading. The primary drawback to investigation via computer simulation is that model validation is difficult due to the lack of data from physical experiments, so combinations of common benchmark problems are required for validation.

Several studies utilize computer simulation to study the influence of relative vortex-to-structure size on structure loading amplitude as summarized in Table 1.1. The collective conclusion of the three studies is that structure loading amplitude progressively increases as the relative vortex-to-structure size increases. Due to the variation in simulation and body type, there is apprehension to generally accept this conclusion. Although a relatively wide range of relative vortex-to-structure size ratios is covered, three different body geometries are utilized, and two different vortex wind field models are used. Furthermore, the grid refinement utilized in most of Alrasheedi's (2012) work is far too coarse to adequately resolve flow around the structure. Consequently, the only reliable force coefficients available in the literature are likely for impinging vortices having radii of  $0.15 \cdot D$  to  $4.0 \cdot D$ , which is far below the range possible for tornado and hurricane vortices.

**Table 1.1:** Summary of studies on the influence of relative vortex-to-structure size on structure loading amplitude.

Property	Study		
	Ilie (2009)	Alrasheedi (2012)	Gorecki and Selvam (2013)
Simulation Type	2D	3D	2D
Impacted Body	Airfoil	Prism	Circular Cylinder
Vortex Critical Radius	$0.15 \cdot D$ to $0.50 \cdot D$	$0.375 \cdot D$ to $30.0 \cdot D$	$1.0 \cdot D$ to $4.0 \cdot D$

The impact of a vortex with a structure is a transient loading event. Substantial research effort has been spent evaluating the capability of tornado vortices to produce dynamically-amplified structure response. Mehta and McDonald (1986) and Womble et al. (2009) believe that tornados are too large to dynamically load structures. However, a collection of studies apply assumed tornado forcing time histories and collectively conclude that dynamically amplified

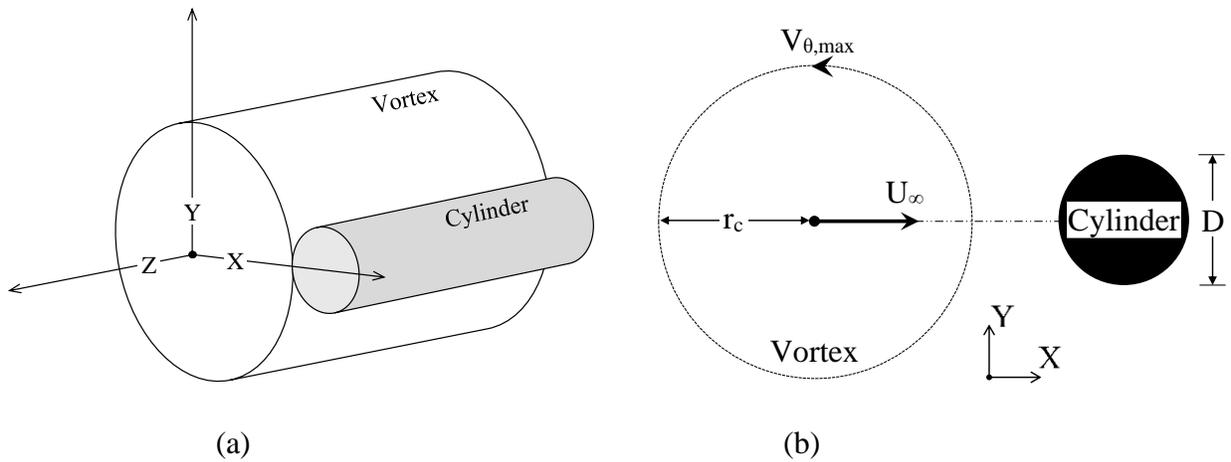
structure response to tornado wind loads is in fact a real threat (Wen (1975), Tan (1975), Seniwongse (1977), Dutta et al. (2002)). The collective shortcoming of these studies is that they fail to develop an applicable and generalized methodology to assess the dynamic loading threat that tornados pose to real world structures but rather assess the dynamic amplification of specific structures' responses to specific forcing histories. Furthermore, their forcing time histories are computed from empirical equations that correlate straight-line wind speed to forces. Consequently, aerodynamic effects such as the shape of the structure and the rotational nature of the vortex wind field are not accounted for.

Adequate design of aerospace vehicles and residential and civil structures necessitates accurate computation of vortex-induced loading. Therefore, the overarching goal of this thesis is to establish the influence of relative vortex-to-structure size on both the structure loading amplitude and the possible dynamic amplification thereof.

A thorough literature review is conducted on the factors that influence the amplitude of vortex loading by structures along with the dynamic amplification thereof. Subsequently, an exhaustive review of vortex tangential velocity profiles (TVPs) is conducted. The present study utilizes computer modelling to simulate vortex impact with structures. Physically-realistic computer simulation necessitates the use of a realistic vortex model. Therefore, analytical and measured vortex TVPs are compiled and compared. Analytical profiles that best represent the measured TVPs are identified and integrated into the computer model for use in subsequent computer simulation.

The physical system modeled in the present work is the parallel impact of a vortex with a rigid circular cylinder, which is illustrated by the schematic Figure 1.5a. The term "parallel" defines the orientation of the impacting vortex's axis of rotation to the major axis of the cylinder

(The  $Z$  – axis). Filipone and Afgan (2008) extensively survey and illustrate impact orientations, and parallel impact is most commonly encountered (Weland and Vlachos, 2009), thereby prompting its selection. Generally stated, the vortex having maximum tangential velocity  $V_{\theta, \max}$  at critical radius  $r_c$  travels within a free stream having velocity  $U_{\infty}$  to impact the cylinder having diameter  $D$ . The vortex and cylinder are both long in the  $z$ -direction, meaning the effects of flow around the cylinder ends is negligible and the interaction is two-dimensional as illustrated in Figure 1.5b.



**Figure 1.5:** (a) 3D and (b) 2D schematics of parallel interaction of a vortex with a cylinder.

The selected, physically-realistic vortex models are integrated in an adaption of Selvam’s (1997b) incompressible finite element model, and parallel vortex impact with a circular cylinder is directly simulated. The computer model is validated using the common benchmark problem of free stream flow over an immersed cylinder and then by evaluating the accuracy with which the impinging vortex is transported across the computational domain. Necessary grid refinement for accurate vortex transport increases rapidly as the impinging vortex size decreases, therefore, the minimum impinging vortex critical radius is  $1 \cdot D$ . The vortex critical radius is progressively increased at fixed  $V_{\theta, \max}$  until the structure loading amplitude becomes asymptotic, and the

corresponding trend in structure loading amplitude is documented. Unique phenomenon controlling the trend in structure loading amplitude with respect to impinging vortex critical radius are illustrated and explained.

Yang et al. (2009 and 2011) conduct physical experiments and report that shifting the path travelled by the vortex center in the Y-direction (See Figure 1.5b) increases structure loading amplitude up to a maximum path shift of one critical radii. However, Ilie (2009) conducts a similar study via computer simulation and reports that structure loading amplitude decreases for a Y-direction vortex path shift. Yang et al. (2009 and 2011) use a counter-clockwise (CCW) vortex and positive Y-direction path shift, while Ilie (2009) uses a clockwise (CW) vortex and negative Y-direction path shift, meaning they effectively conduct the same study and attain different results. This study utilizes the impinging vortex size which produces asymptotic cylinder loading amplitude to define the influence of Y-direction vortex path shift on structure loading amplitude. The vortex path is shifted so that it travels both above and below the structure, and maximum loading amplitude is extracted and used to define the structure loading amplitude trend with respect to path shift. The maximum structure loading amplitude that a vortex can produce is assessed, and the question as to whether or not a rotational flow field can produce greater loading than a free stream of equivalent velocity is answered.

The final section of this thesis evaluates the capability of impacting vortices to produce dynamically-amplified structure response. Previous studies define tornado forcing using empirical equations developed for straight-line wind. The forcing is then applied to a response model, and the dynamic amplification of the assumed structure's response to the assumed tornado forcing is reported. The present study simulates vortex impact with a cylindrical structure, and utilizes force coefficient time history from the computer simulation as forcing. The

dynamic load factor concept is then utilized develop and generalize a procedure to assess possible dynamic amplification of a structure's response. Documented tornado vortex parameter ranges are then used to develop a generalized definition of the possible dynamic response of a structure as function of its fundamental period. Documented fundamental periods of real-world structures are compiled, and conclusions are drawn regarding the dynamic loading threat that tornados pose to real-world structures.

## **1.2 Thesis Objectives**

Following the introductory chapter of this thesis, a comprehensive literature review on vortex loading of structures is provided. The literature review focuses on the current body of knowledge of structure loading by vortices and is grouped according to study methodology: physical experiment, computer simulation, or post-storm damage investigation. Additional discussion of atmospheric vortex occurrence and damage statistics as well as theories about the physics controlling the structure of vortices' tangential velocity profiles are discussed as well. The subsequent chapters address the primary thesis objectives outlined below.

### 1.2.1 Objective 1

❖ *To select physically-realistic analytical vortex tangential velocity profiles (TVPs) for use in computer simulation of structural loading by vortices.*

Physically-realistic computer simulation of structural loading by vortices necessitates the use of physically-realistic vortex TVPs. Analytical vortex TVPs that adequately replicate at least one measured vortex TVPs are documented in the literature. However, no single analytical TVP adequately represents the entire spectrum of measured TVPs. The present study will assemble, compile, and normalize common analytical TVPs found in the literature. Measured vortex TVPs will also be assembled from all experimental and atmospheric vortices documented in the

literature. Analytical vortex TVPs which best represent the typical measured TVPs and bound the range of measured TVPs will be identified and used in computer simulation.

### 1.2.2 Objective 2

❖ *To identify the phenomenon that cause variation in structural loading when an impinging vortex impacts a structure, which is shedding vortices, at different times and to quantify the possible variation in the maximum force coefficient amplitude.*

During preliminary stages of this study, it was observed that computer-simulated vortex impact produced different structural loading when an impinging vortex impacted the structure at different times with respect to the beginning of the simulation. This finding is counterintuitive, as one would assume that an impacting vortex having fixed parameters would produce the same structural loading. If structural loading amplitude produced by an impacting vortex is time dependent, using maximum force coefficient amplitudes from a single simulation could lead to dramatic under-prediction of the maximum forces that the impacting vortex can produce. The present study will systematically simulate direct vortex impact by a single-sized vortex at different times to assess the variation in maximum force coefficient amplitude with vortex impact time. Contour plots of the velocity and vorticity fields will be utilized to identify the phenomenon that produce the variation in maximum structure loading amplitude. Subsequently the impinging vortex size will be increased, and the influence of the impinging vortex size on the variation in structure loading with vortex impact time will be assessed as part of Objective 3.

### 1.2.3 Objective 3

❖ *To define the trend in maximum force coefficient amplitude produced by a directly-impacting vortex when the vortex size is increased at fixed maximum tangential velocity and to explain the phenomenon controlling the maximum force coefficient amplitude trend.*

Structural loading by vortices is a hazardous phenomenon at practically all relative vortex-to-structure size scales, yet the literature is devoid of a systematic evaluation of how the size of an impacting vortex influences the amplitude the structural loading that it produces. The present study will evaluate the influence of the impacting vortex's size on the amplitude of structural loading that it produces by simulating the direct vortex impact with a slender structure. The maximum tangential velocity of the vortex will be fixed, and the vortex's size will be incrementally increased. Maximum force coefficient amplitudes will be reported for each impinging vortex size, and phenomenon controlling the trend in structural loading amplitude with respect to impinging vortex size will be illustrated and explained using contour plots of velocity and vorticity. As identified in Objective 2, maximum force coefficient amplitudes may vary when the impinging vortex impacts the structure at different times. A sufficient number of simulations will be conducted for each impacting vortex size to quantify the influence of the vortex size on the variation in structural loading when the vortex impacts at different times.

#### 1.2.4 Objective 4

❖ *To evaluate the influence of shifting an impacting vortex's path from the structure's centerline on the maximum structural loading produced by the vortex and to determine if the vortex produces greater structural loading than an equivalent-velocity free stream.*

An impinging vortex may not impact a structure directly, rather the vortex's path may be offset from the centerline of the structure that it loads. Adequate structural design for loading from impacting vortices necessitates that the impinging vortex path which produces maximum force coefficient amplitudes be identified and considered. The present study will evaluate the influence of shifting the impinging vortex's path from the impacted structure's centerline on the structural loading that the impacting vortex produces. The impinging vortex size that produces

the asymptotic, maximum force coefficient amplitudes for direct vortex impact is identified in Objective 3. This vortex size will be exclusively used, and its path will be shifted so that it travels both above and below the impacted structure's centerline. Maximum force coefficient amplitudes will be reported, and the vortex path shifts that produce maximum structural loading amplitude will be identified. Maximum force coefficients produced by the impacting vortex will be compared with those produced by an equivalent-velocity free stream to evaluate if the vortex travelling within a free stream produces greater structural loading than a free stream having equivalent maximum velocity to the vortex within the free stream.

#### 1.2.5 Objective 5

❖ *To develop a generalized methodology to assess dynamic amplification of structures' response to loading produced by an impacting vortex and to apply the methodology to assess the dynamic loading threat that tornados pose to residential and civil structures.*

Vortex impact with a structure produces a transient loading event, hence there is propensity for the vortex to produce dynamically-amplified response of the impacted structure. Numerous studies evaluate the capability of atmospheric vortices to dynamically excite structures. Hurricane vortices are massive and slow-moving, hence it is evident that they are incapable of dynamically exciting any realistic structure. However, tornados may be small and translate rapidly, hence their capability to dynamically load structures cannot be simply dismissed. Previous studies have concluded that the tornado-like wind loadings can excite some structures. However, these studies assume that tornado wind loads are no different that straight-line wind loads, and more significantly, they fail to produce a generalized, applicable methodology to assess the possible dynamic amplification of vortex-induced loading.

The present study will use the dynamic load factor (DLF) concept to develop the first generalized methodology to assess dynamic amplification of structures' response to loading produced by an impacting vortex. The load-application period  $T_v$  of an impacting vortex will be defined as a function of its TVP and parameters. Subsequently, documented tornado-vortex parameters will be surveyed and utilized to establish the possible range of tornado-vortex  $T_v$  values. Finally, the possible dynamic amplification of a structure's response to tornado-vortex loading will be defined as a function of the structure's fundamental period. Documented fundamental periods of real-world structures will be surveyed and used to evaluate the dynamic loading threat that tornados pose to typical residential and civil structures.

## CHAPTER 2: LITERATURE REVIEW

### 2.1 Introduction

This review summarizes the documented studies of structure loading by vortices from the literature. Subsequent subsections are organized to discuss studies conducted via the three primary approaches: physical experiment, computer simulation, and post-storm damage investigation. Post-storm damage investigation produces valuable statistics that allow assessment of the risk of severe storm occurrence based upon region of the country; this is valuable information for the designer of a structure, hence the statistics are included and discussed as well. Following the review of structure loading by vortices, the review proceeds to discuss the vortex tangential velocity profile. Vortices that produce loading in engineering applications are intense, meaning the tangential velocity component is much greater than the axial or radial velocity components; therefore, the vortex tangential velocity profile is of great importance in the present study. Theories regarding the underlying physics controlling vortex structure are presented and discussed. The reader is then referred to Chapter 3 for a thorough review and comparison of measured and analytical vortex tangential velocity profiles. Finally, the review structure loading by vortices is summarized, and the thesis motivation and progression are outlined.

### 2.2 Physical Experiments

#### 2.2.1 Blade-Vortex Interaction

There are no concise studies of structure loading from blade-vortex interaction experiments reported in the literature. The typical experiment configuration is a pair of in-line airfoils; the leading air foil produces a vortex that is tripped by some mechanism (wire, plunge, etc.) and shed to impact the trailing air foil (Ilie, 2009). Vortices can be consistently generated,

and the locations of the foils can be adjusted to ensure that the impinging vortex impacts the downstream air foil. However, the major problem with these physical experiments is that the shed vortex is rapidly attenuated as it is convected to impact the downstream air foil. Horner et al. (1996) report that it is difficult to transport a vortex through a distance that is more than a few times its diameter when the free stream velocity is greater than 50 m/s. Seath et al. (1989) report that interaction with the side-walls of the wind tunnel also rapidly attenuates the vortex during transport. Ilie (2009) reports that most blade-vortex interaction experiments use free stream velocity exceeding 100 m/s, thereby explaining why it is difficult to find reliable measurements of blade loads produced by impacting vortices. Briefly summarized, the impinging vortex is attenuated while being convected within the wind tunnel to impact the blade. Therefore, the parameters of the vortex that actually impacts the blade are not known. Consequently, it is not possible to develop meaningful correlations between impacting vortex parameters and the blade loading that is produced.

### 2.2.2 Tornado Simulator Experiments

The research group at Iowa State University uses the first large-scale, translating tornado simulator; their research is the current state of art in physical simulation of structure loading by tornado-like vortices. Sengupta et al. (2006 and 2008) report that tornado-like vortices produce greater loading when they travel at slow speeds than when they travel rapidly. They illustrate that interaction with the ground's boundary layer causes the vortex to tilt with the portion on the ground lagging behind the top of the vortex and conclude that this distortion is responsible for the decreased structure loading. Yang et al. (2009 and 2011) study the influence of y-direction (See Figure 1.5) path shift on the structure loading amplitude. They conclude that the greatest structure loading is produced when the vortex center travels above the structure at a shift of  $r_c$ .

Haan et al. (2010) simulate loading of a typical gabled residence. They report that lateral wall loading and uplift produced on the roof are respectively 1.5 and 1.8 to 3.2 times greater for tornado-like vortex loading than for equivalent-velocity straight-line wind.

## **2.3 Computer Simulation**

The complexity and high cost of physically simulating vortex-structure interaction encourages the use of computer simulation. Numerical diffusion is a plaguing issue that attenuates the impinging vortex as is the case in physical experiments. However, the substantial benefit afforded by computer simulation is that the exact properties of the vortex that impacts the loaded structure are known. Furthermore, at the expense of computational time, increased grid resolution and superior numerical solution methodologies can be utilized to reduce numerical vortex diffusion. Consequently, meaningful correlations may be developed between parameters of impacting vortices and the resulting structure loading that they produce.

### 2.3.1 Two-Dimensional Simulations

#### ***2.3.1.1 Blade-Vortex Interaction***

Blade-vortex interaction is primarily studied for application in rotorcraft design. Computer simulations typically consider a fixed- rather than rotating rotor. This is primarily because meshing a rotating system is very difficult and computationally expensive (Ilie, 2009). Furthermore, Ilie (2009) explains that the relative speed between the impinging vortex and the rotating rotor is the most important interaction parameter. This implies that impact of a stationary rotor by an impinging vortex is realistic representation of the chopping of a vortex by a rotating rotor.

Ilie (2009) simulates impact of an air foil by a clockwise-rotating, Scully-Kaufmann vortex at chord Reynolds number  $Re_c = 1.3 \cdot 10^6$ . The air foil chord length is  $c = 0.2$  m, and the

critical radius of the impinging vortex is increased from  $0.2 \cdot c$  and  $0.5 \cdot c$ . The kinematic viscosity of air at 300 K ( $\nu = 1.56 \cdot 10^{-5} \text{ m}^2/\text{s}$ ) is used in the present study to approximate the unreported free stream velocity  $U_\infty \approx 100 \text{ m/s}$ , and contour plots are used to approximate the unreported maximum vortex tangential velocity  $V_{\theta, \max} \approx 25 \text{ m/s}$ . Maximum drag and lift force coefficient amplitudes progressively increase as the impinging vortex size is increased. The path of the impinging vortex is shifted in the y-direction (See Figure 1.5) so that the impinging vortex travels below the air foil centerline by up to  $\delta = -1.25 \cdot r_c$ . Maximum drag and lift force coefficient amplitudes progressively decrease as the magnitude of  $\delta$  is increased. Later work performed in the same group reports that increasing the air foil angle of attack from  $0^\circ$  to  $9^\circ$  progressively increases drag and lift force coefficient amplitudes produced by vortex impact.

### ***2.3.1.2 Circular Cylinder-Vortex Interaction***

The circular cylinder is one of the most commonly-used body shapes in computational fluid dynamics simulations. Selvam et al. (2002) report preliminary work where they simulate loading of slender cylinder by a Rankine vortex at free stream Reynolds number  $Re = 10^3$ . The vortex critical radius is  $r_c = 3 \cdot D$  (The cylinder diameter  $D = 1$ ) and the maximum vortex tangential and vortex translational velocity are respectively  $V_{\theta, \max} = 4.5$  and  $U_\infty = 1$ . Selvam and Gorecki (2012) also simulate loading of a slender cylinder by a Rankine vortex at free stream Reynolds number  $Re = 10^3$ . The maximum vortex tangential velocity and free stream velocity are respectively fixed at  $V_{\theta, \max} = 3$  and  $U_\infty = 1$ . The critical radius of the impinging vortex is increased from  $r_c = 1 \cdot D$  to  $4 \cdot D$ ; the maximum lift force coefficient progressively increases with increasing size while the maximum drag force coefficient amplitude remains constant.

### 2.3.2 Three-Dimensional Simulations

Selvam and Millet (2003a, 2003b, and 2005) and Millet and Selvam (2005) simulate three-dimensional impact of a Rankine vortex with cube-shaped structures. The free stream velocity, maximum vortex tangential velocity, and vortex critical radius are fixed at  $U_\infty = 1$ ,  $V_{\theta, \max} = 4.5$ , and  $r_c = 3 \cdot D$  for all simulations. These studies collectively concluded that the vortex loading produces maximum lateral and uplift forces that are respectively 1.5 and 2 times greater than forces produced by an equivalent-velocity free stream. Also, the vortex loading produces a net torque about the vertical axis that is not produced by an incident free stream.

Alrasheedi and Selvam (2011) and Alrasheedi (2012) simulate three-dimensional impact of a Rankine vortex with square cross-sectioned, prismatic structures. The ratio of the vortex critical radius to the structure side length  $r_c/L$  is increased from 0.375 (vortex smaller than structure) to 30 (vortex much larger than structure), and the trend in force coefficients produced on the structure is documented with respect to  $r_c/L$ . Force coefficient amplitudes progressively increase as the ratio  $r_c/L$  increases. However, the grid refinement is too coarse to accurately resolve the boundary layer around the slender structures, as less than 20 span-wise control volumes are used to discretize structure faces when  $r_c/L > 4$  (Compare with 120 – 180 tangential nodes in the present study).

### **2.4 Post-Storm Damage Rating Investigation**

Post-storm damage investigation is not a usable approach to study the loading produced on individual structures by impacting vortices. It does however afford a viable means to qualitatively document severe damage from atmospheric vortices and document the regional threat thereof. This information can then be used to assess the probability of severe storm

occurrence at the location where a structure is to be constructed so that adequate design loads for the potential wind loading may be utilized.

#### 2.4.1 Damage Rating Scales

Tornados and hurricanes are categorized based upon the maximum wind speeds that they produce. Generally stated, local structure damage levels are correlated to equivalent, straight-line wind speeds required to produce similar damage based upon pre-defined damage markers (removed shingles, collapsed walls, uprooted trees, etc.). Saffir and Simpson (1974) develop the SS damage rating scale used to rate hurricane damage, and Fujita (1971) develops the F scale

**Table 2.1:** Saffir-Simpson damage scale (NOAA, 2012).

Category	Sustained Wind	Damage Description
1	33 – 43 m/s (74 – 95 mph)	<b>Some Damage:</b> roof, shingles, vinyl siding, gutters. Large tree branches snapped, shallow-rooted trees uprooted. Extensive Power Line damage.
2	43 – 49 m/s (96 – 110 mph)	<b>Extreme Damage:</b> major roof and siding damage. Shallowly-rooted trees are snapped and uprooted. Most power knocked out.
3	49 – 58 m/s (111 – 129 mph)	<b>Devastating Damage:</b> major roof damage and removal of roof decking/ gables. Many trees snapped. Power outages for days-weeks.
4	58 – 70 m/s (130 – 156 mph)	<b>Catastrophic Damage:</b> loss of roof structure and exterior walls. Most trees snapped, and power will be out for weeks-months.
5	> 70 m/s ( > 157 mph)	<b>Catastrophic Damage:</b> many framed homes will be destroyed, with total roof failure and wall collapse.

**Table 2.2:** Enhanced Fujita damage Scale (McDonald et al., 2009).

Rating	Wind Speed	Damage Description
EF0	29 – 38 m/s (65 – 85 mph)	<b>Light Damage:</b> Damage to chimneys and antennas; twigs broken off trees shallow-rooted trees pushed over.
EF1	39 – 49 m/s (86 – 110 mph)	<b>Moderate Damage:</b> Shingles removed, windows broken, trailers overturned, cars pushed off road, trees uprooted.
EF2	50 – 60 m/s (111 – 135 mph)	<b>Considerable Damage:</b> Roofs removed, light buildings demolished, light missiles generated, boxcars overturned, large trees snapped/uprooted.
EF3	61 – 74 m/s (136 – 165 mph)	<b>Severe Damage:</b> Roofs and walls removed, steel-framed hangers and warehouses torn, cars picked up, most trees flattened.
EF4	75 – 89 m/s (166 – 200 mph)	<b>Devastating Damage:</b> Houses leveled (debris left); steel structures damaged; trees debarked; cars/trains thrown; large missiles generated.
EF5	> 89 m/s ( > 200 mph)	<b>Incredible Damage:</b> Houses tossed from foundations; reinforced concrete structures damaged badly; car-sized missiles generated.

used to rate tornado damage. McDonald et al. (2009) explain that the F scale over-predicts wind speeds and scale down the original wind speeds attributed to the categories of damage markers thereby producing the Enhanced Fujita or EF scale. Table 2.1 and Table 2.2 respectively summarize the associated straight-line wind speeds and damage markers used to define the SS and EF ratings of hurricanes and tornados.

#### 2.4.2 Applicability of Damage Rating Scales

The SS and EF scales are good tools to document the occurrence of severe atmospheric vortices on the basis of the damage that they cause. This documented information yields valuable statistics regarding the risk of local severe storm occurrence. However, this is the extent of the applicability of post-storm damage investigation data to study structure loading by vortices.

It must be understood that the premise of both rating systems is correlating observed structural damage to straight line wind speeds required to cause the same damage. Both computer simulation (Selvam and Millet (2003a, 2003b, and 2005) and Millet and Selvam (2005)) and physical simulation (Haan et al., 2010) show that force coefficients produced by vortex loading of structures may be 1.5 to 3.2 times greater than force coefficients produced by a free stream having the same maximum velocity. This implies that the character of tornado wind loads increases the forces that they produce on structure compared with the forces produced by equivalent-velocity, straight-line wind. Consequently, the true maximum wind velocity within a tornado vortex may be much less than the correlated wind speed from damage investigation.

An additional problem with post-storm damage investigation is that numerous factors may influence the rating of observed local damage along the storm track. Investigation teams often consist of multiple investigators whom may assign different damage ratings to the same damaged structure. Furthermore, there is no guarantee that the most severely damaged structures

are located at each damage sight. Inconsistent construction techniques may cause structures that outwardly appear similar to suffer very different damage levels. Progressive damage due to longer time of exposure to high wind speeds may result in more severe damage from large tornados than from small tornados having similar maximum wind speeds. Womble et al. (2009) add the fact that damage from storm surge, flying debris, etc. may result in higher damage ratings being assigned than should be attributed to the local wind speed.

The preceding discussion introduces numerous flaws in post-storm damage investigation; however, ever-improving technology will continue to increase the viability of post-storm damage investigation in the future. Recent studies reported in Selvam et al. (2015a and 2015b) demonstrate the viability and extreme benefits of utilizing aerial photography to document and study tornado damage tracts. High-resolution photographs provided by sources such Google Earth and Civil Air Patrol are used to accurately document not only the tornado damage track but to assess the influence of topography on tornado damage levels. Damage documentation via aerial photography not only allows a single investigator to document the entire damage track but also readily provides access to areas that may not be reachable by road. As the quality and availability of aerial photography improves in the future, the process and ease of conducting post-storm damage investigation will be substantially improved.

Briefly summarized, the damage rating scales provide a good method to document the occurrence of severe atmospheric vortices and to identify regions of the country where extra design precautions against severe wind loadings should be made. Observed damage can be used to estimate maximum wind speeds, but the wind speed is likely overestimated.

### 2.4.3 Why are Different Damage Ratings Used for Tornadoes and Hurricanes?

The EF and SS scales discussed in the previous section correlate observed structure damage produced by tornadoes and hurricanes, respectively, to straight line wind speeds required to produce the same damage levels. One may question why two separate rating scales are needed and note that different wind speeds are required to produce similar damage levels in the SS and EF scales. Are hurricane wind loadings different than tornado wind loadings?

This discussion is prefaced by noting that abnormal vortex structure (multi-vortex, etc.) is not considered. Furthermore, the relative size of tornadoes to hurricanes is the primary difference between the two types of vortices. Womble et al. (2009) compare damage produced by hurricanes and tornadoes of similar maximum wind speeds. They conclude that neither tornadoes nor hurricanes are likely a concern for dynamic loading and that both are effectively turbulent straight line wind. Mehta and McDonald (1986) and McDonald et al. (2009) reach the same conclusion, specifically that tornadoes and hurricanes are effectively straight line turbulent wind. The literature concludes that both tornadoes and hurricanes can be considered straight line wind, hence the necessity for separate damage rating systems is still not understood.

Mehta and McDonald (1986) conclude that the difference in structure damage levels caused by tornadoes and hurricanes is due to the difference in time of exposure to high wind speeds, because of the difference in vortex size, and the different “character” of the winds. Womble et al. (2009) use similar language, attributing the difference in damage levels caused by tornadoes and hurricanes to the differences in “temporal and spatial variation in wind speeds”.

Attributing different damage levels to the difference in time of exposure to high wind speeds is reasonable, as progressive failure with time of exposure to high wind speeds could increase the structure damage levels. However, if both tornadoes and hurricanes act as straight

line turbulent wind (ie. Mehta and McDonald (1986) and McDonald et al. (2009)), what is the difference in wind “character” that influences structure loading?

Womble et al. (2009) identify a much more realistic reason for the use of separate damage scales: typical structures constructed in hurricane-prone areas (U.S. Atlantic and Gulf coasts) are designed to be more wind-resistant than structures in tornado-prone areas (U.S. Midwest). The EF and SS scales have been developed using observed structural damage from atmospheric vortices observed in different regions of the country where building codes may differ greatly. In short, a structure constructed for less wind resistance will exhibit greater damage for lower wind speeds than a structure constructed for higher wind resistance. Therefore, damage investigation scales calibrated for a region where structures typically have greater wind resistance is not directly applicable in a region where structures have decreased wind resistance. As long as hurricanes and tornados are to be considered as separate storms rather than collectively as atmospheric vortices, it is necessary that separate damage rating scales be used.

#### 2.4.4 Tornado and Hurricane Occurrence Statistics

Post-storm damage investigation is used to document the severity of both the hurricanes that make landfall on the Atlantic coast and the tornados occurring within the United States each year. Tables 2.3 and 2.4 summarize recorded hurricane and tornado data respectively. Note that hurricanes occurring prior to the creation of the SS scale in 1974 were likely post-documented based upon recorded damage.

Far fewer hurricanes make landfall each year ( $< 2$ ) than documented tornados that occur ( $> 1200$ ). However, due to the large size, duration, and storm surge that accompany hurricanes, they cause about 10 times greater financial loss (Brooks and Doswell, 2001) than do tornados

annually. However, hurricanes are slow-moving and easily tracked long before making landfall, meaning warnings can be delivered for evacuation of areas that will take severe damage.

**Table 2.3:** Summary of hurricane occurrence from 1851-2004 (Blake et al., 2005).

1851-2004	Saffir-Simpson Category					Total
	1	2	3	4	5	
Rating at Landfall	109	72	71	18	3	273
Percentage	39.93%	26.37%	26.01%	6.59%	1.10%	
Cumulative	39.93%	66.30%	92.31%	98.90%	100.00%	
Avg. Per Decade	7.1	4.7	4.6	1.2	0.2	17.8

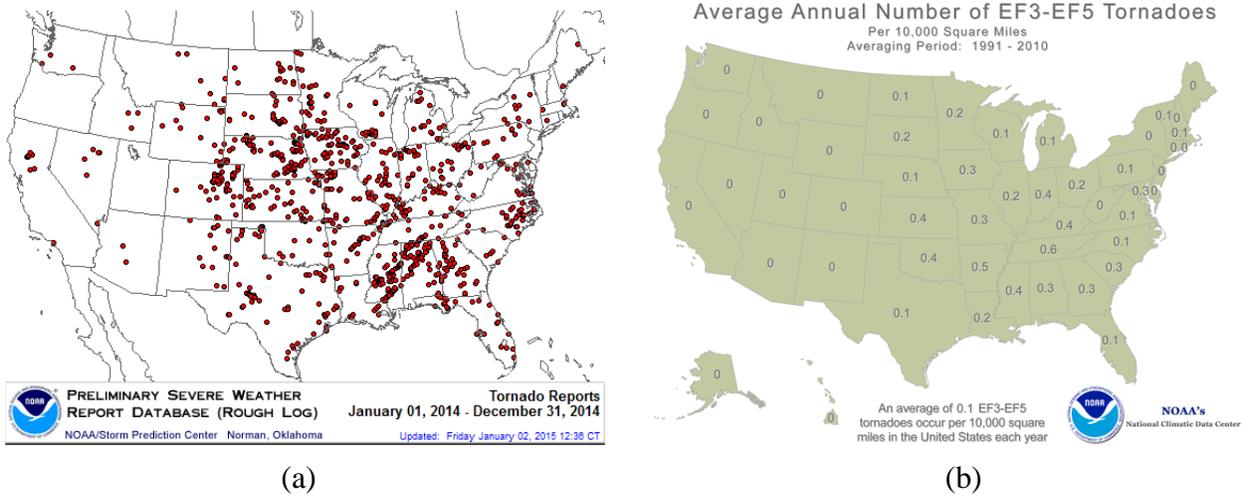
**Table 2.4:** Summary of tornado occurrence from 1995-2009 (Edwards and Brooks, 2010).

1995-2009	Enhanced Fujita Rating					Total
	0 - 1	2	3	4	5	
Damage Rating	17,095	1,440	419	85	7	19,046
Percentage	89.76%	7.56%	2.20%	0.45%	0.04%	
Cumulative	89.76%	97.32%	99.52%	99.96%	100.00%	
Avg. Per Decade	11,396.7	960.0	279.3	56.7	4.7	12,697.3

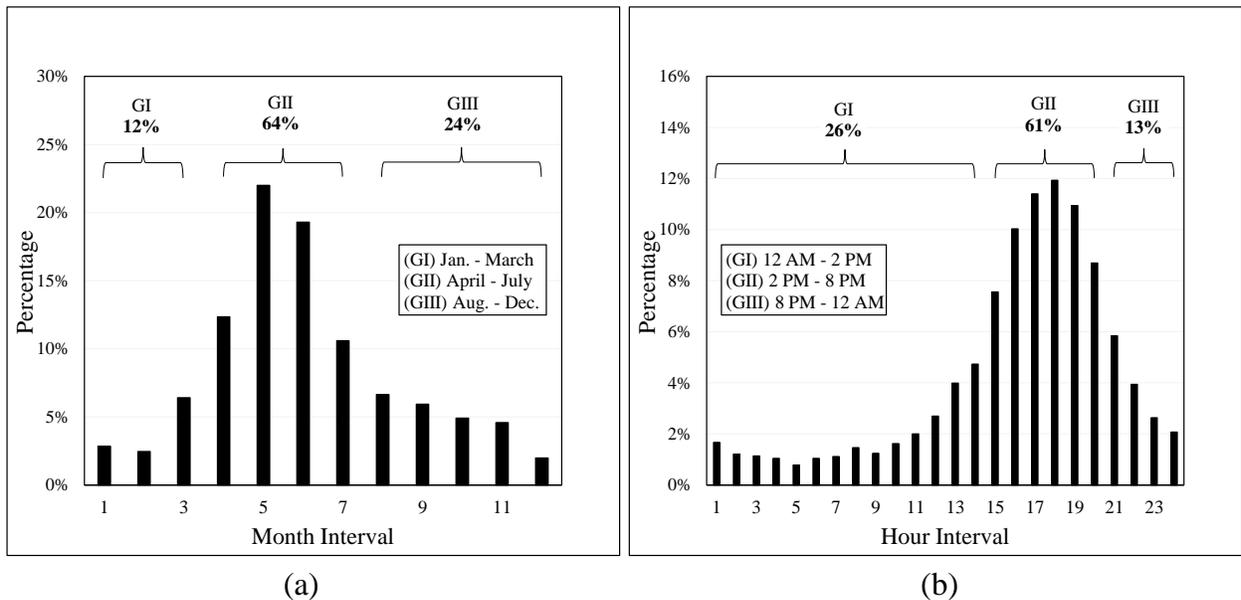
Tornados occur much more sporadically, and are likely on the ground for only a few minutes. Therefore, assessment of the risk of tornado activity in various regions of the country is a valuable statistic. Figure 2.1a illustrates the documented tornados in the United States for the year 2014, which is typical of the annual distribution of documented tornados. Figure 2.1b illustrates the density or average number of severe (EF3-EF5) tornados occurring in each state per 10,000 mi<sup>2</sup> of land area. Much of the western half of the United States has minimal risk of severe tornado occurrence. However, “Tornado Alley” in the central region of the United States along with many Eastern states have significant risk of severe tornado activity.

For staffing of emergency personnel, better prediction of tornado occurrence, and field study of tornados, it is helpful to know the time of year (Figure 2.2a) and time of day (Figure 2.2b) when tornados are likely to occur. Figures 2.2a and 2.2b show that most tornados occur in

the four-month interval from April to July, and most tornadoes occur within the six-hour interval 2:00-8:00 PM. It should be noted that tornadoes may be under-documented in the late evening and early morning hours due to the absence of sunlight to see them.



**Figure 2.1:** (a) Documented tornadoes in the United States in 2014 (Taken from NOAA, 2014a) and (b) tornado occurrences per 10,000 mi<sup>2</sup> land area in each state (Taken from NOAA, 2015a).



**Figure 2.2:** Average annual tornado occurrence as function of (a) month and (b) time of day (NOAA, 2015a).

## 2.5 Dynamic Structure Loading by TORNADOS

TORNADOS and hurricanes produce transient structure loading events, and there is question as to whether or not they can produce dynamically-amplified structure loading. Researchers from the post-storm damage investigation community reason that both tornadoes and hurricanes are too large to dynamically load a structure (Mehta and McDonald (1986) and Womble et al. (2009)). Hurricanes are many kilometers in diameter and move slowly, hence it is intuitive that they cannot dynamically load any practical structure. TORNADOS, however, range greatly in size and translational speed, hence their capability to dynamically load structures cannot be so simply dismissed.

The literature contains no physical experiments measuring dynamically-amplified structure response to vortex loading. However, several studies evaluate dynamic structure response amplification to assumed tornado wind loads. All studies define the time history of the wind velocity incident on the building and then use empirical equations defined as functions of the incident wind velocity to define the forcing time history. Wen (1975) studies the response of a multi-story building to a simplified Kuo (1971) vortex and reports that the maximum structure response is amplified by a dynamic load factor (DLF) of 4.0. Two similar studies evaluate multi-story structures' responses to a modified RCVM vortex (The potential vortex region decays linearly) and report maximum response amplification of  $DLF = 2.0$  (Tan (1975) and Seniwongse (1977)). Dutta et al. (2002) analyze the responses of single- and multi-story structures to a tornado wind field assigned using Fujita's (1976) wind speed record and Mehta et al.'s (1976) vortex model and report maximum structure response amplification of  $DLF = 1.735$ .

The preceding collection of studies jointly conclude that transient tornado wind loads are capable of dynamically amplifying structures' responses. Significant findings from the studies are outlined below.

- Structure response amplitude may be amplified by DLF of 1.735 (Dutta et al., 2002) to 4.0 (Wen, 1975).
- The threat of dynamic amplification of tornado wind loads is greatest when the structure is directly on the tornado's path ( $\delta = 0$ ) due to the rapid change in direction and amplitude of the flow incident on the cylinder which is augmented by the vortex's tangential velocity ((Wen, 1975), (Tan, 1975) and (Seniwongse, 1977)).
- The y-direction forcing has (See Figure 1.5) greatest propensity to produce amplified structure response, hence it is the forcing component that need be considered for dynamic analysis ((Tan, 1975) and (Seniwongse, 1977)).

The collective shortcoming of these studies is that they fail to yield generalized results, but rather discuss the dynamic amplification of a particular structure's response to loading by a particular wind field. Furthermore, all of the studies define the forcing time histories using empirical equations developed for straight-line wind loading, hence they premise that the rotational nature of the vortex's wind field does not influence the structural loading.

Measured tornado parameters and wind field profiles, as well as real-world structure parameters, vary widely. Adequate structure design for dynamically-amplified structure response to tornado wind loads necessitates the definition of a generalized methodology to assess the possible dynamic amplification of a structures response to loading by all possible tornado vortices.

## 2.6 Comparison of Real-World and Simulated Tornado and Hurricane Vortices

Numerous physical and computer simulations of vortex-structure interaction have been discussed. However, to better understand real-world vortex loading of structures, simulation parameters need be realistic, or realistically scaled systems. Parameters for some significant tornados as well as a single hurricane are provided in Table 2.5. The Tri-state and Bridge Creek-Moore tornados are specifically selected because they are the fastest-translating and having the highest recorded wind speeds respectively. Translational velocities ( $U_\infty$ ) are known based upon progressive damage documentation. The tangential velocities are back-computed based upon measured total velocity or approximated using the EF damage rating. Hurricane Allen is included here because it produced the fastest recorded hurricane wind speeds.

**Table 2.5:** Summary of documented tornado and hurricane wind speeds.

Title	$V_{\max}$ m/s	$V_{\theta,\max}$ m/s	$U_\infty$ m/s	$V_{\theta,\max}/U_\infty$ dim	$r_c$ m	Source
Tri-State	134.1	101.5	32.6	3.11	800	Fujita, 1973
GG Nuclear Plant	68.9	53.6	15.2	3.53	46	Fujita, 1981
Bridge Creek-Moore	134.6	123.2	11.4	10.85	1,609	NOAA, 1999
El Reno	132.3	116.7	15.7	7.45	2,008	NOAA, 2013a
Moore	93.9	82.3	11.6	7.08	1,004	NOAA, 2013b
Mayflower	89.4	71.1	18.3	3.88	600	NOAA, 2014b
Hurricane Allen	84.9	76.0	8.9	8.50	46 (km)	NOAA, 1980

The vortex velocity ratio for tornados is  $3.11 \leq V_{\theta,\max}/U_\infty \leq 10.85$  for the cases presented in Table 2.5, which seem to be the representative range of fast-moving to slow and rapidly-rotating. Hurricane wind speeds shortly prior to, and just after landfall, are not precisely documented in the literature. NOAA (2012) averages the forward speed of 39,877 North Atlantic hurricanes and reports the average forward speed to 5.59 m/s. Using the wind speed for a SS1 hurricane (33 m/s) as a lower velocity bound and Hurricane Allen's wind speed as the upper

velocity bound, the velocity ratio range for hurricanes is  $4.90 \leq V_{\theta, \max}/U_{\infty} \leq 14.19$ . Therefore, for atmospheric vortices, a reasonable velocity ratio range is  $3.11 \leq V_{\theta, \max}/U_{\infty} \leq 14.19$ .

The maximum tangential and translational velocities for vortices in computer and physical blade-vortex interaction simulations is not often documented precisely in the literature. However, since the vortices are produced by flow over rotating and fixed wings, it is reasonable to conclude the maximum velocity ratio for blade-vortex interaction experiments is less than one (deduced from contours in (Ilie, 2009) and (Porter et al., 2010)). These vortices are not maintained, hence they decay as they convect away from the rotor or blade that produces them. Therefore, the velocity ratio range relevant for blade vortex interaction can be defined as  $V_{\theta, \max}/U_{\infty} \leq 1$ .

Table 2.6 summarizes the parameters of vortices used in computer simulations used in previously-discussed computer simulations. Parameters for the non-CFD studies of Wen (1975) and Dutta et al. (2002) are included as well. No blade-vortex interaction simulations are reported here, because complete sets of parameters are not provided in any of the cited studies. However,

**Table 2.6:** Summary of vortex parameters for computer experiments.

Source	Type	$V_{\max}$ m/s	$V_{\theta, \max}$ m/s	$U_{\infty}$ m/s	$V_{\theta, \max}/U_{\infty}$ dim	$r_c/D$ dim
Wen, 1975	Eqs.	92.0	74.0	18.0	4.11	2.50
Duttah et al., 2002	Measured	100	82	18.0	4.56	$r_c \approx 25$ m
Selvam et al., 2002	2D CFD	111.7	91.4	20.3	4.50	3.00
Selvam and Millet, 2003a	3D CFD	111.7	91.4	20.3	4.50	3.00
Selvam and Millet, 2003b	...	...	...	...	...	...
Selvam and Millet, 2005	...	...	...	...	...	...
Millet and Selvam, 2005	...	...	...	...	...	...
Selvam and Gorecki, 2012	2D CFD	80.0	60.0	20.0	3.00	1.00 to 4.00

It is again noted that both  $V_{\theta}/U_{\infty}$  and  $r_c/D$  are typically less than or equal to unity in the blade-vortex studies in Ilie (2009) and Porter et al. (2010). The velocity ratio range used in these

simulations,  $3.0 \leq V_{\theta}/U_{\infty} \leq 4.56$ , is very close to, or within the atmospheric vortex velocity ratio range defined previously.

Parameters from the three-dimensional tornado simulations conducted by the group at Iowa State University are summarized in Table 2.7. It is immediately observed that the velocity ratio range  $11 \leq V_{\theta,\max}/U_{\infty} \leq 65$  is much greater than that typically observed in tornados. However, it must also be pointed out that tornados change wind speed and direction frequently and may even stop, at which point  $V_{\theta,\max}/U_{\infty} \rightarrow \infty$ . Therefore, the velocity ratio used in the Iowa State three-dimensional experiments is not physically impossible, although it is not typical of the tornado vortices that they are supposedly studying.

**Table 2.7:** Summary of vortex parameters from three-dimensional tornado simulator at Iowa State University.

Source	$V_{\theta,\max}$ m/s	$V_{\theta}$ m/s	$U_{\infty}$ m/s	$V_{\theta,\max}/U_{\infty}$ dim	$r_c/D$ dim
Sengupta et al., 2006	$\leq 11.6$	11.0	0.30 - 0.61	18 - 37	2.62 - 5.32
Sengupta et al., 2008	$\leq 10.3$	9.7	0.30 - 0.61	16 - 32	2.62 - 9.82
Haan et al., 2010	$\leq 10.4$	6.8-9.8	0.15 - 0.61	11 - 65	5.06 - 11.64
Haan et al., 2014	10.0	9.8	0.15	65	2.00 - 5.60

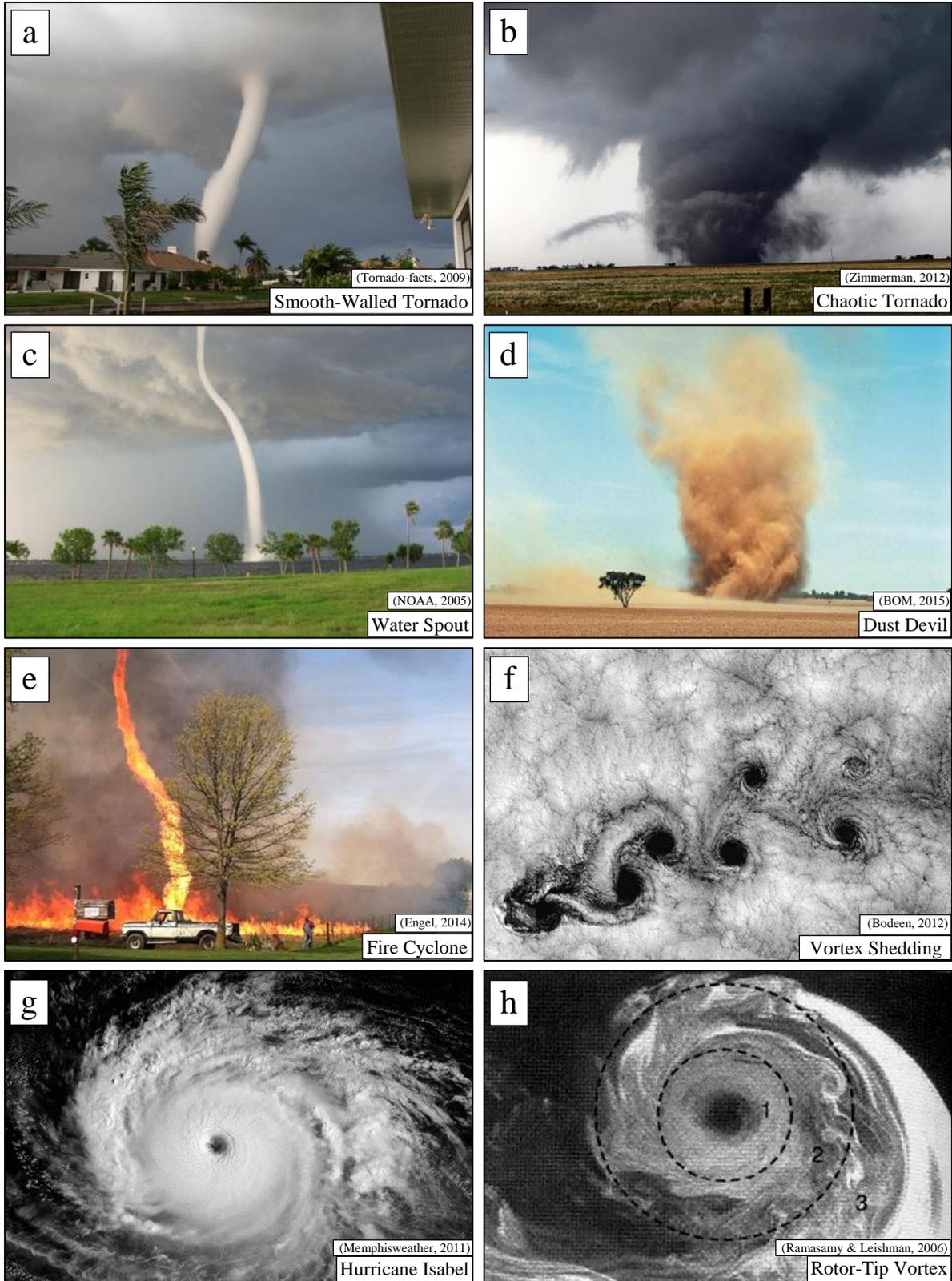
## 2.7 The Tangential Velocity Profile of a Vortex

Vortices are very complex flow features that are produced and sustained by different sources. Atmospheric vortices may be narrow and laminar as illustrated in Figures 2.3a, 2.3c, and 2.3e or wide and turbulent as illustrated in Figures 2.3b and 2.3d. Atmospheric vortices are large, exhibit three-dimensional flow, and are created and sustained by convection currents. Conversely, rotor- and wing-tip vortices are small, mechanically-produced and non-maintained flow structures that decay rapidly after detaching from the wing or rotor from which they originate. Despite the noted differences in scale and production of atmospheric and rotor/wing-

tip vortices, they still exhibit very similar structure as can be seen by comparing the hurricane vortex illustrated in Figure 2.3g and the rotor-tip vortex illustrated in Figure 2.3h.

Vortices are primarily investigated by the meteorological, structural, and aerospace communities. Meteorological researchers have placed substantial research effort on forecasting the trajectory and growth of hurricanes so that ample warning can be given to coastal areas before the hurricanes make landfall (Goerss and Jeffries (1994), Vickery et al. (2009), and Cao et al. (2011)). Research effort has been spent studying the three-dimensional structure of tornados (Lewellen (1993), Nolan and Farrell (1999), and Davies-Jones et al. (2001)) as well as the influence of topography on the near-ground behavior of tornados (Gorecki and Selvam (2014) and Gorecki and Selvam (2015)). The remaining research effort within the structural design and aerospace communities focuses on predicting structure loading by vortices.

The radial profile of a vortex can be thought of as three regions: a laminar core, a transition region, and a turbulent exterior region, as labeled in Figure 2.3h. The literature contains numerous analytical expressions for the radial tangential velocity profile  $V_{\theta}(r)$  or TVP of a vortex. The aerospace community describes shed vortices using the vortex Reynolds number ( $Re_v = \Gamma_{\infty}/\nu$ ), where the maximum circulation  $\Gamma_{\infty}$  is the path integral of the maximum tangential velocity ( $\Gamma_{\infty} = 2 \cdot \pi \cdot r_c \cdot V_{\theta, \max}$ ). Vatistas (2006) concludes that the TVP increasingly flattens for  $r > r_c$  as  $Re_v$  increases, implying a slower decay in tangential velocity. He attributes the increased profile flattening to increasingly-turbulent diffusion of the vortex. The most commonly-used TVPs by the aerospace community are the Scully-Kaufmann (Vatistas, 2006) and Lamb-Oseen (Leishman and Martin, 2006) models. These are both laminar, solutions of the Navier-Stokes equations. Vatistas (2006) shows that both profiles accurately approximate high- $Re_v$  rotor tip vortices when they are normalized so that  $V_{\theta}(r_c)/V_{\theta, \max} = 1$ . Tornados and hurricanes are most



**Figure 2.3:** Examples of real-world atmospheric and mechanically-produced vortices.

commonly modeled using the Modified Rankine Combined or Burgers-Rott TVPs (Wood and White, 2011).

Much research effort has been put into defining the correct TVP in the various vortices of engineering interest. However, understanding of, and analytical approximation of vortices is still at best an educated guess. Laboratory investigation of vortices produced by rotor tips typically measure vortices characterized by  $Re_v < 3 \cdot 10^5$  (Bagai and Leishman (1993), Bhagwat and Leishman (2000), and Martin et al. (2003)), while real-world rotorcraft produce vortices at  $Re_v > 10^7$  (Bhagwat and Leishman, 2002). Field measurements of TVPs within tornados accurately fit several different analytical profiles (Tanamachi et al. (2007), Wurman et al. (2007), and Wood and White (2011)). However, as noted by Kessler (1970) the tornado structure varies widely between tornados, ranging from laminar (Figures 2.3a) to turbulent (Figures 2.3b), hence no single TVP fits all measured profiles.

Mobile Doppler radar has substantially improved the research community's knowledge of tornado wind field structure in recent years. However, radar measurements are limited to about 30 m from the ground, hence the near-ground wind structure of tornado wind fields remains an educated guess (Wurman et al. (2007)). Measured TVPs within hurricanes have also been accurately fit with analytical models (Mallen et al. (2005) and Wood and White (2011)).

However, the vortex structure varies significantly between storms, with the inner core ranging from v- to u-shaped (Kepert, 2010). Furthermore, hurricane structure is influenced by numerous parameters: season, basin environmental pressure, time of day, etc. as discussed in Cao et al. (2011). The fact that all of these parameters influence the hurricane TVP leads to the conclusion that no single TVP profile can analytically represent all hurricanes.

Briefly summarizing the preceding discussion, numerous vortex structures are possible for both mechanically produced (rotor, blade, etc.) and convection driven (tornado, hurricanes, etc.) vortices. The vortex structures change continuously due to viscous diffusion, ground interaction, etc., hence it is not possible to analytically define a single analytical TVP that fits all vortices, even within a specific classification (Rotor tip, tornado, hurricane, etc.).

## **2.8 Summary and Thesis Motivation**

### 2.8.1 Summary of Literature Review

Vortex impact with structures is a complex phenomenon that produces hazardous structure loading and in numerous engineering applications. The aerospace community desires to better predict air loads placed on rotors of rotorcraft as they chop vortices produced by other rotors or shed from leading wings and stabilizers. The structural design community seeks to better understand structure loading produced on residences and civil structures by atmospheric vortices such as tornados and hurricanes so that adequate design provisions may be adopted. Despite the far-reaching necessity for better understanding of structure loading by vortices, the current knowledge base is limited, and that information which is supposedly “known” is typically debated as summarized below.

Both three-dimensional computer simulations (Selvam and Millet, 2003a and 2003b) and three-dimensional physical experiments (Haan et al., 2010) demonstrate that structure loading by tornado-like vortices respectively produces 1.5 and 3.2 times the lateral and uplift forces that are produced by equivalent-velocity, straight-line wind. However, some still reason that both tornado and hurricane wind loads can be assumed to behave as straight-line wind (McDonald et al. (2009) and Womble et al. (2009)). This demonstrates that the academic community still in

unsure whether or not the rotational flow structure of vortices produces greater structure loading than straight-line wind having the same velocity.

Two-dimensional computer simulations are used to show that structure loading amplitude progressively increases as the size of the impinging vortex is increased. Ilie (2009) simulates impact of a Scully-Kaufmann vortex with an airfoil and shows that the air foil loading amplitude progressively increases as the vortex critical radius is increased from  $0.2 \cdot c$  to  $0.5 \cdot c$  ( $c$  is the airfoil chord length). Gorecki and Selvam (2013) simulate impact of a Rankine vortex with a slender, circular cylinder and show that the cylinder loading amplitude progressively increases as the vortex crucial radius is increased from  $1 \cdot D$  to  $4 \cdot D$ . Gorecki and Selvam (2013) fix the vortex's maximum tangential velocity, and Ilie (2009) does not report his treatment thereof.

Both two-dimensional computer simulation (Ilie, 2009) and three-dimensional physical experiments (Yang et al., 2009 and 2011) demonstrate that shifting the impinging vortex path relative to the impacted structure greatly influence the structure loading amplitude. Ilie (2009) simulates impact of an airfoil by a clockwise-rotating vortex and finds that structure loading amplitude progressively decreases as the impinging vortex path is shifted below the structure by up to  $\delta = 1.25 \cdot r_c$ . Yang et al. (2009 and 2011) effectively perform the same study by shifting the path of a counter-clockwise vortex by up to  $\delta = 7 \cdot r_c$  above the structure. However, Yang et al. (2009 and 2011) report that structure loading amplitude increases up to  $\delta = r_c$ . Therefore, it is concluded that the physical influence of vortex path shift on the produced structure loading amplitude is not clearly defined in the literature.

Several studies evaluate the capability of tornado wind loads to produce dynamically-amplified structure response (Wen (1975), Tan (1975), Seniwongse (1977), and Dutta et al. (2002)). In all cases, a time history of the assumed tornado wind field is defined and then

converted to a forcing time history using empirical equations; the forcing is then applied to a response model. All studies conclude that tornado wind loads are capable of dynamically amplifying structure response, with maximum reported dynamic load factor values ranging from 1.735 (Dutta et al., 2002) to 4.0 (Wen, 1975). However, others conclude that neither tornados nor hurricanes pose a dynamic loading threat simply upon the basis of post-storm damage investigation and intuition (Mehta and McDonald (1986) and Womble et al. (2009)).

Numerous analytical expressions have been developed for the radial tangential velocity profile (TVP) within a viscous vortex. The aerospace community typically uses the Lamb-Oseen (Leishmann and Martin, 2006) or Scully-Kaufmann (Vatistas, 2006) TVPs to approximate rotor and wing-tip vortices. Atmospheric vortices are typically approximated with the Modified Rankine Combined or Burgers-Rott (Wood and White, 2011) TVPs. Theoretical discussions of the factors influencing the TVP of a vortex are present in the literature, but the noted variation in structure between similar types of vortices underscores the fact that the physics governing vortex structure is not well-understood. At the present time that there is no single analytical vortex model that can be used to represent all vortices.

### 2.8.2 Thesis Motivation

Structure loading by vortices is a complex phenomenon, and the resulting structure loading introduces hazards in numerous fields. The complexity of vortex structure interaction necessitates study by physical experiment and computer simulation. Physical experiments are inherently expensive and difficult to utilize. Computational power of computers is rapidly increasing, and it is becoming possible to utilize increasingly-realistic computer models to simulate realistic vortex-loading of structures. The goal of this thesis is to analyze and define some of the key aspects of vortex loading of structures, the shortcomings of which have been

summarized in the previous subsections. The attained understanding can then be applied in numerous fields to increase structure resistance to vortex loading.

Vortex impact with structures is a relevant, load-producing phenomenon at practically all relative vortex-structure size scales. Rotor- and wing tip vortices may be much smaller than the chord length of the rotors that chop them ( $r_c/c \leq 0.15$ ). Tornado and hurricanes respectively have critical radii exceeding 2 km and 45 km, so the relative vortex-structure size ratio  $r_c/D \rightarrow \infty$ . Practical engineering applications necessitate understanding of structure loading amplitude produced by vortices across the relative size ratio range of  $0.15 \leq r_c/D \leq \infty$ . However, the current range covered in the literature, neglecting the coarse-grid simulations of Alrasheedi (2012), is  $0.15 \leq r_c/D \leq 4.0$ . Furthermore, the computer simulations spanning this range utilize different physical systems (airfoil vs. slender cylinder) and different vortex models (Scully-Kaufmann vs. Rankine Combined). In short, the upper bound of the relative size ratio range at 4.0 is far below the upper bound for practical situations. A consistent data set attained using a fixed physical system and vortex model is needed to establish the influence of the relative vortex-structure size on the trend in maximum structure loading amplitude.

In aerospace applications, vortices are produced by- and chopped by rotors, so designers may know the vortex path with respect to the rotor. However, in scenarios such as tornado loading of structures, the vortex may travel along any path relative to the loaded structure. The literature contains computer simulations and physical experiments that simulate vortex travel above and below the loaded structures; however, the studies use different physical models (air foil vs. prismatic structure) and vortex rotation directions (clockwise vs. counter-clockwise). A consistent data set is needed to establish the influence of vortex path shift on structure loading amplitude. This knowledge can potentially be applied in the design of aerospace vehicles to

minimize vortex-applied air loads or to assess the maximum loading that tornado or hurricane vortices can produce on structures.

The capability of tornados to produce dynamically amplified structure response is reported in the literature. Investigators relying on intuition and post-storm damage investigation conclude that tornados are too large to dynamically load structures. However, researchers who apply assumed tornado wind fields to response models arrive at the contradictory conclusion that tornado wind loads may be amplified by as much as four times. Current design codes make no provision for tornado wind loads, which have already been demonstrated by computer simulation and physical experiment to produce 1.5 to 3.2 times greater aerodynamic loading than straight line wind of the same maximum velocity. Dynamic amplification of the already under predicted aerodynamic forces could result in structure loading that is much greater than that predicted by assuming that tornado wind loads are the same as straight line wind loads. The collective shortcomings of the previous study are that they simply dismiss the dynamic loading threat and those that consider it fail to develop a generalized, applicable assessment or methodology to assess the possible dynamic load amplification of a given structure. Integration of provisions for dynamic amplification of structure response to tornado wind loads necessitates the definition of a generalized methodology to assess the possible dynamic amplification of any structure's response to any tornado wind loading.

Realistic simulation of vortex loading of structures requires the use a realistic vortex model. Vortices that pose structure loading threats are “intense”, meaning that the tangential velocity is much greater than the radial or axial velocity components. Tangential velocity profiles of real-world vortices are qualitatively similar, but differ quantitatively, hence no single analytical vortex tangential velocity profile realistically represents all vortex tangential velocity

profiles. Therefore, it is necessary to identify a group of analytical vortex tangential velocity profiles that represent the possible range of measured profiles for use in computer simulation.

### 2.8.3 Progression of Thesis

Chapter 3 documents the selection of realistic vortex tangential velocity profiles (TVPs) for use in computer simulation. Analytical TVPs are assembled and normalized so that  $V_{\theta}(r_c) = 1$  for comparison. Measured TVPs from various types of experimentally-produced vortices as well as from field-measured tornado and hurricane vortices are then assembled and normalized for comparison as well. Analytical TVPs that best represent the upper and lower bounds of the measured TVPs are identified along with the analytical TVP that best represents the typical measured TVP.

Chapter 4 introduces and outlines the physical model and numerical method used to simulate two-dimensional impact of a slender cylinder by a vortex. Boundary conditions and specific details of the utilized grids are illustrated and discussed. The model and grids are then validated using simulated free stream flow over an immersed cylinder. The capability of the model and grid to transport the impinging vortices is subsequently assessed.

Chapter 5 defines the trend in maximum cylinder loading amplitude with respect to the relative vortex-structure size. The impinging vortex critical radius is incremented from one to one-hundred times the cylinder diameter while the free stream velocity and maximum vortex tangential velocity are fixed. Phenomenon influencing the cylinder loading amplitude trend are illustrated and explained. The vortex size beyond which cylinder loading amplitude becomes asymptotic is identified. The vortex size producing asymptotic loading is then used to evaluate the influence of vortex path shift on structure loading amplitude. The vortex path is shifted so that the vortex center travels both above and below the cylinder. The trend in cylinder loading

with respect to path shift is documented, and illustrations are presented and discussed to explain the loading trend.

Chapter 6 is the final portion of this thesis and evaluates the capability of tornados to produce dynamically amplified structure response. The dynamic load factor concept is utilized to define the load application period of an impinging vortex as function of its critical radius, translational velocity, and tangential velocity profile. Fundamental periods of real-world structures are surveyed and categorized. The range of documented tornado vortex parameters is compiled and applied to define a universal dynamic load factor curve for tornado wind loads as function of the fundamental period of the loaded structure.

### CHAPTER 3: SELECTION OF REALISTIC TANGENTIAL VELOCITY PROFILES

Realistic computer simulation of structure loading by vortices necessitates the use of a physically-realistic vortex velocity field model. Vortices of interest in engineering applications are “intense”, meaning the tangential velocity  $V_\theta$  is much greater than the radial or axial velocity components (Vatistas, 1998). Therefore, it is imperative that vortex model used in the present study have a physically-realistic tangential velocity profile (TVP).

The literature contains many analytical vortex TVP models, which are reviewed extensively by Bhagwat and Leishman (2002) and Wood and White (2011). Most of the analytical TVPs are derived by using simplifying assumptions to solve the Navier Stokes equations. Each analytical TVP realistically represents a measured experimental or atmospheric vortex TVP. However, measured vortex TVPs vary greatly ((Wurman et al., 2007) and (Kepert, 2010)) and no single analytical TVP represents all of the measured TVPs. The goal of the forthcoming study is to establish a group of analytical TVPs that represent the range of measured vortex TVPs.

The following sections begin with a review of the commonly-used analytical TVPs. Derivations and simplifying assumptions are not discussed herein, but the interested reader may find them in the referenced studies. The analytical TVPs are then normalized as necessary so that  $V_\theta(r_c) = \alpha \cdot r_c$ . Subsequently, measured TVPs are extracted from the literature, normalized for comparison, and categorized according to experiment and vortex type. Finally, the analytical and measured TVPs are compared, and the analytical TPVs which best represent the measured TVPs are identified for subsequent use in computer-simulated vortex loading of structures.

### 3.1 Analytical Tangential Velocity Profiles

#### 3.1.1 Bi-Regional Profiles

The Rankine Combined Vortex Model (RCVM) developed by Rankine (1882) is the oldest and simplest TVP that is still in use. The TVP is bi-regional, consisting of a solid-body inner core ( $r/r_c \leq 1$ ) and an potential vortex beyond the critical radii ( $r/r_c > 1$ ). The RCVM TVP exhibits a sharp peak at the critical radius ( $r = r_c$ ) where the TVP transitions from a solid-body vortex to a potential vortex. This sharp peak is not realistic of real-world vortices because the fluid's viscosity causes dissipation of the vortex, producing a smooth, rounded peak at the critical radius (Vatistas et al., 1991). The solid-body inner core and exterior potential vortex are defined by Eqs. (3.1) and (3.2) respectively.

$$V_{\theta,RCVM}(r) = \left(\frac{r}{r_c}\right) \quad (0 \leq r/r_c \leq 1.0) \quad (3.1)$$

$$V_{\theta,RCVM}(r) = \left(\frac{r_c}{r}\right) \quad (r/r_c > 1.0) \quad (3.2)$$

Hughes (1952) modifies Rankine's (1882) profile by adding an exponent ( $x$ ) to the definition of the potential vortex, defining the Modified Rankine Combined Vortex Model (MRCVM). The MRCVM is used extensively to model tornado and hurricane TVPs in the literature, with values of  $x$  ranging from 0.04 to 0.9 as summarized in Table 3.1.

**Table 3.1:** Values of exponent “ $x$ ” used in the literature for tornado and hurricane TVPs.

Source	Vortex Type	$x$
Hughes (1952)	Hurricane	0.62
Goerss and Jefferies (1994)	Hurricane	< 0.9
Leslie and Holland (1995)	Hurricane	0.5 to 0.6
Mallen et al. (2005)	Hurricane	0.04 to 0.67
Wurman et al. (2007)	Tornado	0.6
Kosiba and Wurman (2010)	Tornado	0.4 to 1.0
Holland et al. (2010)	Hurricane	0.5

High values of  $x$  theoretically correlate to a laminar vortex because the TVP decays rapidly for  $r/r_c > 1$ ; conversely, low values of  $x$  correlate to a slowly-decaying vortex for  $r/r_c > 1$  whose profile is flattened due to turbulent diffusion. The solid-body inner core and modified exterior potential vortex regions are defined by Eqs. (3.3) and (3.4) respectively.

$$V_{\theta, \text{MRCVM}}(r) = \left(\frac{r}{r_c}\right) \quad (0 \leq r/r_c \leq 1.0) \quad (3.3)$$

$$V_{\theta, \text{MRCVM}}(r) = \left(\frac{r_c}{r}\right)^x \quad (r/r_c > 1.0) \quad (3.4)$$

### 3.1.2 Continuous Profiles

The Lamb (1932) – Oseen (1912) (L-O) TVP is commonly used to model rotor-tip vortices (Ramasamy and Leishmann, 2006). The TVP is defined in Eq. (3.5). Rotor tip vortices are not maintained after being shed and progressively grow larger due to the fluid viscosity. The growth of the vortex is incorporated into the TVP using the time-dependent critical radius definition Eq. (3.6), where  $\alpha_o = 1.25643$  is the Oseen constant. The L-O TVP is singular at the center of the vortex ( $r = 0$ ), hence special provision must be made when defining the TVP at this ordinate.

$$V_{\theta, \text{L-O}}(r) = \frac{r_c}{r} \cdot \left[ 1 - \exp\left(-\alpha_o \cdot \frac{r^2}{r_c^2}\right) \right] \quad (3.5)$$

$$r_c(t) = \sqrt{4\alpha_o vt} \quad (3.6)$$

Burgers (1948) and Rott (1958) propose a steady solution of the L-O TVP by fixing the vortex critical radius. Their steady solution is used extensively to represent the TVP of maintained atmospheric vortices such as tornados (Tanamachi et al., 2007).

Sullivan (1958) derives the “two-cell” vortex TVP defined using Eqs. (3.7) and (3.8). Sullivan’s TVP is used extensively to represent tornado vortex TVPs (Wood and White, 2011).

$$V_{\theta,s}(r) = \frac{r_c}{r} \cdot \left[ \frac{H\left(\frac{ar^2}{2v}\right)}{H(\infty)} \right] \quad (3.7)$$

$$H(x) = \int_0^x \exp\{-\tau + 3 \int_0^\tau [(1 - e^{-\tau})/\tau] \} d\tau \quad (3.8)$$

Wood and White (2011) simplify and normalize Eq. (3.7) so that  $V_{\theta}(r_c) = 1$ , yielding Eq. (3.9).

Further simplification is provided by Leslie and Snow (1980) who report that the denominator of Eq. (3.9) is 37.9043 and by Vatisstas (1998) who reports that  $\beta = 6.238$ . Incorporating these two constants, the simplified and normalized adaption of Sullivan's TVP is provided in Eq. (3.10).

$$V_{\theta,s}^*(r) = \frac{r_c}{r} \cdot \left[ \frac{H(\beta \cdot (r/r_c)^2)}{\lim_{r/r_c \rightarrow \infty} H(\beta \cdot (r/r_c)^2)} \right] \quad (3.9)$$

$$V_{\theta,s}^*(r) = \frac{r_c}{r} \cdot \left[ \frac{H(6.238 \cdot (r/r_c)^2)}{37.9043} \right] \quad (3.10)$$

Leslie and Snow (1980) show that  $H(x)/H(\infty) \geq 0.99$  for  $r/r_c \geq 1.3$ . This means that the Sullivan TVP (Defined in Eq. (3.7)) is effectively the potential vortex defined in Eq. (3.2) for  $r/r_c \geq 1.3$ . However, numerical integration of Eq. (3.10) is required to define the TVP for each radial ordinate where  $r/r_c < 1.3$ . Consequently, use of the Sullivan TVP is cumbersome and time consuming if the vortex tangential velocity need be known at many radial ordinates.

The Scully (1975) – Kaufmann (1962) (S-K) TVP defined in Eq. (3.11) is used extensively to define rotor tip vortices ((Vatisstas, 2006), (Thom and Duraisamy, 2010), and (Liu et al., 2012)). Bhagwat and Leishman (2002) demonstrate that the S-K profile realistically represents measured vortex TVPs; Vatisstas (2006) reports that it is especially realistic for radial ordinates  $r/r_c > 3$ . Aboelkassem et al. (2005) note that the S-K TVP typically under-predicts the maximum vortex tangential velocity near the critical radius. One noted advantage that the S-K profile holds over the L-O profile is that it is defined at the vortex center.

$$V_{\theta,s-k}(r) = \left( \frac{r \cdot r_c}{r^2 + r_c^2} \right) \quad (3.11)$$

For the sake of completeness, the H-family of profiles developed by G.J. Holland between 1980 and 2010 shall be briefly discussed. The first and latest versions, referred to as H80 and H10 respectively, are the most commonly-used TVPs for modeling hurricane vortex wind fields (Kepert, 2010). However, the H-family TVPs require an excessive number of parameters (Fluid density, fluid temperature, assumed wind speeds at specified heights, etc.) along with calibration of various constants based upon “peripheral observations” (Holland et al., 2010). Forecasting applications may require such an extensively detailed profile, as the trajectory of a hurricane may be greatly influenced by its TVP (Cao et al., 2011). Hurricanes are often tracked for weeks before they make landfall, hence it is feasible to collect the large number of parameters and update models. However, other vortices (tornado, rotor-tip, etc.) are short-lived and volatile, so readily collecting and updating the many parameters is not feasible. Consequently, application of the H-family of profiles is limited to the study of hurricanes.

### 3.1.3 Algebraic Profiles

The term “algebraic profiles” refers to TVPs that are developed to replicate the commonly-used, derived profiles that have been discussed up to this point. The algebraic profiles are advantageous for use in computer modeling because they can be used to study vortices having numerous TVPs without recursive modification of the program. Furthermore, the algebraic profiles afford a much more time-efficient manner to compute profiles such as the Sullivan profile (Defined in Eqs. (3.7) to (3.10)).

Vatistas et al. (1991) introduce the n-family of TVPs defined in Eq. (3.12). The exponent (n) is varied to accurately reproduce the most commonly-used, derived TVPs from **Section 3.1.1**: S-K (n = 1), steady L-O (n = 2), and RCVM (n = 100). Vatistas (1998) modifies Eq. (3.12) to

replicate the two-celled Sullivan vortex. However, the definition of this new TVP cannot be evaluated directly, hence it difficult and cumbersome to use.

$$V_{\theta,v}(r) = \frac{(r \cdot r_c)}{(r^{2n} + r_c^{2n})^{1/n}} \quad (3.12)$$

Wood and White (2011) begin with Vatistas' (1991) profile, and develop possibly the most robust algebraic profile. Eq. (3.13) approximates both single- and two-celled vortex TVPs with great accuracy. Three exponents are used to modify the profile:  $\kappa$  controls the slop of the profile in the inner core ( $r \leq r_c$ ),  $\eta$  controls the decay rate of the free vortex ( $r > r_c$ ), and  $\lambda$  controls the profile shape. Values of the three exponents that reproduce the common, derived TVPs are summarized in Table 3.2. Wood and White (2011) report that these exponents replicate the L-O and Sullivan TVPs with RMS error of only 0.008 and 0.005 respectively, demonstrating the accuracy of these approximation.

$$V_{\theta,w-w}(r) = \frac{(r/r_c)^\kappa}{\left[1 + \frac{\kappa}{\eta} \cdot ((r/r_c)^{\kappa/\lambda} - 1)\right]} \quad (3.13)$$

**Table 3.2:** Exponents used to replicate derived TVPs using Eq. (3.13).

Model	$\kappa$	$\eta$	$\lambda$
Scully	0.850	1.700	0.700
L-O	1.000	2.265	0.830
Sullivan	2.401	3.433	0.435
RCVM	1.000	2.000	0.010

### 3.1.4 Normalization and Comparison of Analytical Profiles

For comparison, all vortex parameters are normalized with respect to the maximum vortex tangential velocity and the vortex critical radius so that  $V_\theta(r_c) = V_{\theta,\max} = 1$ . The Rankine TVPs Eqs. (3.1-4) and the W-W TVP Eq. (3.13) are already normalized; however, the L-O, S-K, and Vatistas profiles need be normalized. Eqs. (3.5), (3.11), and (3.12) are evaluated at  $r = r_c$ , yielding Eqs. (3.14-16) respectively; the steady version of Eq. (3.5), where  $r_c$  is fixed, is used.

$$V_{\theta,L-O}(r_c) = (1 - \exp(-\alpha_o)) \quad (3.14)$$

$$V_{\theta,S-K}(r_c) = 0.5 \quad (3.15)$$

$$V_{\theta,V}(r_c) = 2^{-1/n} \quad (3.16)$$

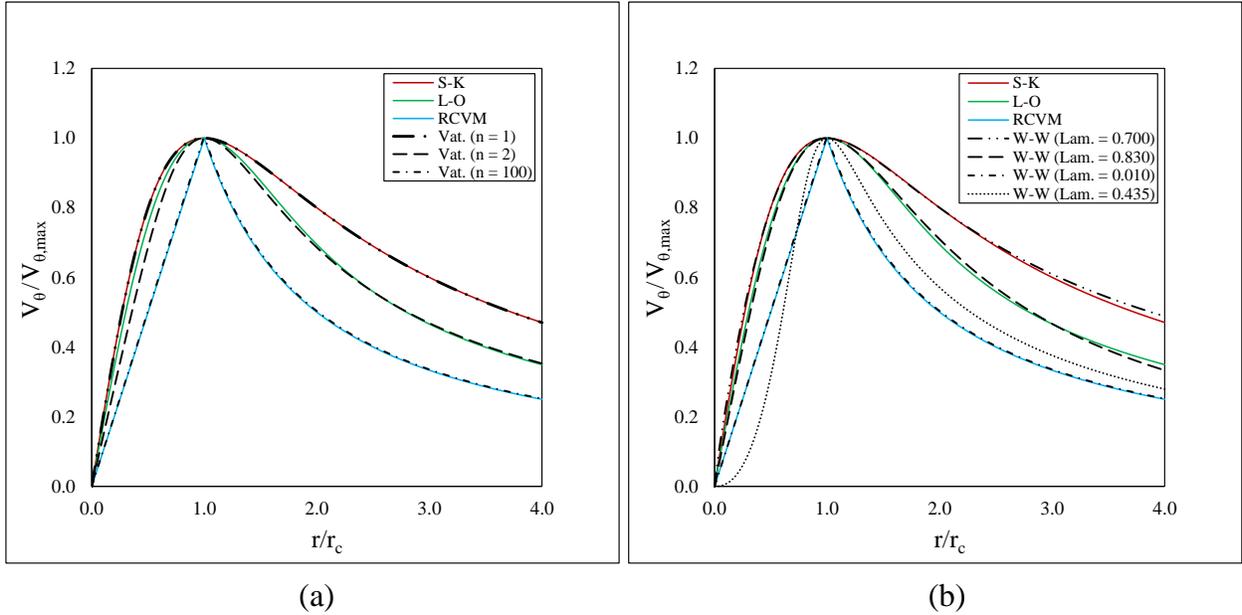
Now the TVPs defined in Eqs. (3.5), (3.11), and (3.12) are divided by their respective limits given in Eqs. (3.14-3.16). The normalized TVPs are listed in Eqs. (3.17-3.19) respectively.

$$V_{\theta,L-O}^*(r) = \frac{1}{(1 - \exp(-\alpha_o))} \cdot \frac{r_c}{r} \cdot \left[ 1 - \exp\left(-\alpha_o \cdot \frac{r^2}{r_c^2}\right) \right] \quad (3.17)$$

$$V_{\theta,S-K}^*(r) = 2 \cdot \left( \frac{r \cdot r_c}{r^2 + r_c^2} \right) \quad (3.18)$$

$$V_{\theta,V}^*(r) = r \cdot r_c \cdot \left[ \frac{2}{r_c^{2n} + r^{2n}} \right]^{1/n} \quad (3.19)$$

Figure 3.1a and Figure 3.1b respectively illustrate qualitative comparison of normalized adaptations of the S-K, L-O, and RCVM TVPs with their replications by the normalized Vatisas profile and W-W profile. Vatisas' TVP is more accurate for  $r/r_c > 1$ , while the W-W TVP is



**Figure 3.1:** Comparison of normalized S-K, L-O, and RCVM vortex TVPs with their replications by the normalized (a) Vatisas and (b) W-W profiles.

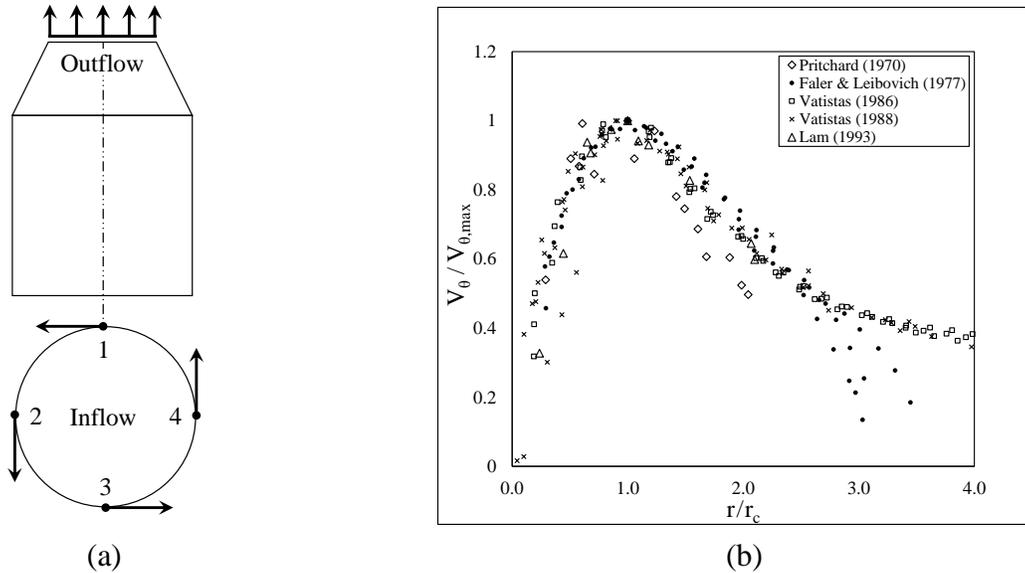
## 3.2 Measured Tangential Velocity Profiles

The literature contains numerous measured vortex TVPs. These range from small, mechanically-produced and maintained experimental vortices to large, convection-driven, atmospheric vortices such as tornados and hurricanes. The measured TVPs are assembled and grouped into six categories based upon vortex type: vortex chamber experiments, tornado simulation experiments, fixed-wing experiments, rotor-experiments, measured tornado, and measured hurricane. Details of the vortices in each of the six groups as well as the measurement methodology used to record the TVP are discussed. All measured TVPs are reported in the same normalized format  $V_{\theta}(r_c) = V_{\theta, \max} = 1$  as the analytical TVPs for ease of comparison.

### 3.2.1 Vortex Chamber Experiments

Vortex chambers are common experiment configurations for studying flows in vortex combustors and separators ((Vatistas et al., 1986), (Vatistas and Lin, 1988), and (Lam, 1993)). Figure 3.2a is a general schematic of a vortex chamber. Specifically, fluid is input at one end of the cylindrical chamber as four tangential jets spaced at  $\pi/2$  around the circumference of the chamber and extracted as a single axial stream at the opposite end of the chamber. Vatistas et al. (1986) provide a thorough description of the flow within vortex chambers.

Faler and Leibovich (1977) report that their experiments are characterized by the Reynolds number range of  $3000 \leq Re \leq 6,000$ ; however, they do not discuss how they calculate  $Re$ . The remaining four sources do not discuss  $Re$  for their experiments. Pritchard (1970) stirs a vat of water, and uses “streak photographs” to deduce the TVP. Faler and Leibovich (1977) use a vortex chamber with water as fluid, and measure TVPs via laser Doppler velocimetry. Vatistas et al. (1986) and Vatistas and Lin (1988) use a vortex chamber with air as fluid, measure pressure



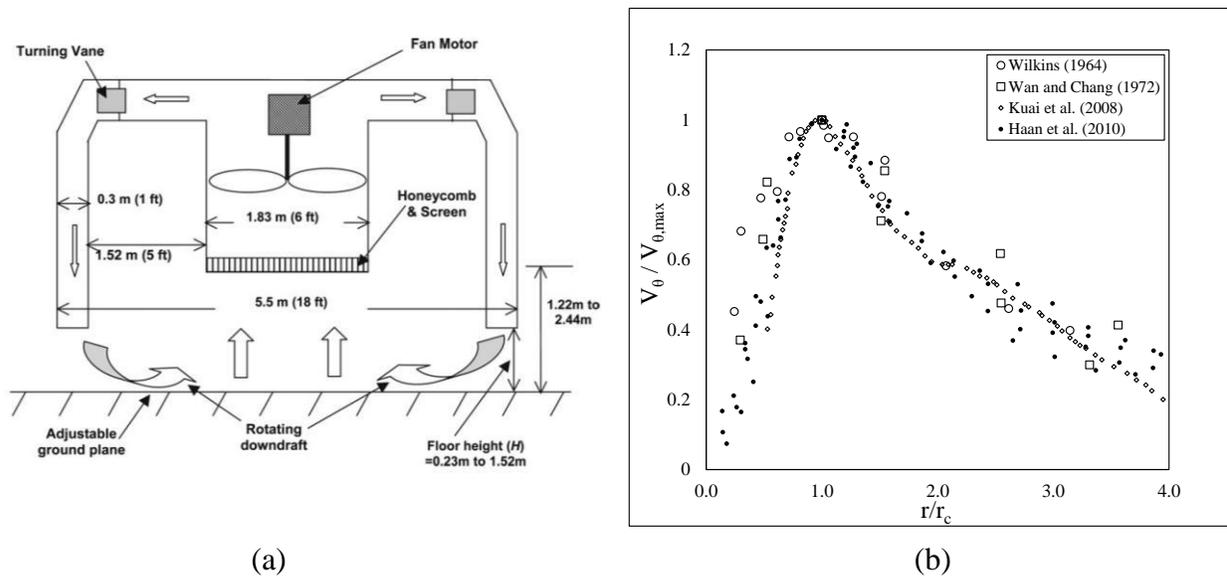
**Figure 3.2:** (a) Schematic of the vortex chamber experiment configuration and (b) vortex TVPs from vortex chamber experiments.

using a three-dimensional probe, and compute the TVP based upon the pressure profile. Lam (1993) also uses air in a vortex chamber, measures pressure using a 5-hole pitot tube, and back-computes the TVP.

The measured TVPs are summarized in Figure 3.2b; all profiles are qualitatively similar. In the range  $r/r_c < 2$ , Pritchard's (1970) data exhibits greatest deviation from the other data. This is likely because his experiment is less controlled than the vortex chambers used by all of the other authors. Also, his method of measuring the TVP via streak photographs is less accurate than directly measuring velocity or measuring pressure and back-computing velocity. Faler and Leibovich's (1977) data separates from the three remaining data sets and exhibits unrealistically-rapid decay for  $r/r_c > 2.5$ . It is therefore postulated that the measurements were taken too close to the walls of the vortex chamber, hence the TVP was artificially damped by the presence of the wall.

### 3.2.2 Tornado Simulator Experiments

Tornado simulators are used to study both the general structure of tornados (Church et al., 1979) and the loading they produce on structures (Haan et al., 2010). Figure 3.3a is a schematic of the state of the art, translating tornado simulator currently in use at Iowa State University. Haan et al. (2008) discuss the technical parameters and specifications of the tornado simulator. Church and Snow (1993) provide a thorough review of early tornado simulators and studies. Generally speaking, a powerful blower or fan is mounted at the top of a cylindrical or dome-shaped hood, and fluid is pulled into the hood through numerous angled vanes spaced around the hood's circumference. The hood may be stationary ((Wilkins, 1964) and (Wan and Chang, 1972)) or may translate ((Kuai et al., 2008) and (Haan et al., 2010)).



**Figure 3.3:** (a) Schematic of the translating tornado simulator at Iowa State University (Modified from Haan et al. (2010)) and (b) vortex TVPs from tornado simulator experiments.

The simulated tornado vortices are characterized using the vortex Reynolds number  $Re_v = \Gamma_\infty / \nu$ , where total circulation  $\Gamma_\infty = V_{\theta, \max} \cdot r_c \cdot 2\pi$  is the path integral of the maximum tangential velocity. Some studies choose to report the swirl ratio (S), which is the ratio of tangential to axial vortex velocity, instead of  $\Gamma_\infty$ . Given the volumetric flow rate through the fan (Q), the total

circulation is defined  $\Gamma_\infty = 2 \cdot S \cdot Q \cdot r_c^{-1}$ . The ranges of  $Re_v$  for the considered studies are summarized in Table 3.3.

**Table 3.3:** Vortex Reynolds number ranges for tornado simulator TVPs.

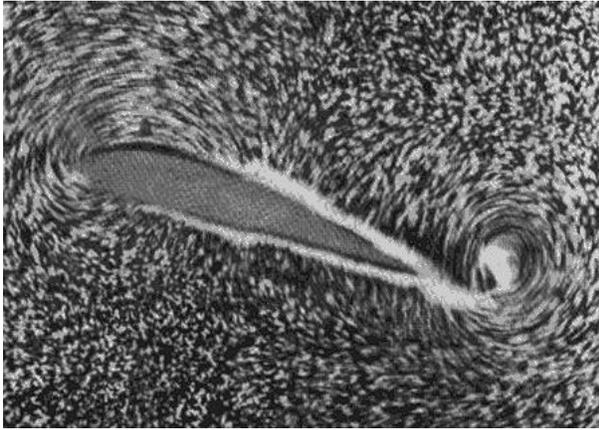
Source	$Re_v$
Wilkins (1964)	205,000
Wan and Chang (1972)	710,000 to 1,300,000
Kuai et al. (2008)	1,798,000 to 2,062,000
Haan et al. (2010)	1,800,000 to 4,165,000

Wilkins (1964) uses a small cup anemometer to directly measure velocity; he also attains the same TVP by computing the velocity profile from a pressure profile measured using a pitot tube. Wan and Chang (1972) measure the TVP directly using a constant temperature anemometer with hot film probes as sensors. Kuai et al. (2008) and Haan et al. (2010) use the same 18-point pressure probe to measure the vortex pressure profile, from which, the vortex TVP is back-computed.

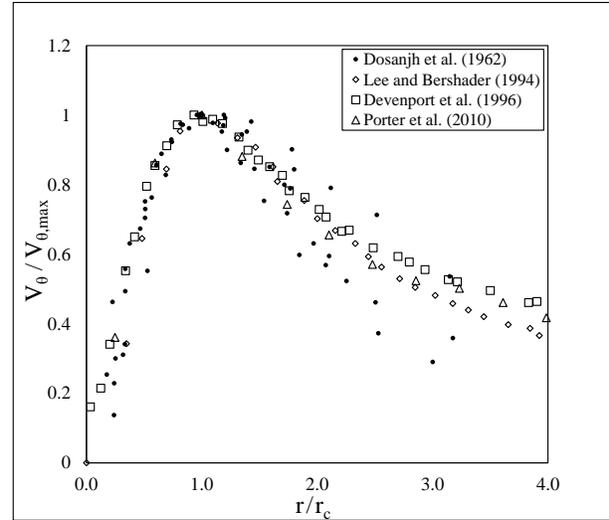
Measured TVPs from the tornado simulator experiments are summarized in Figure 3.3b; all profiles are qualitatively similar. The tornado simulator TVPs are visibly sharper than the vortex chamber TVPs (Figure 3.2b). This is counter-intuitive, as the literature currently theorizes that the TVP should flatten as  $Re_v$  increases due to increased turbulence in the potential vortex (Vatistas, 2006).

### 3.2.3 Fixed-Wing Experiments

Vortices produced by fixed wings are typically studied in the aerospace community where the objective is to evaluate air-loads produced on trailing aerospace vehicles when they encounter vortices shed from leading aerospace vehicles (Dosanjh et al., 1962). Figure 3.4 illustrates the shedding of a vortex from the downstream edge of a fixed wing. Generally speaking, a wing is rigidly fixed in a tank and fluid is circulated over it.



(a)



(b)

**Figure 3.4:** (a) Prandtl’s fixed-wing experiment (Modified from Barba (2013)) and (b) vortex TVPs from fixed-wing experiments.

Fixed-wing experiments are classified using the chord Reynolds number  $Re_c = c \cdot U_\infty / \nu$ .

The range of  $Re_c$  for the considered studies, all of which use air as fluid, is summarized in **Table 3.4**. Dosanjh et al. (1962) fix the wing in a wind tunnel, measure the vortex pressure profile, and compute the TVP. Lee and Bershader (1994) fix the wing in a shock tube and also measure the vortex pressure profile then back-compute the TVP. Devenport et al. (1996) fix the wing in a wind tunnel and use a four-sensor, hot-wire probe to directly measure the vortex TVP. Porter et al. (2010) also fix the wing in a wind tunnel and use hot wire anemometry to directly measure the vortex TVP.

**Table 3.4:** Chord Reynolds number range for fixed wing vortex experiments.

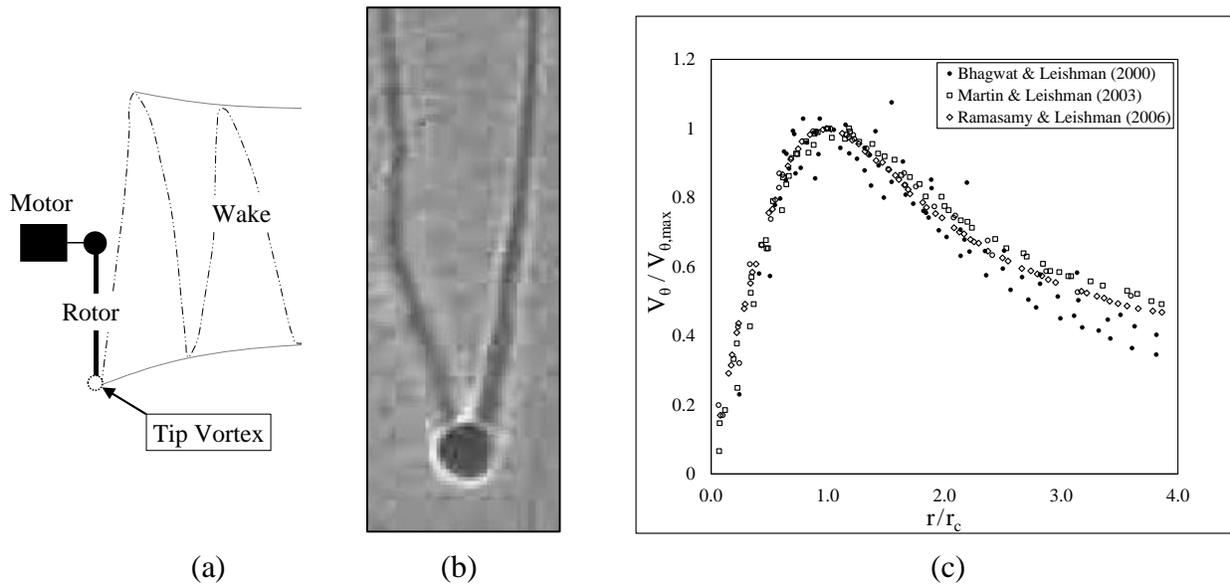
Source	$Re_c$
Dosanjh et al. (1962)	10,000
Lee and Bershader (1994)	900,000 to 1,300,000
Devenport et al. (1996)	318,000 to 742,000
Porter et al. (2010)	830,000

The fixed-wing vortex TVPs are summarized in Figure 3.4b; all profiles are qualitatively similar and generally well grouped. The data of Dosanjh et al. (1962) deviates from the other

three tightly-grouped data sets for  $r/r_c > 1.5$ . This deviation is attributed to the fact that Dosanjh et al. (1962) use a much lower  $Re_c$  than is used in the other three studies. It is also noted that the fixed-wing vortex TVPs resemble the vortex chamber TVPs (Figure 3.2b) rather than the tornado simulator TVPs (Figure 3.3b).

### 3.2.4 Rotor Experiments

Rotor vortices are primarily studied in the aerospace community for mitigation of the impulsive noise and vibration of helicopters (Ramasamy and Leishman, 2004). The schematic Figure 3.5a illustrates the layout of the common single-rotor experiment. Figure 3.5b is a shadowgraph (density gradient measurement) illustrating the scale of a rotor-tip vortex to the propeller that produces it. Generally stated, the rotor is driven to rotate by the motor and different techniques are used to measure the vortices developed on the rotor and shed in its wake.



**Figure 3.5:** (a) Schematic of the rotor tip vortex experiment, (b) shadowgraph of a rotor tip vortex (Modified from Bagai and Leishman (1993)), and (c) measured vortex TVPs from rotor tip experiments.

**Table 3.5:** Chord Reynolds number range for rotor vortex experiments.

Source	$Re_c$
Bhagwat and Leishman (2000)	270,000
Martin and Leishman (2003)	272,000
Ramasamy and Leishman (2006)	272,000

Bhagwat and Leishman (2000) seed the air with olive oil having particle diameter  $d_p = 6 \pm 2 \mu\text{m}$  and use laser Doppler velocimetry to directly measure the vortex TVP. Martin and Leishman (2003) also use laser Doppler velocimetry to directly measure the TVP, but they use a “mineral oil fog” with particle diameter  $d_p < 0.5 \mu\text{m}$  to seed the air, which improves the resolution of their measurements. Ramasamy and Leishman (2006) also measure the TVP directly using laser Doppler velocimetry with mineral oil fog as seed particles.

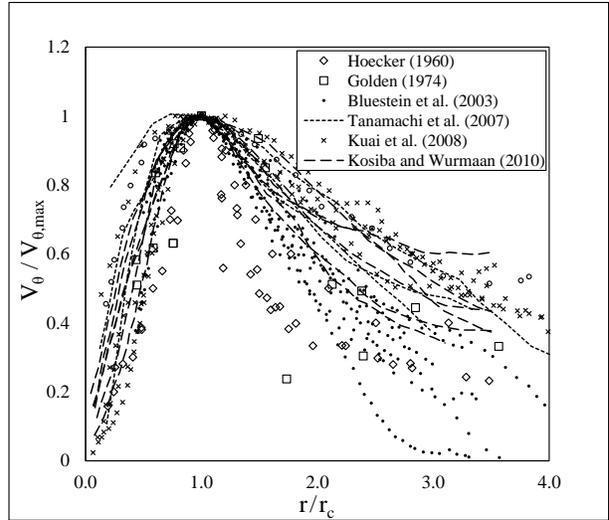
Measured TVPs from the rotor tip vortex experiments are summarized in Figure 3.5c; the TVPs are qualitatively similar and well-grouped. Bhagwat and Leishman’s (2000) data exhibits more scatter than the other two data sets. This is likely due to tracking error (discussed in Martin and Leishman (2003)) because the olive oil seeding particles are much larger than the mineral oil seeding particles. It is noted that the measured rotor-tip TVPs are similar to the vortex chamber and fixed-wing TVPs rather than exhibiting the sharp profile observed in the tornado simulator TVPs.

### 3.2.5 Measured Tornadoes

Tornado vortices are studied primarily so that the wind loadings that they place on structures may be better understood. The current state of the art in measurement of tornado TVPs is via mobile Doppler radar as illustrated in Figure 3.6a. Generally stated, those working in the field attempt to forecast the tornado path and position themselves so that they can record the tornado from the closest safe distance.



(a)



(b)

**Figure 3.6:** (a) Measurement of tornado vortex TVP via mobile Doppler radar (Modified from NSF (2005)) and (b) measured tornado vortex TVPs.

Hoecker (1960) takes successive, timed photographs of debris and using the change in debris location to approximate the tornado-vortex TVP. Golden (1974) uses similar methodology to approximate the TVP within waterspouts.

The four remaining studies use mobile, W-band Doppler radar ( $\lambda = 3$  mm,  $f = 95$  Hz) to measure tornado TVPs. Bluestein et al. (2003) and Tanamachi et al. (2007) use measurements made by their group in 1999. Kuai et al. (2008) analyze measurements recorded in 1998 and reported in Alexander and Wurman (2005). Kosiba and Wurman (2010) also analyze measurements recorded in 1998. Summary of measurement details is provided in Table 3.6. All measurements were made using similar radar technology and at similar measurement distances.

**Table 3.6:** Details for mobile Doppler radar measurement of tornado TVPs.

Source	Measurement Year	Measurement Height	Measurement Distance
Bluestein et al. (2003)	1999	N/A	2.3 to 7.0 km
Tanamachi et al. (2007)	1999	70 to 155 m	4.5 to 6.8 km
Kuai et al. (2008)	1998	20 to 660 m	1.7 to 12.9 km
Kosiba and Wurman (2010)	1998	$\approx 40$ m	1.7 to 6.5 km

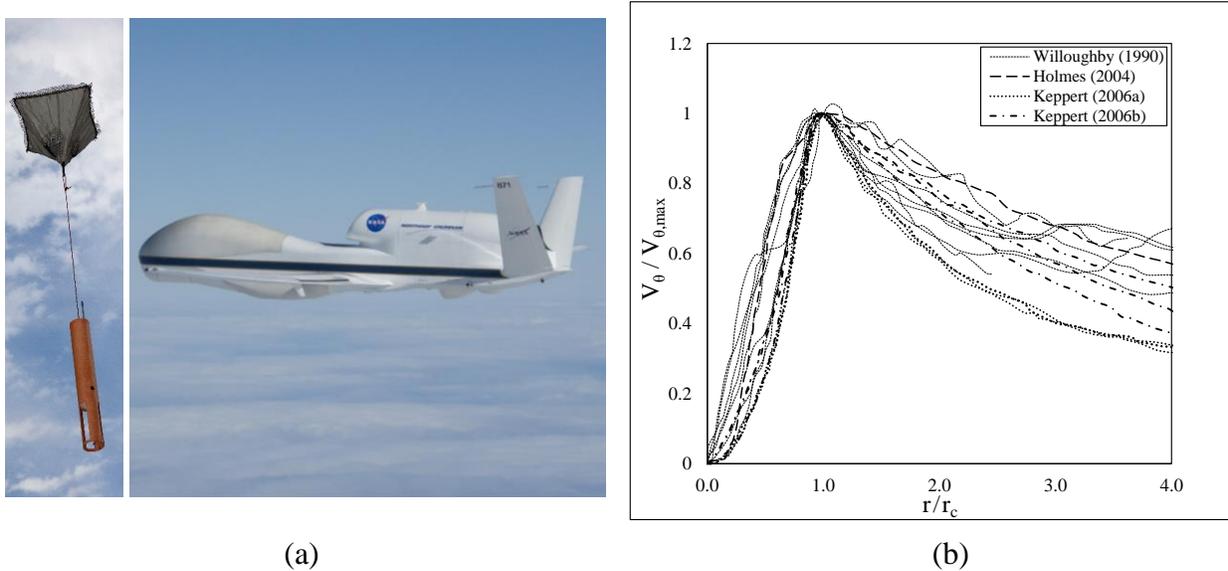
Figure 3.6b summarizes the measured tornado-vortex TVPs. The data are qualitatively similar for  $r/r_c \leq 1$ ; however, there is substantial scatter in the data for  $r/r_c > 1$ . The data acquisition methodology of Hoecker (1960) and Golden (1974) is imprecise and at best yields an estimate of the TVP. Now considering the radar data, several of the series reported in Bluestein et al. (2003) exhibit abnormally-rapid decay for  $r/r_c > 1.5$ . The accuracy of radar measurements depend upon the strength of the reflected signal, which can be attenuated and distorted by numerous sources (moisture in the atmosphere, level of precipitation, debris in the vortex, etc.), hence it is believed that Bluestein et al.'s results do not accurately represent the TVP for  $r/r_c > 1.5$ .

### 3.2.6 Measured Hurricanes

Hurricane vortices are primarily studied to allow better forecasting of their trajectory and strength at landfall. Early studies report measurements collected via manned flight through the hurricane eyewall, which is obviously hazardous to human life. Modern studies are conducted by flying above the tropical cyclone (NOAA uses the manned WP-3D Orion (NOAA, 2015b) and NASA uses the unmanned Global Hawk (Newman, 2013)) and seeding the hurricane with dropsondes (Discussed in Hock and Franklin, (1999)). Dropsondes transmit their location (via GPS), horizontal and vertical velocity, temperature, pressure, and relative humidity with high accuracy (EOL, 2015). Illustration of a deployed dropsonde and the Global Hawk delivery plane of NASA are provided in Figure 3.7a.

Willoughby (1990) and Holmes (2004) report hurricane TVP measurements made via manned aircraft flight through the hurricanes. Keppert (2006a and 2006b) report the measured TVP within hurricanes at 500, 1000, and 2000 meters elevation measured using dropsondes. The hurricane considered in Keppert (2006a) shows minimal variation in the TVP with elevation

while the TVP of the hurricane considered in Keppert (2006b) progressively flattens with increased elevation.



**Figure 3.7:** (a) Deployed dropsonde (Modified from UCAR (2012)) and unmanned Global Hawk (Gutro and Braun, 2012) and (b) measured hurricane TVPs.

The measured hurricane TVPs are summarized in Figure 3.7b. Generally, the data are well-grouped and follow a consistent trend. Simply due to the scale of the hurricane vortex, there is question regarding the resolution of the measurements. To put this in perspective, Keppert (2006a) uses 4 to 5 dropsondes to define the 25 km critical radius, a cluster of dropsondes are dropped at the critical radius, and another cluster are dropped at approximately 75 km from the center of the hurricane; the TVP is defined from this data. Keppert (2006b) uses more than twice as many dropsondes to define the TVP, hence he has much better resolution.

### 3.3 Comparison and Analysis of Analytical and Measured Profiles

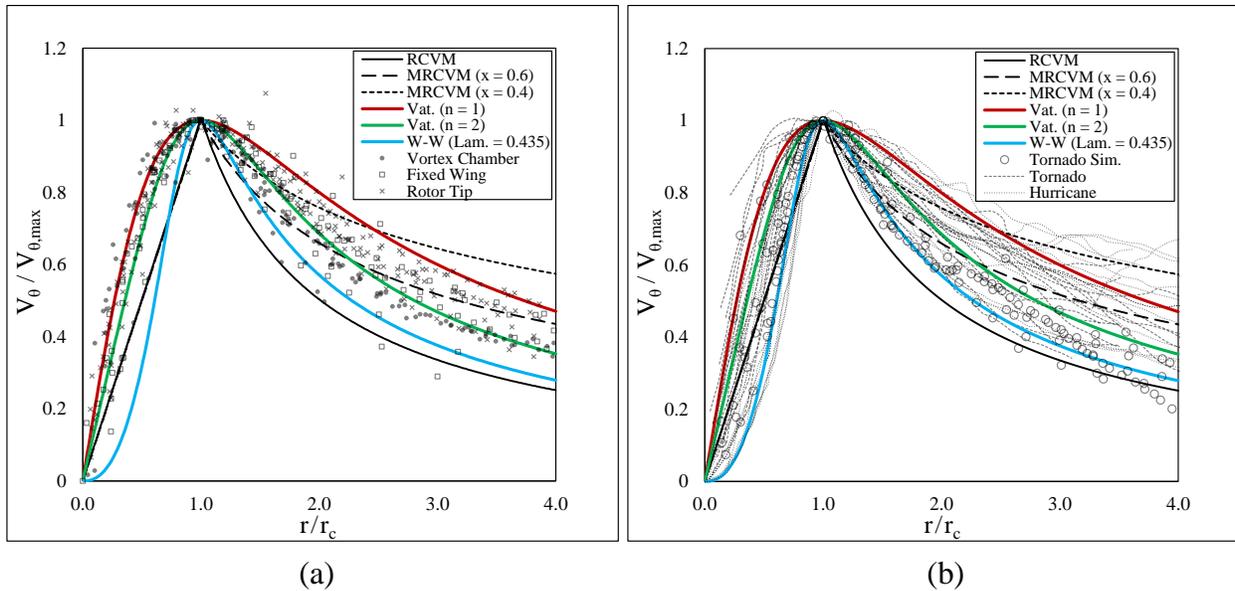
Six sources of measured vortex TVPs have been discussed, and measured TVPs from each source have been compiled and normalized. Now the measured TVPs shall be compared with the normalized analytical profiles defined in Section 3.1, so that the most realistic analytical TVPs can be identified. Section 3.2 identifies several measured TVPs that deviating substantially

from the other TVPs within their classification. These data sets are summarized below and omitted from comparison with the analytical TVPs for the noted reasons.

- From the vortex chamber experiments (Section 3.2.1):
  - Faler and Leibovich (1977)
    - The vortex TVP exhibits unrealistically-rapid decay for  $r/r_c > 2.5$ .
- From the measured tornados (Section 3.2.5):
  - Hoecker (1960) and Golden (1974)
    - The vortex TVPs are computed by tracking debris movements in successive, timed photographs. These measurements are qualitative and deviate substantially from the direct radar-measured TVPs for  $r/r_c > 1$ .
  - Bluestein et al. (2003)
    - Many of the vortex TVPs exhibit unrealistically-rapid decay for  $r/r_c > 1.5$ .

Six commonly-used, analytical TVPs are selected for comparison with the measured vortex TVPs. The selected analytical TVPs are: RCVM, MRCVM ( $x = 0.6$ ), MRCVM ( $x = 0.4$ ), Vatistas ( $n = 2$ ) which approximates the steady L-O profile, Vatistas ( $n = 1$ ) which approximates the S-K profile, and W-W ( $\lambda = 0.435$ ) which approximates the Sullivan profile. The six groups of measured TVPs are split into two groups of three to avoid excessive data overlap and compared with the analytical TVPs in Figures 3.8a and 3.8b.

Figure 3.8a shows that Vatistas  $n = 1$  and  $n = 2$  profiles are excellent representation of the inner core ( $r/r_c \leq 1$ ) of the measured vortex chamber, fixed-wing and rotor tip TVPs. The  $n = 1$  profile is an upper boundary to the measured TVPs beyond the critical radius ( $r/r_c > 1$ ). The  $n = 2$  profile falls from the middle of the measured TVPs at  $r/r_c = 1$  to be the lower boundary of the measured TVPs at  $r/r_c > 4$ .



**Figure 3.8:** Comparison of analytical TVPs with measured TVPs from (a) vortex chamber, fixed-wing, and rotor tip experiments and (b) tornado simulators, tornados, and hurricanes.

The Rankine and W-W profiles under-predict the tangential velocity within the critical radius.

Beyond the critical radius, the MRCVM ( $x = 0.6$ ) profile is a good representation of the measured TVPs. However, the RCVM and W-W profiles decay much more rapidly than the measured TVPs while the MRCVM ( $x = 0.4$ ) profile decays much more slowly than the measured TVPs.

Figure 3.8b shows that the W-W profile is an effective lower boundary for the measured tornado simulator, tornado, and hurricane TVPs. Vatisas  $n = 2$  profile consistently falls in the middle of the measured TVPs. Vatisas  $n = 1$  profile is near the top of the measured TVPs, but some of the measured hurricane TVPs fall above it. The MRCVM ( $x = 0.4$ ) is a better upper boundary for  $r/r_c > 2.5$ , but it is a poor fit of the measured TVPs for  $r/r_c < 1.25$ .

### 3.4 Conclusions

Physically-realistic analytical vortex tangential velocity profiles (TVPs) are required to simulate physically-realistic vortex-structure interaction. All measured vortex TVPs are qualitatively similar but differ quantitatively, hence no single analytical vortex TVP can

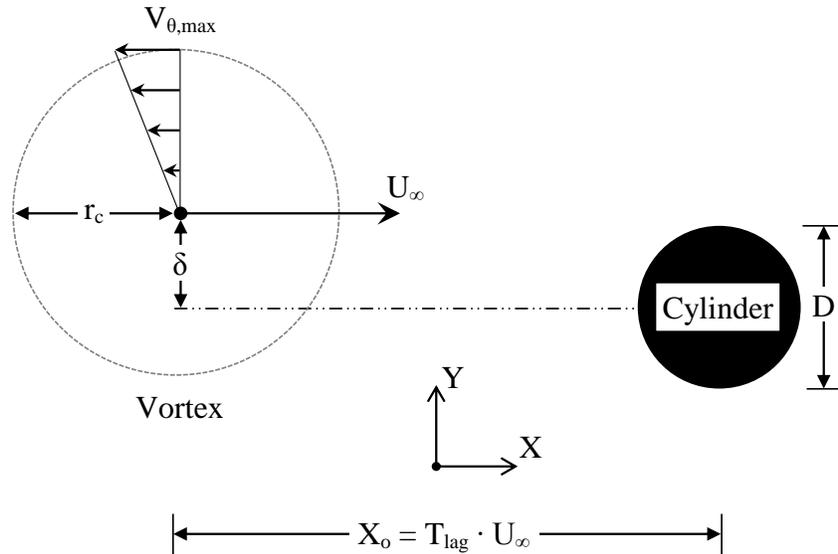
represent all measured vortex TVPs. Measured vortex TVPs are exhaustively surveyed in the literature and compiled. Analytical vortex TVPs are also compiled and normalized so that the maximum tangential velocity is unity at the critical vortex radius  $V_{\theta}(r_c) = 1$  for comparison with the measured vortex TVPs. A group of analytical vortex TVPs which adequately represent the spectrum of measured vortex TVPs are identified for use in computer simulation of vortex-structure interaction. Based upon the content presented in Chapter 3, the subsequent conclusions have been reached.

1. The Vatistas  $n = 1$  TVP (Marked by red line in Figure 3.8) which replicates the normalized S-K profile is the best upper boundary to most of the measured TVPs.
  - a. This implies the  $n = 1$  profile represents a “flat” and supposedly turbulent vortex.
2. The Vatistas  $n = 2$  TVP (Marked by green line in Figure 3.8) which replicates the normalized, steady L-O profile bisects most of the measured TVPs.
  - a. This implies the  $n = 2$  profile represents a “typical” vortex.
3. The W-W  $\lambda = 0.435$  TVP (Marked by blue line in Figure 3.8) which replicates the normalized Sullivan profile is the best lower boundary to the measured TVPs.
  - a. This implies the W-W profile represents a “sharp” and supposedly laminar vortex.
  - b. The W-W profile is an excellent lower boundary for the measured tornado simulator, tornado, and hurricane TVPs. However, when the measured vortex chamber, fixed-wing, and rotor tip TVPs are considered, the Rankine profiles are a better lower boundary for  $r/r_c < 1$ , and the W-W profile is a better lower boundary for  $r/r_c > 1$ .
4. The Vatistas  $n = 2$  TVP best represents a typical vortex, hence it should be used as the default vortex TVP for computer simulations of vortex-structure interaction.

## CHAPTER 4: COMPUTER MODEL

### 4.1 Problem Description

The physical system modeled in the present study is the parallel interaction of a vortex with a long, rigid, circular cylinder that is immersed in a free stream. As illustrated in Figure 4.1, the vortex is initialized at some horizontal ordinate ( $X_0$ ) to the left of the cylinder and travels within the free stream at bulk velocity ( $U_\infty$ ) to impact the cylinder. The cylinder is “long”, implying that flow around the ends of the cylinder is negligible, and hence the problem is two-dimensional. The vortex’s impact with the cylinder is defined as “parallel” because the vortex’s axis of rotation and the major cylinder axis are always parallel. The vortex and cylinder centers may be on the same horizontal line, or the vortex’s path may be shifted vertically by some distance ( $\delta$ ).



**Figure 4.1:** Schematic of the parallel interaction of an impinging vortex with a long, rigid, circular cylinder.

The vortex’s tangential velocity ( $V_\theta$ ) increases with radius ( $r$ ) from the center of vortex and reaches a maximum value ( $V_{\theta,max}$ ) at the critical radius ( $r_c$ ). The maximum tangential velocity  $V_{\theta,max}$  is the product of  $r_c$  and the dimensionless vortex angular velocity ( $\alpha$ ). The time

lag ( $T_{lag}$ ) is specified so that the vortex and cylinder centers fall on the same vertical line when dimensionless time ( $t^*$ ) is zero. The time lag  $T_{lag}$  is illustrated in Figure 4.1 by fixing the starting position  $X_o$  of the vortex.

#### 4.1.1 System Parameters

The fluid density ( $\rho = 1$ ), fluid kinematic viscosity ( $\nu = 1/150$ ), and cylinder diameter ( $D = 1$ ) are fixed throughout the study. The stream-wise (X-direction) drag force ( $F_d$ ) and cross-stream (Y-direction) lift force ( $F_l$ ) are computed by integrating the pressure and shear force acting on the cylinder surface. The drag and lift forces are then converted to the commonly-discussed drag ( $C_d = 2 \cdot F_d / \rho \cdot A \cdot U_{ref}^2$ ) and lift ( $C_l = 2 \cdot F_l / \rho \cdot A \cdot U_{ref}^2$ ) force coefficients, where  $A = D \cdot L$  is the reference area per unit cylinder length ( $L = 1$ ).

All simulations in the present study use maximum vortex tangential velocity and free stream velocity of unity ( $V_{\theta, max} = U_{\infty} = 1$ ). Consequently, the free stream Reynolds number  $Re = U_{\infty} \cdot D / \nu$  is 150 for all simulations. The vortex critical radius  $r_c$  ranges from  $1 \leq r_c / D \leq 100$ . Noting that  $V_{\theta, max} = \alpha \cdot r_c = 1$ , it follows that the vortex's dimensionless angular velocity is the inverse of the vortex critical radius ( $\alpha = r_c^{-1}$ ). The vortex impacts the cylinder directly ( $\delta = 0$ ) unless otherwise stated, but its path is also shifted so that it travels above or below the cylinder by path shift of  $-4 \leq \delta / r_c \leq 4$ .

#### 4.1.2 Low Reynolds Number Limitation

The simulations performed herein utilize a free stream Reynolds number of 150, which is quite low considering the fact that vortex interaction with rotors and impact of atmospheric vortices with structures are both characterized by high Reynolds numbers ( $Re > 10^7$ ). A low Reynolds number is utilized for two primary reasons which are identified below and expounded subsequently. Firstly, the numerical model directly simulates the vortex-structure interaction, and

grid resolution requirements increase rapidly with increasing simulation Reynolds number.

Secondly, the physical system modeled herein is the two-dimensional impact of a vortex with a slender, circular cylinder, and simulation at Reynolds numbers above 300 necessitates use of a three-dimensional model.

The presently-used computer model resolves the velocity field by solving conservation of mass and momentum at each time step. This approach is known as “Direct Simulation” because no terms are added to the momentum conservation equations to model turbulence. Direct simulation is more physically realistic than using turbulence models, but requires excessive grid refinement to resolve the boundary layer around a structure as well as the turbulence in its wake. When directly simulating flow over a cylindrical structure, the first radial node spacing is generally  $\Delta r_0 < 0.1 \cdot \text{Re}^{-0.5}$  ((Selvam and Paterson, 1993) and (Cao et al., 2010)). The boundary layer around the structure progressively becomes thinner as Re increases, hence the necessary grid refinement to resolve the boundary layer rapidly increases with Reynolds number. Illie (2009) reports that grid resolution requirements increase with Reynolds number according to  $\text{Re}^{9/4}$  and reasons that  $\text{Re} \approx 5,000$  is a practical limit for direct simulations.

Grid resolution requirements aside, the other major factor constraining the free stream Reynolds number to a low value is that the physical system modeled herein is two-dimensional. Free stream flow over a cylindrical structure produces opposite-signed vortices on the cylinder which are alternately shed. Williamson’s (1996) physical experiments show that the structure of these vortices is two-dimensional up to approximately Re of 190 but begins to transition to three dimensional at higher Reynolds numbers. The transition is not abrupt, and Williamson’s (1996) experimental data shows that vortex shedding can adequately be resolved using a two-dimensional model up to Re of 300 (See Figure 4.5b). The sum of the maximum vortex

tangential velocity and the free stream velocity used throughout this study is 2.0, hence all computer simulations are characterized by  $Re \leq 300$  based upon the maximum velocity in the domain.

## 4.2 Governing Equations

The simulation is governed by incompressible conservation of mass Eq. (4.1) and conservation of x- and y-direction momentum which are expressed in Eqs. (4.2a) and (4.2b) respectively.

$$\frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = 0 \quad (4.1)$$

$$\frac{\partial U}{\partial t} + U \frac{\partial U}{\partial x} + V \frac{\partial U}{\partial y} + \frac{1}{\rho} \frac{\partial P}{\partial x} - \frac{\mu}{\rho} \left[ \frac{\partial^2 U}{\partial x^2} + \frac{\partial^2 U}{\partial y^2} \right] = 0 \quad (4.2a)$$

$$\frac{\partial V}{\partial t} + U \frac{\partial V}{\partial x} + V \frac{\partial V}{\partial y} + \frac{1}{\rho} \frac{\partial P}{\partial y} - \frac{\mu}{\rho} \left[ \frac{\partial^2 V}{\partial x^2} + \frac{\partial^2 V}{\partial y^2} \right] = 0 \quad (4.2b)$$

Eqs. (4.1) and (4.2) are non-dimensionalized using the dimensionless groups summarized in Eq. (4.3), where the asterisk implies that a value is dimensionless.

$$U^* = \frac{U}{U_\infty} \quad V^* = \frac{V}{U_\infty} \quad x^* = \frac{x}{D} \quad y^* = \frac{y}{D} \quad t^* = \frac{t \cdot U_\infty}{D} \quad P^* = \frac{P}{\rho \cdot U_\infty^2} \quad Re = \frac{D \cdot U_\infty}{\nu} \quad (4.3)$$

Mass and momentum conservation are written in dimensionless format in Eqs. (4.4) and (4.5) respectively using the compact tensor notation. Subscript “i” is the index, and subscripts “i and j” are 1 for “x” or 2 for “y”. Subscript “t” corresponds to time, and a comma implies differentiation with respect to the following subscript.

$$U_{i,i}^* = 0 \quad (4.4)$$

$$U_{i,t}^* + U_j^* U_{i,j}^* + P_{,i}^* + Re^{-1} [U_{i,j}^* + U_{j,i}^*]_{,j} = 0 \quad (4.5)$$

The convection term  $U_j^* U_{i,j}^*$  in Eq. (4.5) is nonlinear, hence it is difficult to numerically approximate accurately. Selvam (1998) reviews techniques for numerical approximation of the

convection term and concludes that the balance tensor diffusion scheme (Dukowicz and Ramshaw, 1979) is the most accurate approach. Specifically,  $-\delta t^*/2 \cdot (U_j^* U_k^* U_{ij}^*)_{,k}$  is added to  $U_j^* U_{ij}^*$ , where  $\delta t^*$  is the solution time step. Selvam (1997a) provides further details regarding the implementation of the balance tensor diffusion scheme.

#### 4.2.1 Vortex Tangential Velocity Profile

The three most commonly-used, analytical vortex tangential velocity profiles (TVPs) are the Scully-Kaufmann (S-K), steady Lamb-Oseen (L-O), and the Rankine Combined (RCVM). Chapter 3 contains a thorough review and comparison of measured and analytical vortex TVPs. The S-K profile forms the upper boundary to most of the measured TVPs, hence it is a “flat”, theoretically turbulent vortex. The L-O profile falls within the middle of the measured TVPs making it a “typical” vortex. The Sullivan profile is named the best representative of the lower boundary of the measured TVPs, hence it is a “sharp” theoretically laminar vortex. The RCVM profile also realistically represents a “sharp” vortex profile and is utilized much more commonly in the literature. The present study utilizes the S-K, L-O, and RCVM TVPs in computer simulations.

As shall be discussed subsequently, the impinging vortex is introduced into the computational domain using a transient boundary condition that combines the free stream and vortex tangential velocity. It is therefore convenient to use the previously defined and normalized version of Vatistas (1991) TVP given by Eq. (4.6), because the exponent ( $n$ ) can be specified as 1, 2, or 100 to replicate the S-K, L-O, and RCVM profiles respectively. This eliminates the need to modify the computer program when simulating different vortex profiles.

$$V_{\theta}(r) = r \cdot r_c \cdot \left[ \frac{2}{r_c^{2n} + r^{2n}} \right]^{1/n} \quad (4.6)$$

Eq. (4.6) is currently formatted so that  $V_{\theta,\max}$  is unity. Although all simulations in the present study, use vortices having maximum tangential velocity of unity, it is desired that the maximum tangential vortex velocity be specifiable. Therefore, Eq. (4.6) is scaled by the dimensionless vortex angular velocity  $\alpha$  and critical radius  $r_c$  so that  $V_{\theta,\max}$  of  $\alpha \cdot r_c$  can be specified. The resulting expression has  $r_c^2$  in the numerator; this term is distributed to reduce the number of necessary computations when computing  $V_\theta$ , resulting in Eq. (4.7) which is the TVP definition used in the present model. The exponent  $n = 2$ , corresponding to the steady L-O profile, is used for all simulations unless otherwise noted.

$$V_\theta(r) = \alpha \cdot r \cdot \left[ \frac{2}{(r/r_c)^{2n} + 1} \right]^{1/n} \quad (4.7)$$

### 4.3 Numerical Method

The present numerical model is a finite element adaption of the node-centered, non-staggered, control volume procedure introduced in Selvam (1997a). Time advancement is performed using the Backward Euler scheme. The velocity and pressure fields are resolved at each time step using the four-step advancement procedure outlined below. Steps 1 and 3 use preconditioned conjugate gradient solvers where Jacobi and Incomplete Choleski preconditioners are used respectively. Steps 1-4 are iterated until the average residual at each node falls below  $10^{-7}$  at each time step.

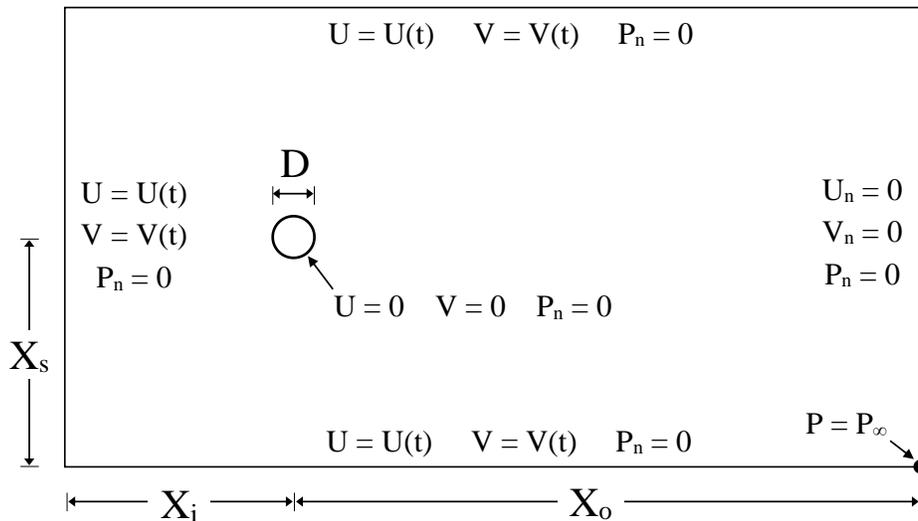
- Step 1 - Solve  $U_i^*$  from Eq. (4.5)
- Step 2 - Update velocity field:  $U_i^{**} = U_i^* + \delta t^* \cdot P_i^*$
- Step 3 - Solve for pressure field:  $(P_i^*)_i = U_{i,i}^{**} / \delta t^*$
- Step 4 - Correct velocity for incompressibility:  $U_i^* = U_i^{**} - \delta t^* \cdot P_i^*$

Equal-order interpolation of the velocity and pressure terms, along with Step 2, eliminates the checkerboard pressure field discussed in Patankar (1980). The higher order portion of the convection term and the diffusion term are solved implicitly, removing the restriction on maximum time step for numerical stability. A constant time step of  $\delta t^* = 0.01$  is used for all simulations so that the Courant–Friedrichs–Lewy (CFL) number is always less than one. Selvam (1997b) provides further discussion of the finite element methodology and solution algorithm.

#### 4.4 Domain, Grid, and Boundary Conditions

##### 4.4.1 Domain and Grid

The computational domain is illustrated by the schematic in Figure 4.2 where  $X_i$ ,  $X_o$ , and  $X_s$  are the inflow, outflow and span-wise boundary dimensions respectively. Two grids are used in the present study. Grid 1 (G1) is smaller, more refined, and used exclusively in Chapter 5. Grid 2 (G2) is larger, less refined, and used for the remaining studies reported in Chapters 6 and 7. Summary of the domain boundary dimensions for Grids 1 and 2 is provided in Table 4.1.

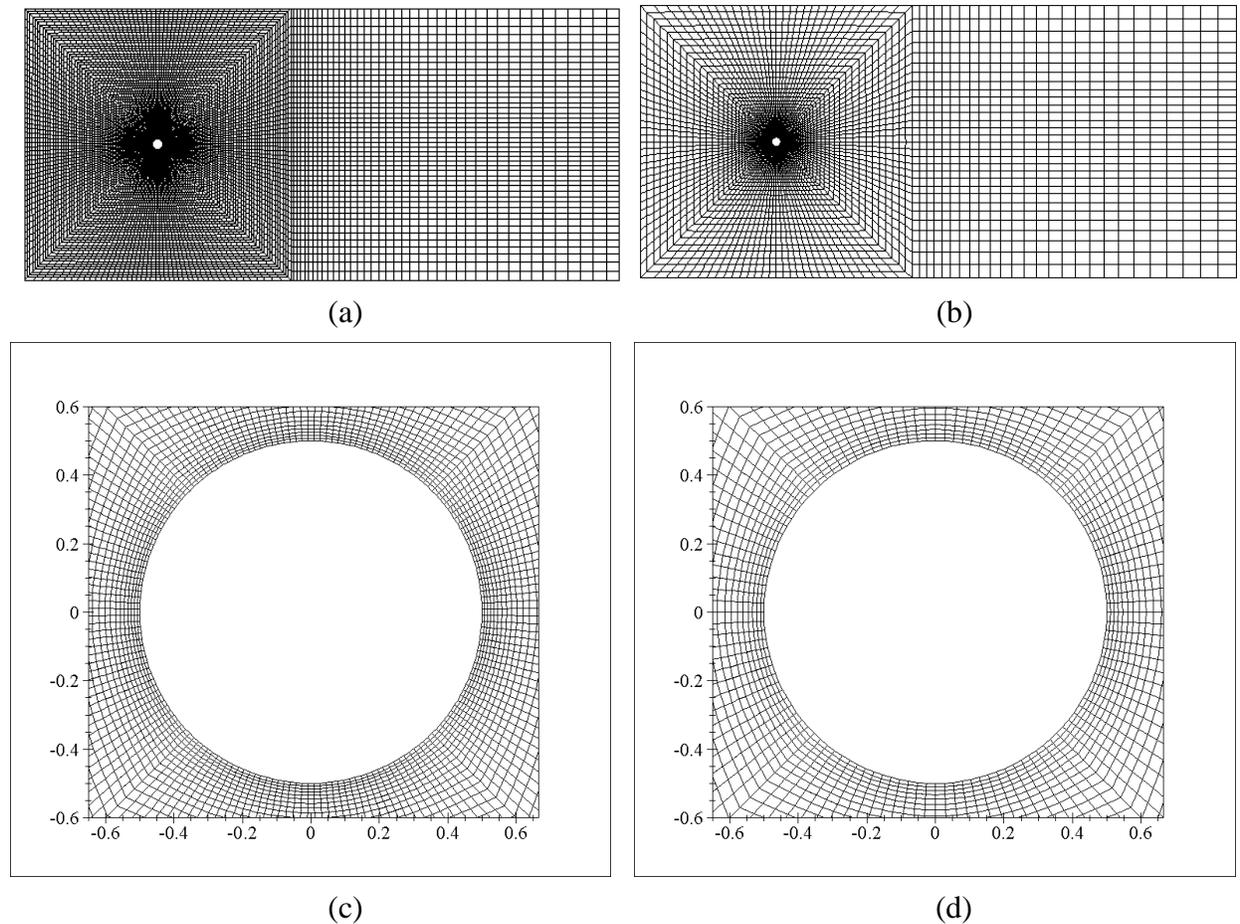


**Figure 4.2:** Schematic of the computational domain and boundary conditions.

**Table 4.1:** Summary of domain dimensions for Grid 1 and Grid 2.

Grid	$X_i$	$X_o$	$X_s$
1	$12.5 \cdot D$	$42.5 \cdot D$	$25.0 \cdot D$
2	$15.0 \cdot D$	$50.0 \cdot D$	$30.0 \cdot D$

Figures 4.3a and 4.3b respectively illustrate Grid 1 and Grid 2, and Figures 4.3c and 4.3d respectively illustrate close-up views of the grid refinement around the cylinder. The first radial node spacing from the cylinder surface is  $\Delta r_o = 0.0075 \cdot D$ . Subsequent node spacing is stretched according to  $\Delta r_i = 1.10 \cdot \Delta r_{i-1}$  until the specified maximum radial node spacing  $\Delta r_m$  is



**Figure 4.3:** Illustrations of (a) Grid 1 and (b) Grid 2 along with close-up views of grid refinement around the cylinder for (c) Grid 1 and (d) Grid 2.

reached ( $\Delta r_i \leq \Delta r_m$ ). After reaching the maximum radial node spacing, constant radial node spacing of  $\Delta r_m$  is used for the remainder of the computational domain. The maximum radial node

spacing  $\Delta r_m$  is  $0.25 \cdot D$  for Grid 1 and  $0.5 \cdot D$  for Grid 2. The surface of the cylinder is defined using 181 nodes for Grid 1 and 121 nodes for Grid 2; the tangential node spacing is  $2^\circ$  and  $3^\circ$  for the respective grids. Grids 1 and 2 are respectively composed of 15,700 and 7,526 nodes.

The primary reason for utilizing the less-refined Grid 2 is that it requires much less CPU time to execute a simulation than is required by Grid 1. Grid 1 is composed of approximately twice as many grid points as Grid 2; however a simulation of length  $t^* = 250$  requires 11.5 hours to execute on a personal computer when Grid 1 is used as opposed to 3 hours when Grid 2 is used. This correlates to an increase in computational time of 3.83 for an increase in nodes of only 2.09 times. **Table 4.2** summarizes the grid, number of simulations, and simulation time required

**Table 4.2:** Summary of computer simulations and associated CPU time for the simulations reported in this thesis.

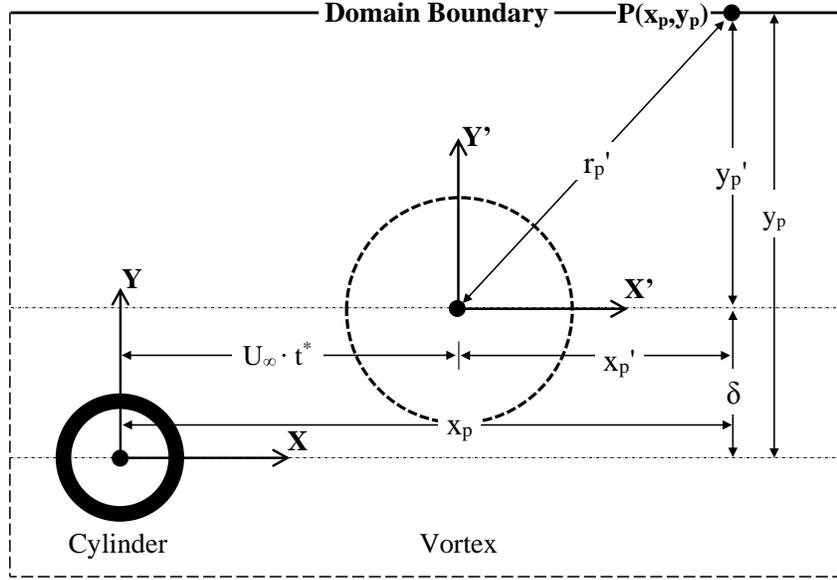
Grid	Simulations	Duration ( $\Delta t^*$ )	CPU Time	
			Hours	Days
Free Stream Validation Studies (Chapter 4.5)				
1	18	300	248.4	10.4
2	18	300	64.8	2.7
Load Variation with Vortex Impact Time (Chapter 5.2)				
1	76	250	874.0	36.4
Influence of Vortex Size (Chapter 5.3)				
2	15	200	36.0	1.5
...	30	300	108.0	4.5
...	5	450	27.0	1.1
...	10	600	72.0	3.0
...	20	3000	720.0	30.0
...	10	4000	480.0	20.0
...	5	6000	360.0	15.0
Influence of Vortex Path Shift (Chapter 5.4)				
2	23	3000	828.0	34.5
Defining the Vortex Loading Period (Chapter 6)				
2	1	200	12	0.5
...	2	400		
...	15	2100	378.0	15.8
<b>Totals</b>	248		4208.2	175.3

for the simulations reported in this thesis. Utilizing Grids 1 and 2, the required simulation time is 175.3. If the same simulations were conducted only using Grid 1, the required simulation time would be increased to 503.3 days, or by a factor of 2.87. This demonstrates the time savings benefit of designing grids tailored to specific studies.

#### 4.2.2 Boundary Conditions

Boundary conditions are indicated along each surface of the computational domain schematic Figure 4.2, where the subscript (n) implies differentiation with respect to the normal direction. The no-slip or zero velocity condition is applied at the cylinder surface. The downstream boundary is assumed to be sufficiently far from the cylinder for flow to re-develop, hence the normal derivative of velocity is zero. The normal derivative of pressure at the cylinder surface and domain boundaries is zero. The pressure value at the lower-downstream corner of the domain is specified to be the ambient pressure ( $P_\infty$ ) as is needed in the adopted solution procedure. Finally, velocity is specified as a transient boundary condition along the upstream and span-wise domain boundaries. This boundary condition is the vehicle for introducing the impinging vortex into the domain and is first implemented by Selvam (1985).

The schematic Figure 4.4 aids in understanding the computation of the transient velocity boundary condition. The fixed XY coordinate system originates from the cylinder center. The moving X'Y' coordinate system is attached to the vortex center and translates with it. The XY and X'Y' coordinate systems are linked by the fact the vortex and cylinder centers are aligned on the Y-axis when  $t^* = 0$ .



**Figure 4.4:** Schematic illustrating terms used to compute transient velocity boundary condition.

The coordinates  $(x_p, y_p)$  of each boundary point (P) are constant. The relative position  $(x_p', y_p')$  of each boundary point with respect to the vortex center is computed at each time step using Eqs. (4.8a) and (4.8b). The radial distance  $(r_p')$  between the vortex center and the boundary point is then computed using Eq. (4.9).

$$x_p' = x_p - U_\infty \cdot t^* \quad (4.8a)$$

$$y_p' = y_p - \delta \quad (4.8b)$$

$$r_p' = (x_p'^2 + y_p'^2)^{1/2} \quad (4.9)$$

Now  $V_\theta(r_p')$  is computed using Eq. (4.6), and Eqs. (4.10a) and (4.10b) are used to transform the tangential velocity into horizontal and vertical components and add them to the free stream velocity. This process is repeated for each boundary node at each time step. Eqs. (4.10a) and (4.10b) are also valid for any node within the computational domain and are used to initialize the velocity field based upon the initial position of the vortex.

$$U(t) = U_\infty - V_\theta(r_p') \cdot y_p' / r_p' \quad (4.10a)$$

$$V(t) = V_\theta(r_p') \cdot x_p' / r_p' \quad (4.10b)$$

## 4.5 Numerical Validation

The literature contains very few studies of vortex loading of structures, hence model validation is challenging. In the present study, a two-stage model validation procedure is adopted. First, free stream flow over the cylinder is simulated using both grids. This is a common benchmark problem for many studies within the realm of fluid mechanics, and data from previous studies are available for comparison. This phase of the model validation allows assessment of the model and grids' capability to resolve the boundary layer around the cylinder. Subsequently, the model's capability to transport the impinging vortex across the domain to impact the cylinder is assessed by extracting the vortex TVP at discrete intervals and comparing it with the TVP defined by Eq. (4.6). This validation study is used to demonstrate that (1) the numerical model accurately resolves the flow and (2) that the specified vortex impacts the cylinder. It is therefore argued that the resulting cylinder loading is realistic representation of the loading that an impacting vortex would produce.

### 4.5.1 Validation Using Free Stream Simulations

All simulations in the present study are characterized by  $Re \leq 300$ ; the present validation study is performed for the Reynolds number range of 50 to 300. Few physical experiments that measure loading on a cylinder immersed in a free stream are reported in the literature. This is likely because such low- $Re$  flows produce minimal loading that is difficult to measure. Therefore, all data used to validate the present model is from other computer simulations except for the data of Williamson (1996).

Table 4.3 summarizes the details of the free stream computer simulations from which data is taken to validate the present model. These studies are selected because the report sufficient data points for characterizing the cylinder loading across the specified Reynolds

number range. Also, they utilize domains that are large enough to avoid blockage effects which are produced by imposition of boundary conditions and artificially inflate Cd (Posdziech and Grundmann, 2007).

**Table 4.3:** Free stream computer simulations used to validate the present model.

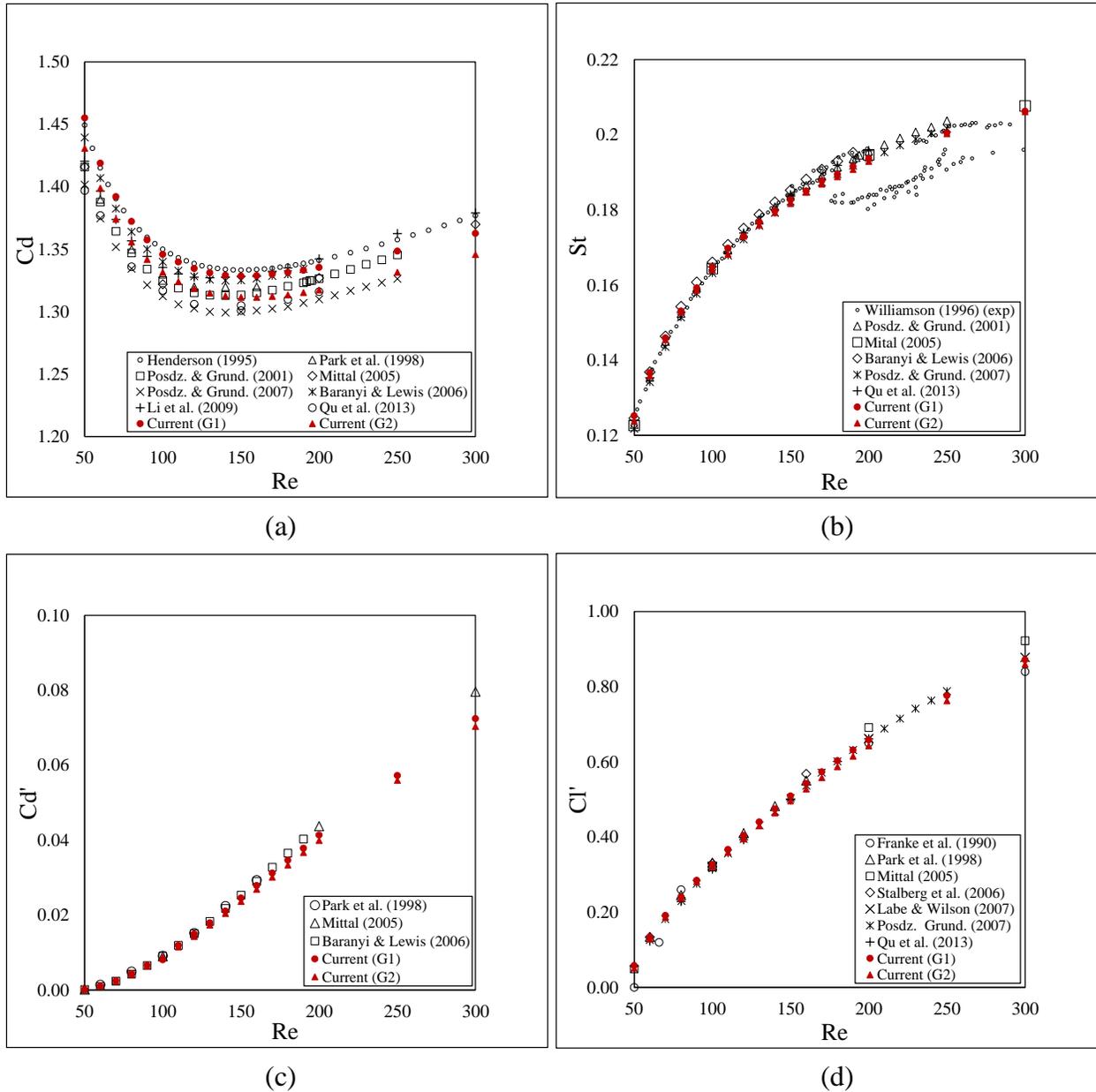
Source	Model	Re	Domain			
			Type	X <sub>i</sub>	X <sub>o</sub>	X <sub>s</sub>
Franke et al. (1990)	DS·CV	50-5000	O	...	20·D	...
Henderson (1995)	DS·SE	10-1000	R	16·D	25·D	28·D
Park et al. (1998)	DS·CV	2-160	C	50·D	20·D	50·D
Posdz. and Grund. (2001)	SE	40-240	C/R	70·D	50·D	70·D
Mittal (2005)	DS·FE	50-350	R	...	50·D	...
Baranyi and Lewis (2006)	DS·FD	10-220	O	...	40·D	...
Stålberg et al. (2006)	DS·FD	7-180	O	...	40·D	...
Labbe and Wilson (2007)	LES·CV	40-1000	O	...	15·D	...
Posdz. and Grund. (2007)	SE	5-250	C/R	...	4000·D	...
Li et al. (2009)	LB	50-300	R	...	50·D	...
Qu et al. (2013)	DS·CV	50-200	R	...	60·D	...
Current (G1)	DS·FE	50-300	R	12.5·D	45·D	12.5·D
Current (G2)	...	...	...	15·D	50·D	15·D

\*CV (Control Volume), DS (Direct Simulation), FD (Finite Difference), FE (Finite Element)

\*LB (Lattice Boltzman), LES (Large Eddy Simulation), SE (Spectral Element)

Figures 4.5a – 4.5d compares cylinder loading data from the present model (Grid 1 and Grid 2) with data reported by the studies listed in Table 4.3. Cd is the mean drag force coefficient, Cd' and Cl' are the amplitudes of the drag and lift force coefficients respectively, and  $St = f_{Cl} \cdot D / U_{\infty}$  is the Strouhal number, which is the dimensionless vortex shedding frequency. Mean drag force coefficient (Figure 4.5a) and Strouhal number (Figure 4.5b) are the typical metrics used in model validation. Values computed by the present model are in excellent agreement with values reported in the literature. The Cd curve for Grid 2 falls below the curve for Grid 1 because Grid 2 is larger, hence the blockage effect is reduced. It is interesting to note that a portion of Williamson's (1996) experimental Strouhal number data falls away from the

data from computer simulations for Reynolds numbers above 190. This is because the vortex shedding becomes three-dimensional in some of the experiments. Figures 4.5c and 4.5d respectively compare drag and lift force coefficient amplitudes computed by the current model with data from the literature. Computed values of  $C_d'$  and  $C_l'$  generally agree well with the literature although  $C_d'$  begins to fall below data from the literature for  $Re > 190$ .



**Figure 4.5:** Comparison of free stream cylinder loading data computed using the present model and grids with data from literature (a)  $C_d$ , (b)  $St$ , (c)  $C_d'$ , and (d)  $C_l'$ .

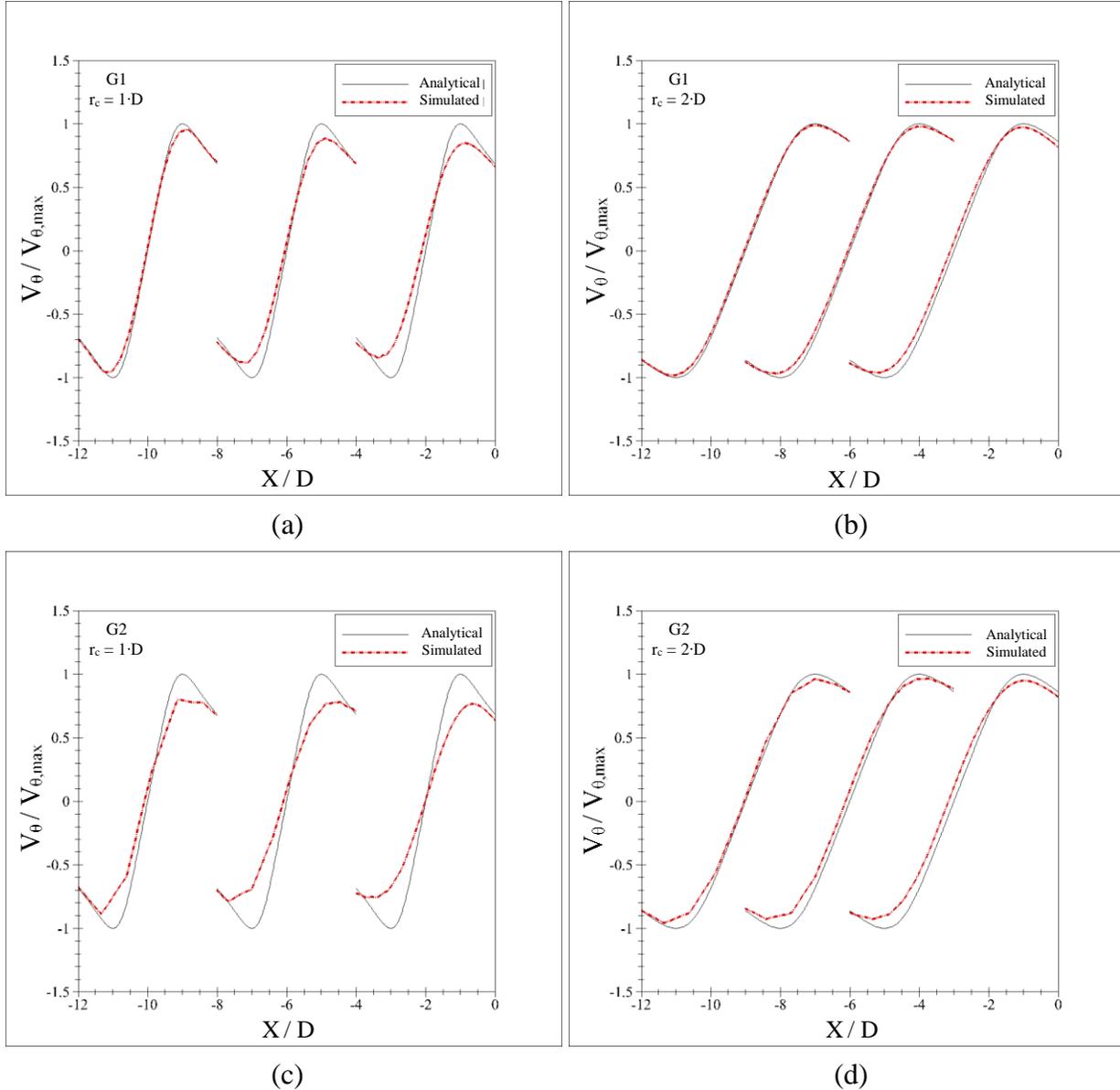
The free stream cylinder loading from the present model and grids adequately corresponds to that from the literature. Therefore, it is concluded that the present model and grids can accurately resolve the boundary layer around the cylinder in the considered regime that is characterized by Reynolds numbers less than 300.

#### 4.5.2 Validation by Assessment of Vortex Transport

Both Selvam (1998) and Tamura et al. (2008) cite the difficulty in convectively transporting a vortex. The vortex is transported through the nonlinear convection term of Eq. (4.5), and numerical dissipation alters the vortex profile as it is transported across the domain. It is therefore necessary to quantify the dissipation of the vortex so that the structure of the vortex that actually traverses the domain and impacts the cylinder is known. The L-O vortex profile is used primarily in the present study, hence the first stage in assessing the accuracy of vortex transport is to compare the accuracy with which Grid 1 and Grid 2 transport a L-O vortex. The vortex TVP is extracted at discrete intervals as the vortex crosses the domain and compared with the analytical profile defined by Eq. (4.6) where  $n = 2$ . The study is performed for impinging vortex sizes of  $r_c = 1 \cdot D$ , which is the smallest impinging vortex size considered in this study, and  $2 \cdot D$ . Figures 4.6a – 4.6d summarize the extracted TVPs from simulation and compare them with the analytical TVPs.

Numerical diffusion progressively flattens the TVP and reduces the maximum tangential velocity as the vortex crosses the domain. For G1,  $V_{\theta, \max}$  is reduced to 83% ( $r_c = 1 \cdot D$ ) and 97% ( $r_c = 2 \cdot D$ ) of the analytical value by the time the impinging vortex impacts the cylinder. The dissipation of the vortex is greater for G2, where  $V_{\theta, \max}$  is reduced to 77% ( $r_c = 1 \cdot D$ ) and 94% ( $r_c = 2 \cdot D$ ) of the analytical value prior to impacting the cylinder. The vortex transport accuracy

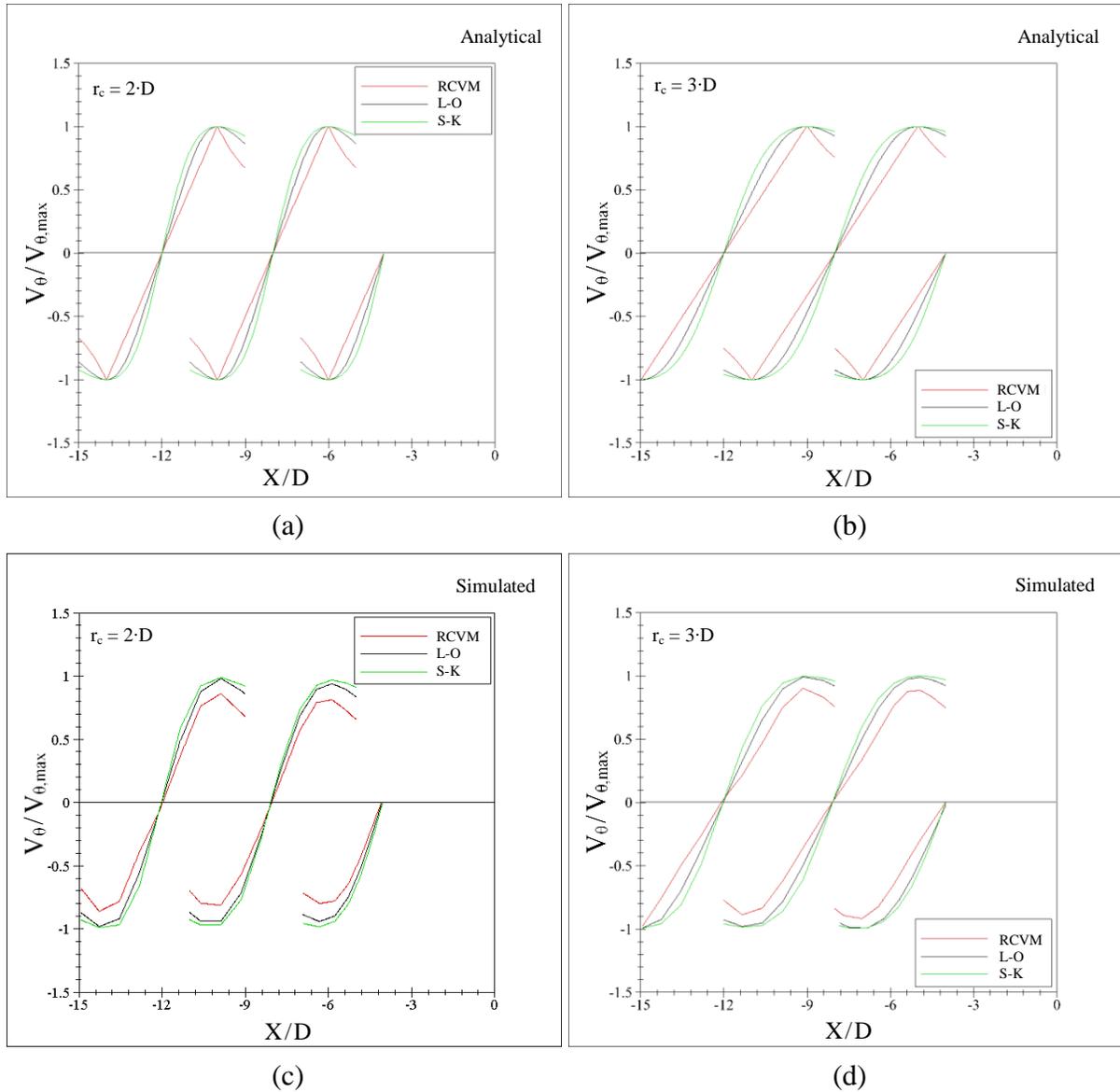
progressively increases with impinging vortex size because more grid points are used to approximate the TVP.



**Figure 4.6:** Comparison of L-O vortex transport accuracy using Grid 1 and Grid 2: (a) G1,  $r_c = 1 \cdot D$ , (b) G1,  $r_c = 2 \cdot D$ , (c) G2,  $r_c = 1 \cdot D$ , and (d) G2,  $r_c = 2 \cdot D$ .

The sharpness of the vortex TVP greatly influences the vortex transport accuracy because a sharper profile changes more between grid points hence the approximation accuracy decreases. This is illustrated by simulating the transport of S-K, L-O, and RCVM vortices having critical

radii of  $2\cdot D$  and  $3\cdot D$  using Grid 2. Analytical TVPs are summarized in Figures 4.7.1a – 4.7.1c, and simulated profiles are summarized in Figures 5.7.2a – 4.7.2c.



**Figure 4.7:** Comparison of analytical (a-b) and simulated (c-d) vortex tangential velocity profiles.

From *Figures 4.7a – 4.7d*, it is evident that the RCVM profile is transported with much less accuracy than the L-O or S-K profiles. This is because the sharp discontinuity in the RCVM profile near  $r = r_c$  cannot be accurately approximated without excessive grid refinement. Table 4.4 summarizes the percentage of  $V_{\theta,max}$  that is actually transported across the domain to the

cylinder. The smooth and continuous L-O and S-K profiles are transported with excellent accuracy for  $r_c \geq 3 \cdot D$ , while the RCVM profile must be larger ( $r_c \geq 5 \cdot D$ ) to be transported with excellent accuracy.

**Table 4.4:** Percentage of  $V_{\theta, \max}$  transported for each vortex size and profile.

$r_c$	Profile		
	RCVM	L-O	S-K
2·D	80%	94%	98%
3·D	91%	> 99%	> 99%

From this study, it is concluded that S-K and L-O vortices having  $r_c \geq 3 \cdot D$  and RCVM vortices having  $r_c \geq 5 \cdot D$  can be transported with excellent accuracy by the coarser Grid 2. Numerical diffusion decreases the transport accuracy when smaller vortices are simulated. Simulations are conducted and reported in the present study using vortices as small as  $r_c = 1 \cdot D$  because the simulation results are physically meaningful. However, care must be taken when the cylinder loading from these simulations is being analyzed, as the smaller vortices are weakened by numerical diffusion prior to impacting the cylinder.

## **CHAPTER 5: THE INFLUENCE OF THE IMPINGING VORTEX'S SIZE AND PATH ON STRUCTURAL LOADING**

### **5.1 Chapter Overview and Progression**

Structures are loaded by vortices at practically all relative vortex-to-structure size scales. Rotor- and wing-tip vortices encountered in aerospace applications are similar in size to the structures that they load. Atmospheric vortices such as tornados and hurricanes range from similar-sized to many times larger than the structures that they load. The forthcoming study simulates vortex impact with a slender, cylindrical to study and document the influence of the impinging vortex's size on the resulting structural loading.

The first portion of the forthcoming study simulates direct vortex impact (See Figure 5.14b) with the structure to identify and explain a unique phenomenon that produces variation in the structural loading when the impinging vortex impacts the structure at different times. The phenomenon is illustrated and explained, and the range of variation in the structural loading amplitude for vortex impact at different times is quantified. Subsequently, the impinging vortex size is incrementally increased while holding the vortex's maximum tangential velocity constant. Direct vortex impact is simulated for each impinging vortex size, and maximum force coefficient amplitudes are utilized to define the structural loading amplitude trend with respect to impinging vortex size. Also, the influence of the impinging vortex size on the aforementioned variation in structural loading amplitude with vortex time is assessed. Phenomenon controlling the trend in structural loading with respect to the impinging vortex size are illustrated and discussed. Finally, the influence of laterally shifting the impinging vortex's path (See Figures 5.14a and 5.14c) on the structural loading amplitude is evaluated. A single vortex size is selected, and its path is incrementally shifted so that it travels above and below the structure. Maximum force coefficients are utilized to assess the trend in structural loading amplitude with respect to the

vortex's path shift. Maximum force coefficients produced by the vortex are compared with maximum force coefficients produced by an equivalent-velocity free stream to assess whether the rotational nature of the vortex produces greater structural loading than a free stream.

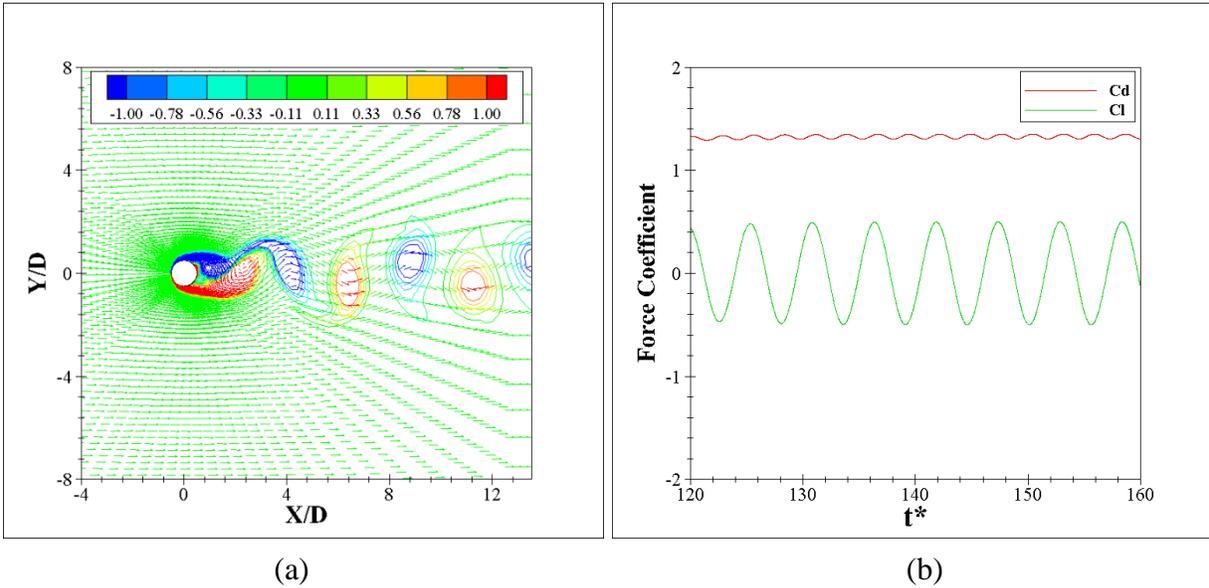
## **5.2 Variation in Maximum Cylinder Loading Due to Vortex Impact Time**

Direct vortex impact of  $r_c \leq 3 \cdot D$  vortices with a cylindrical structure is simulated to illustrate, explain, and quantify variation in structural loading amplitude when the impinging vortex directly impacts (Direct impact is illustrated in Figure 5.14b) the structure at different times. During the early stages of this study, it was observed that an  $r_c = 1 \cdot D$  vortex produces different structural loading when directly impacts the structure at different times with respect to the start of the simulation. This implies that the vortex's starting position  $X_o$  (See Figure 4.1) physically influences the structural loading that the vortex produces upon impacting the structure and is a fallacious. The starting position of the vortex  $X_o$  is set sufficiently far to the left of the cylinder so that vortex shedding from the cylinder develops well before vortex impact. The relative size and strength of the vortices attached to the cylinder change continuously during vortex shedding, hence changing  $X_o$  effectively causes the impinging vortex to impact the structure at different times with respect to the vortex shedding cycle (The vortex shedding cycle is discussed in the subsequent subsection). It is postulated that interaction between the impinging vortex and different attached vortices produces the variation in structural loading when  $X_o$  is changed. The subsequent subsections develop and implement methodology to assess this postulation.

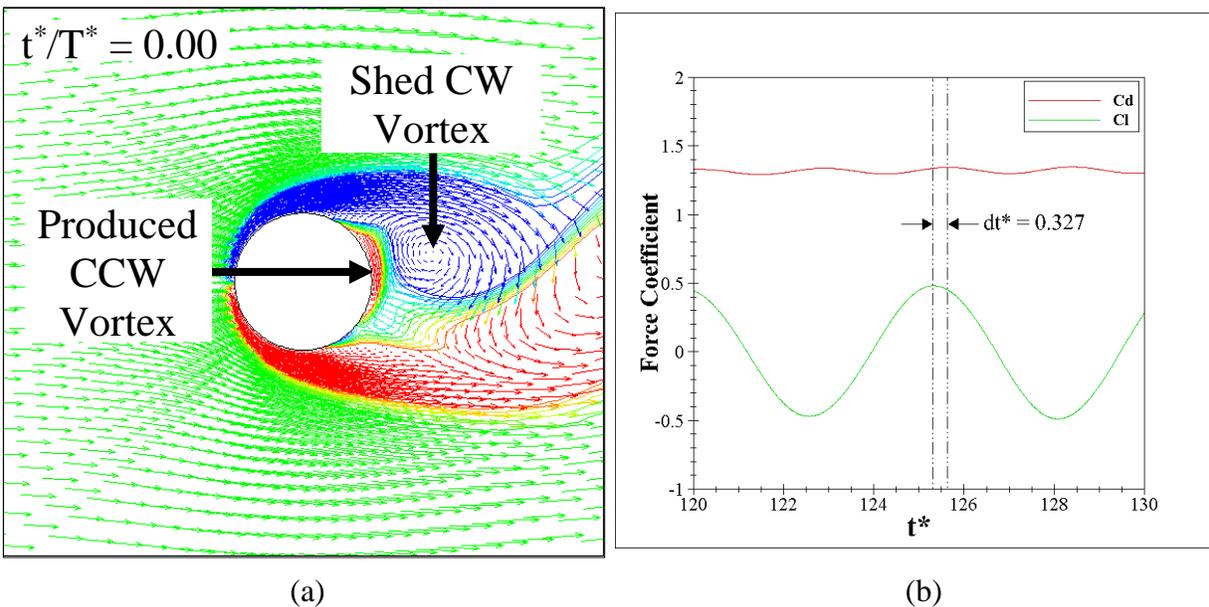
### 5.2.1 Methodology and Approach

A cylinder immersed in a free stream characterized by the approximate range  $47 \leq Re \leq 2 \cdot 10^5$  alternately sheds clockwise (CW) and counter-clockwise (CCW) vortices from its top and

base respectively. The alternate shedding of these vortices produces harmonic cylinder loading both in the stream direction and normal to the stream direction. Figure 5.1a illustrates vortex shedding from a cylinder at  $Re = 150$ , and Figure 5.1b illustrates the resulting cylinder loading.



**Figure 5.1:** (a) Vorticity contour and velocity vectors of free stream vortex shedding at  $Re = 150$  and (b) corresponding drag and lift force coefficients.



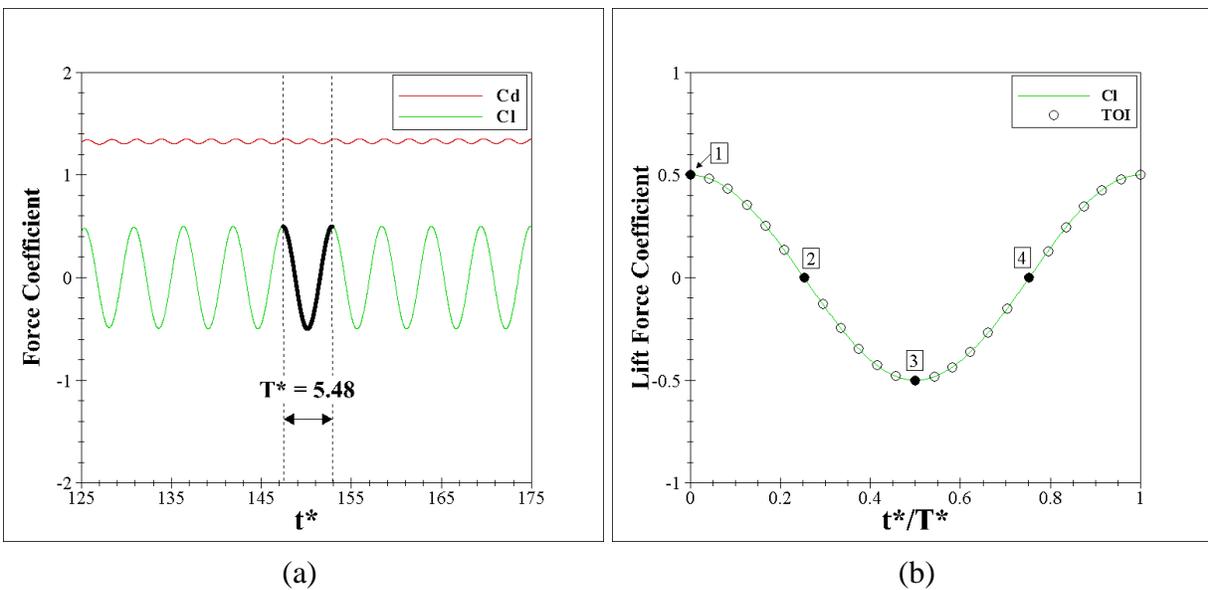
**Figure 5.2:** (a) Shedding of a CW vortex from the top of the cylinder and (b) illustration of the lag between maximum values of the lift and drag force coefficients.

The lift force coefficient frequency ( $f_{Cl}$ ) is the frequency at which same-signed vortices are shed, and the drag force coefficient frequency is  $f_{Cd} = 2 \cdot f_{Cl}$ . The lift force coefficient reaches maximum positive amplitude when a CW vortex is shed from the top of the cylinder and maximum negative amplitude when a CCW vortex is shed from the base of the cylinder. The drag force coefficient reaches its maximum positive amplitude shortly after the shedding of a vortex of either sign as indicated by  $dt^*$  in Figure 5.2b. The positive amplitude peak in  $C_d$  is caused by the opposite-signed vortex produced on the downstream edge of the cylinder when a vortex of either sign is shed; this is illustrated in Figure 5.2a for the shedding of a CW vortex from the top of the cylinder.

The present study defines the vortex shedding cycle (VSC) as one period ( $T^*$ ) of the lift force coefficient. The VSC begins and ends with the shedding of a CW vortex from the cylinder, and its duration is  $T^* = 5.48$  for a free stream characterized by  $Re = 150$  on Grid 1. The relative size and strength of the attached CW and CCW vortices change continuously during the VSC, and it is postulated that interaction between the impinging vortex and different attached vortices produces the variation in structure loading amplitude. It is therefore necessary to simulate vortex impact at discrete intervals throughout  $T^*$  to evaluate the variation in cylinder loading amplitude when the impinging vortex impacts at different times.

Free stream flow around an immersed cylinder is simulated, and the temporal location of a single period of the VSC is identified (See Figure 5.3a). The VSC period  $T^* = 5.48$  is then discretized by assigning 25 times of vortex impact (TOI) spaced at  $\Delta t^* \approx 0.23$  interval as shown in Figure 5.3b. Note that there is no scientific basis for using 25 TOI to discretize  $T^*$ , rather this number of TOI is sufficient to capture the variation in cylinder loading amplitude for the given system and parameters. The vortex starting position  $X_0$  is adjusted for each simulation so that the

vortex impacts the cylinder at each TOI. The present study defines “impact” as the instant that the leading edge of the vortex touches the leading edge of the cylinder assuming no slowing or deformation of the impinging vortex due to interaction with the cylinder’s boundary layer. The greatest and least values of the drag and lift force coefficient are extracted following each simulation. These are reported as signed amplitudes ( $Cl^+$ ), ( $Cl^-$ ), ( $Cd^+$ ), and ( $Cd^-$ ), which are computed by subtracting the mean values of the drag and lift force coefficient for  $Re = 150$  flow over an immersed circular cylinder ( $Cd,mean = 1.329$  and  $Cl,mean = 0.0$ ).

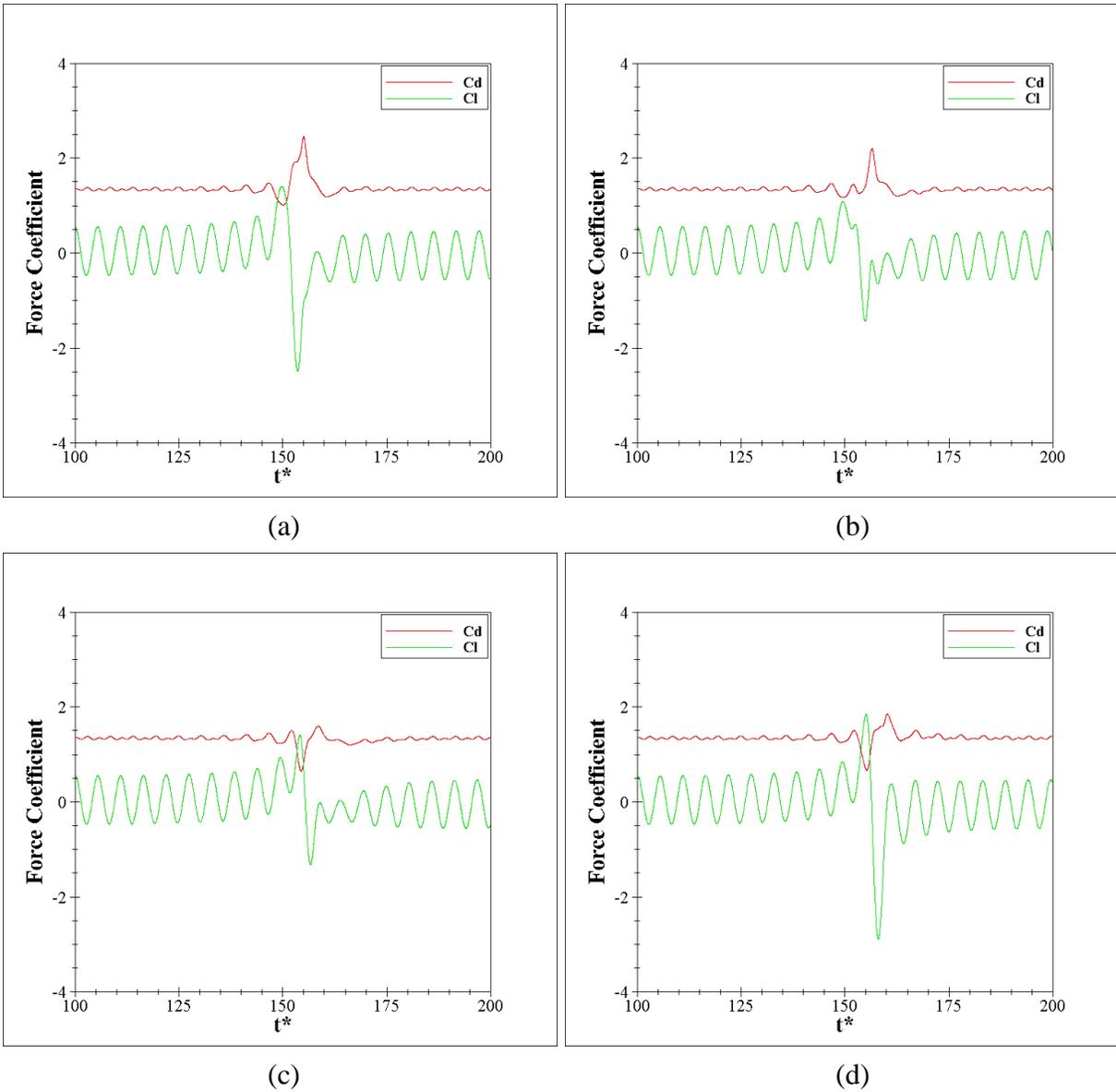


**Figure 5.3:** (a) Selected period of the VSC and (b) discretization of the VSC with 25 TOI (Strasser and Selvam, 2015).

### 5.2.2 Maximum Force Coefficient Amplitudes

Four vortex impact times are marked by boxed numbers 1-4 in Figure 5.3b. These TOI are specifically selected to illustrate the variation in structure loading when the impinging vortex impacts the cylinder at different times in the VSC because the structure of the vortices attached to the cylinder is well understood at these times, as shall be expounded in Section 5.2.2.2. Vortex impact at TOI 1-4 is simulated, where the critical radius of the impinging vortex is  $r_c = 1 \cdot D$ , and the four resulting force coefficient time histories are illustrated in Figures 5.4a – 5.4d. Both the

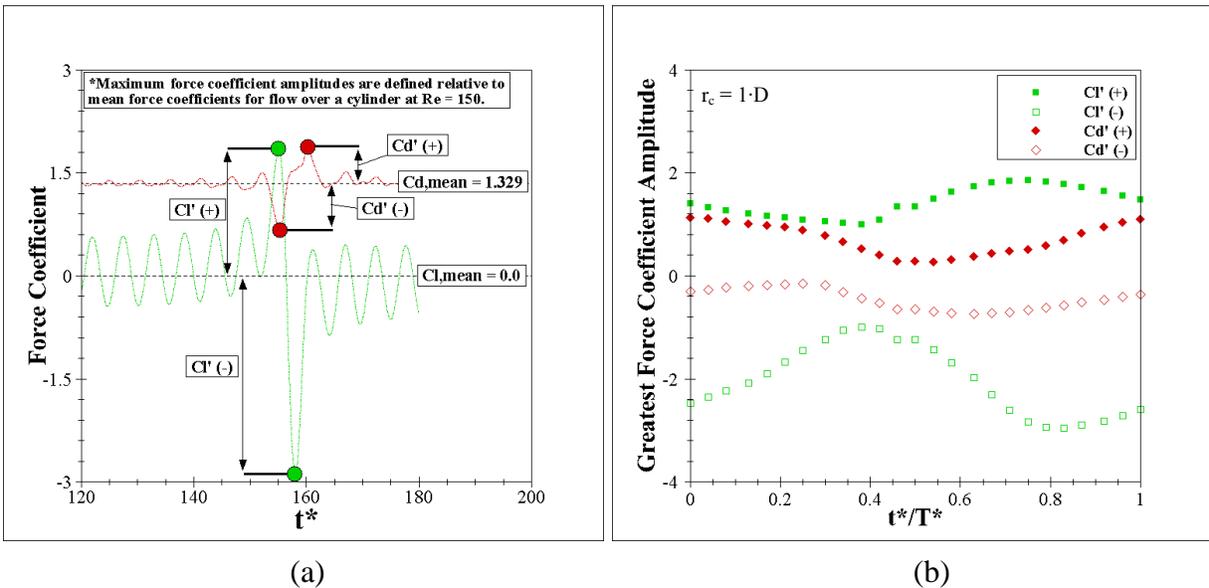
character and maximum amplitudes of both force coefficients exhibit great variation when the impinging vortex impacts the cylinder at different times.



**Figure 5.4:** Force coefficient time histories for impinging vortex of radius  $r_c = 1 \cdot D$  impacting the cylinder at (a) TOI 1, (b) TOI 2, (c) TOI 3, and (d) TOI 4 (Strasser and Selvam, 2015).

Now, the starting position of the impinging vortex is adjusted for each simulation so that the vortex impacts the cylinder at the 25 TOI defined in Figure 5.3b. Maximum force coefficient amplitudes are extracted from each force coefficient time history as illustrated in Figure 5.5a following each simulation and summarized in Figure 5.5b. The curves of maximum force

coefficient amplitudes in Figure 5.5b are smooth and continuous. The curves are smooth because the attached vortex structure changes continuously rather than abruptly throughout the VSC. The continuous nature of the curves confirms that the 25 TOI used to discretize the VSC provide sufficient resolution to capture the variation in cylinder loading with vortex impact time.



**Figure 5.5:** (a) Extraction of signed amplitudes from a typical force coefficient time history and (b) summary of signed amplitudes for impinging vortex impact at 25 TOI (Strasser and Selvam, 2015).

The greatest and least values of each of the four signed force coefficient amplitudes from Figure 5.5b are tabulated and compared in Table 5.1. The maximum underprediction (UP) of the greatest amplitude by the least amplitude is computed and also reported in Table 5.1.

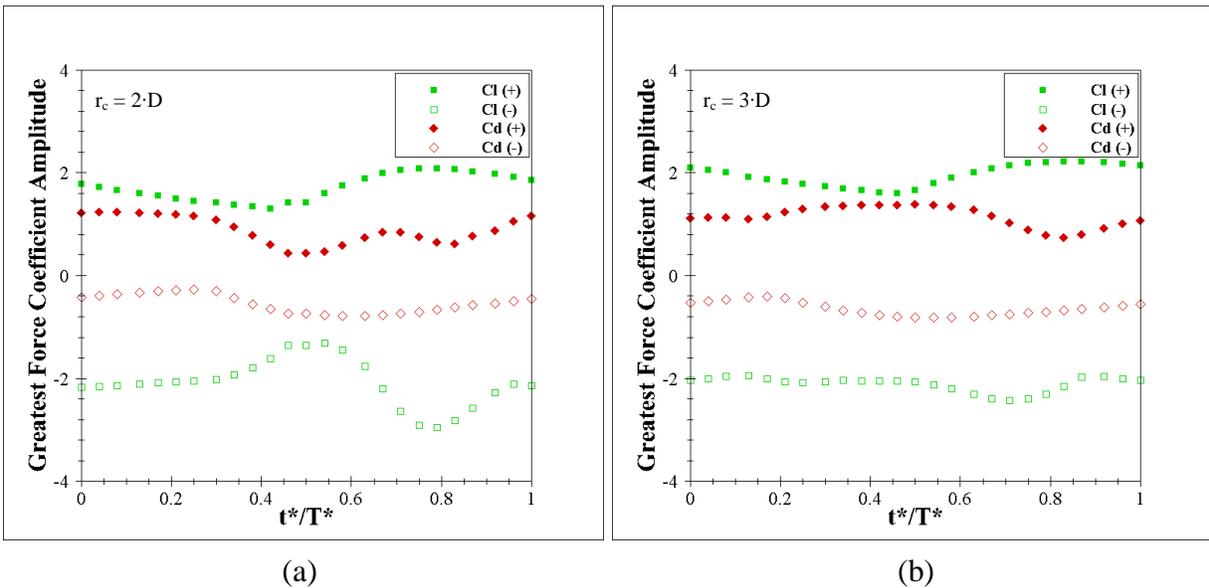
**Table 5.1:** Greatest and least maximum force coefficient amplitudes for impinging vortex of radius  $r_c = 1 \cdot D$  impacting the cylinder at 25 TOI (Illustrated in Figure 5.5a).

	Maximum Force Coefficient Amplitudes From 25 Simulations			
	$Cl' (+)$	$Cl' (-)$	$Cd' (+)$	$Cd' (-)$
Greatest	1.851	-2.952	1.128	-0.735
Least	1.000	-0.994	0.275	-0.145
UP	84.99%	197.04%	310.42%	407.15%

\*Underprediction:  $UP = (Greatest/Least - 1.0) \cdot 100\%$

UP quantifies the possible error that may be present if maximum force coefficient amplitudes from only a single simulation are considered. Gross under-prediction of the maximum force coefficients may occur if variation in cylinder loading amplitude with vortex impact time is neglected.

Now the preceding methodology, specifically using 25 TOI to define the curves of maximum force coefficient amplitudes for vortex impact at any time in the VSC, is repeated for impinging vortex critical radii of  $r_c = 2 \cdot D$  and  $3 \cdot D$ . Recall that the maximum vortex tangential velocity  $V_{\theta, \max}$  remains fixed as the vortex size increases. Resulting curves of the maximum force coefficient amplitudes for critical radii of  $r_c = 2 \cdot D$  and  $3 \cdot D$  are respectively summarized in Figures 5.6a and 5.6b. Qualitative comparison of the maximum force coefficient amplitude curves for impinging vortex radii of  $1 \cdot D$  to  $3 \cdot D$  (Figure 5.5b and Figures 5.6a and 5.6b), shows that the variation in maximum force coefficient amplitude decreases rapidly as the size of the impinging vortex increases.



**Figure 5.6:** Maximum force coefficient amplitudes for impinging vortex impact at 25 TOI for impinging vortex radii of (a)  $2 \cdot D$  and (b)  $3 \cdot D$  (Strasser and Selvam, 2015).

Table 5.2 quantifies the Variation in maximum force coefficient amplitude with vortex impact time by reporting the mean and mean-normalized standard deviation (NSD) for each maximum force coefficient amplitude curve. Mean maximum force coefficient amplitudes consistently increase with increasing vortex size. This is because increased cylinder surface area is exposed to high tangential velocity as the impinging vortex size increases; therefore, the amplitude of cylinder loading increases as well.

**Table 5.2:** Mean and mean-normalized standard deviation of maximum force coefficient amplitude curves for impinging vortex critical radii of 1·D to 3·D.

	Maximum Force Coefficient Amplitudes from 25 Simulations			
Vortex Radius	Cl (+)	Cl (-)	Cd (+)	Cd (-)
	Mean Values			
1·D	1.432	-2.030	0.707	-0.459
2·D	1.710	-2.083	0.886	-0.548
3·D	1.949	-2.109	1.150	-0.647
	Mean-Normalized St. Deviation			
	20.34%	33.44%	42.75%	45.90%
	15.62%	22.12%	31.30%	32.96%
	11.09%	6.91%	17.86%	20.91%

The NSD in maximum force coefficient amplitude curves progressively decreases as the impinging vortex size increases. Decreased variation in cylinder loading amplitude with vortex impact time implies that the interaction between the impinging and attached vortices has increasingly-less influence on the cylinder loading amplitude as the impinging vortex size increases. This is because the larger impinging vortex exerts greater influence on the flow over the cylinder prior to impacting it. The impinging vortex increasingly disrupts and then controls vortex shedding from the cylinder prior to impacting it. As the impinging vortex size is continually increased, it controls vortex shedding from the cylinder prior to impacting it, meaning the vortex impact time with respect to the original vortex shedding cycle no longer

influences the cylinder loading amplitude. Consequently, any starting position of the vortex will produce the same cylinder loading amplitude.

Table 5.3 summarizes the greatest and least values of the maximum force coefficient amplitudes for impinging vortex radii of  $1 \cdot D$  to  $3 \cdot D$ . The greatest underprediction of all maximum force coefficient amplitudes progressively decreases as the impinging vortex size increases, supporting the prior finding that the variation in maximum cylinder loading amplitude with vortex impact time rapidly decreases as impinging vortex size increases. However, underprediction, especially for the drag force coefficient's signed amplitudes, is still substantial when the vortex critical radius is  $3 \cdot D$ .

**Table 5.3:** Summary of greatest and least maximum force coefficient amplitudes for impinging vortex radii of  $1 \cdot D$  to  $3 \cdot D$ .

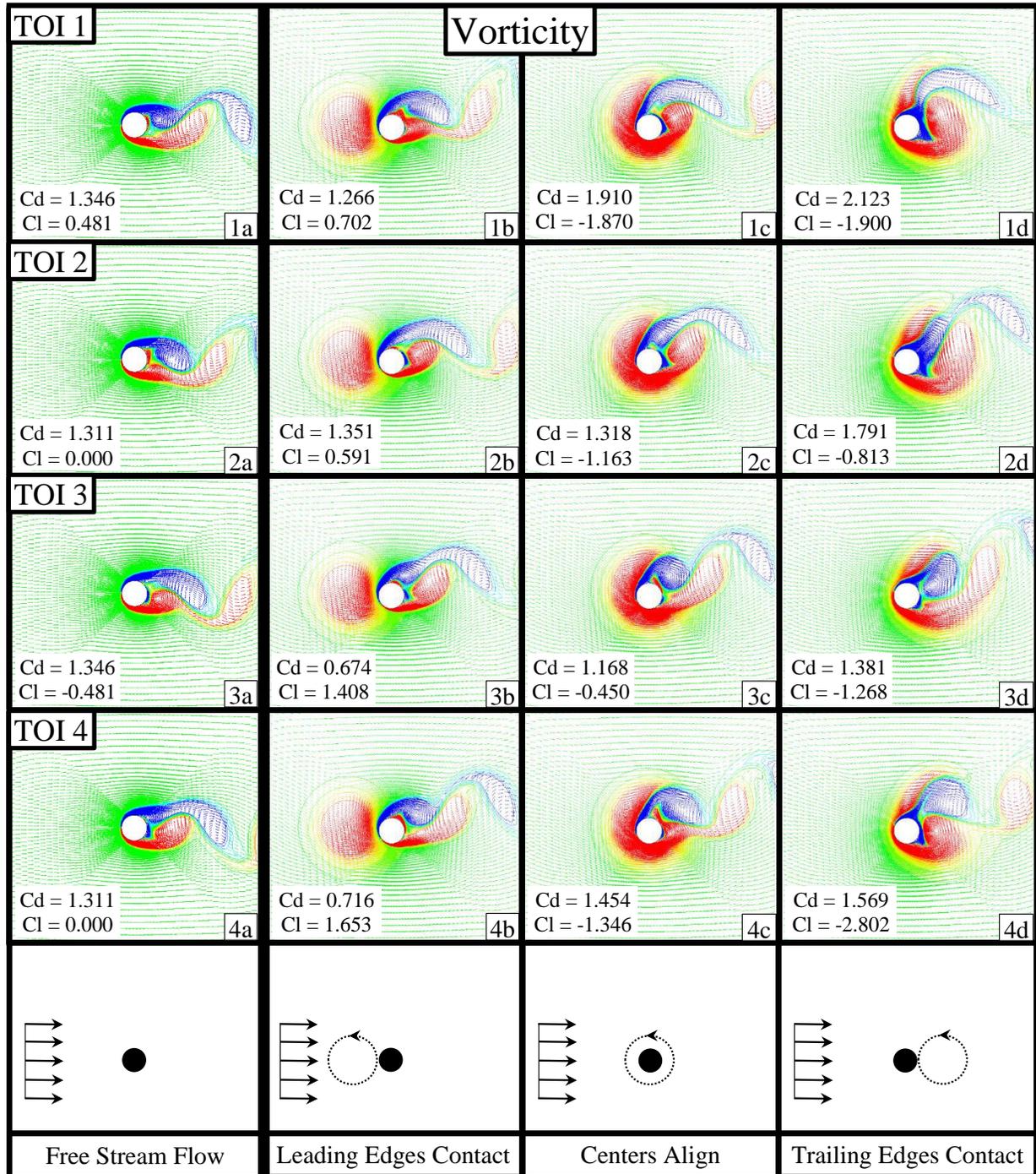
	Maximum Force Coefficient Amplitudes from 25 Simulations			
Vortex Radius	Cl (+)	Cl (-)	Cd (+)	Cd (-)
	Greatest Value			
$1 \cdot D$	1.851	-2.952	1.128	-0.735
$2 \cdot D$	2.085	-2.954	1.243	-0.784
$3 \cdot D$	2.214	-2.431	1.380	-0.817
	Least Value			
	1.000	-0.994	0.275	-0.145
	1.303	-1.319	0.433	-0.267
	1.591	-1.947	0.740	-0.401
	Greatest Under-Prediction			
	84.99%	197.04%	310.42%	407.15%
	59.94%	123.94%	186.85%	193.74%
	39.15%	24.84%	86.54%	103.64%

\*Underprediction:  $UP = (Greatest/Least - 1.0) \cdot 100\%$ .

### 5.2.3 Visualization of the Interaction between Impinging and Attached Vortices

Impact of an  $r_c = 1 \cdot D$  vortex at TOI 1-4 is used to illustrate interaction between the impinging and attached vortices when the impinging vortex impacts the cylinder at different

times in the VSC. TOI 1-4 are selected for illustration of the interaction between vortices because the structure of the vortices attached to the cylinder due to the free stream are well



**Figure 5.7:** Vorticity contours and velocity vectors at TOI 1-4 (by row) for free stream flow over the cylinder (column a) and for the three relative positions of the vortex to the cylinder illustrated in row 5 (columns b-d) (Strasser and Selvam, 2015).

understood. Qualitative description of the attached vortex structure at TOI 1-4 is listed in bullet format below; rows 1-4 of column “a” in Figure 5.7 illustrate corresponding vorticity contours.

- TOI 1 – CW vortex is just shed. (CW – Largest, CCW – Small).
- TOI 2 –  $T^*/4$  from the shedding of a CCW vortex. (CW – Smallest, CCW – Large).
- TOI 3 – CCW vortex is just shed. (CW – Small, CCW – Largest).
- TOI 4 -  $T^*/4$  from the shedding of a CW vortex. (CW – Large, CCW – Smallest).

Figure 5.7 illustrates the vorticity field surrounding the cylinder for impinging vortex impact at TOI 1-4. Rows 1-4 represent TOI 1-4, and columns b-d correspond to the three “expected” relative positions of the vortex to the cylinder illustrated by the schematics in row 5. The contours are said to illustrate “expected” relative positions of the vortex with respect to the cylinder because it is assumed that the impinging vortex continues to translate at  $U_\infty$  throughout interaction with the cylinder. Instantaneous values of  $C_d$  and  $C_l$  are provided in the lower-left corner of each figure. The visualization in Figure 5.7 is used to discuss two significant forms of inter-vortex interaction, which is interaction between the impinging vortex and those attached to the cylinder. Specifically, the impinging vortex alters the positions of the vortices attached to the cylinder, and the impinging vortex delays the vortex shedding cycle.

#### ***5.2.3.1 Observed Interaction between Impinging and Attached Vortices***

The free stream flowing over the cylinder develops CW and CCW vortices at the top and base of the cylinder, respectively, which are shed alternately as illustrated in column “a” of Figure 5.7. As the impinging CCW vortex approaches the cylinder, its tangential velocity alters the velocity of the stream over the cylinder. Consequently, the attached vortices are rotated CCW around the cylinder as shown in the column “b” of Figure 5.7. The attached CW and CCW vortices now partially occupy the upstream and downstream edges of the cylinder respectively.

The net strength of these attached vortices controls the amplitude of  $C_d$ , as shall be discussed shortly.

The impinging vortex delays the vortex shedding cycle by approximately  $T^*/4$  in addition to shifting the positions of the vortices attached to the cylinder. This is seen when comparing the similarity of the vortices attached to the cylinder in Figures 5.7(1a) and 5.7(4b) and those in Figures 5.7(3a) and 5.7(2b). Therefore, the structure of the attached vortices when the impinging vortex impacts at TOI 1-4 is actually:

- TOI 1 –  $T^*/4$  from the shedding of a CCW vortex. (CW – Smallest, CCW – Large).
- TOI 2 – CCW vortex is just shed. (CW – Small, CCW – Largest).
- TOI 3 -  $T^*/4$  from the shedding of a CW vortex. (CW – Large, CCW – Smallest).
- TOI 4 – CW vortex is just shed. (CW – Largest, CCW – Small).

The attached vortices illustrated in column “b” of Figure 5.7 are noticeably larger than those in column “a”. It is postulated that the delay of the vortex shedding cycle and the increased stream velocity over the cylinder (due to the impinging vortex’s tangential velocity) cause the attached vortices to grow larger, thus they exert greater loading on the cylinder.

### ***5.2.3.2 Influence of Impinging-Attached Vortex Interaction on Cylinder Loading***

Discussion shall focus on inter-vortex interaction producing large force coefficient amplitudes. Interaction scenarios producing small amplitudes can be inferred but are omitted to avoid redundancy. It is convenient to begin discussion with  $C_d$ . A vortex is a low pressure region, hence, the attached vortices pull the cylinder towards themselves. The amplitude of the force on the cylinder is the net force produced by the two vortices, and the sign of the amplitude is in the direction of the larger attached vortex.

The attached CW and CCW vortices are rotated to partially occupy the upstream and downstream surfaces of the cylinder, respectively, as the impinging vortex approaches. One of the attached vortices is always larger than the other. A larger CW vortex produces high  $Cd'(-)$ , while a larger CCW vortex results in high  $Cd'(+)$ . Time histories in Figures 5.4a – 5.4d show that vortex impact at TOI 1 and 2 produce large  $Cd'(+)$  while impact at TOI 3 and 4 produce large  $Cd'(-)$ . It is now known that the VSC is delayed  $T^*/4$  by the impinging vortex. Therefore, the structures of attached vortices (defined in the second set of bullet points) that produce large  $Cd'(+)$  and  $Cd'(-)$  are as expected.

The largest values of  $Cl'(+)$  and  $Cl'(-)$  occur when the impinging vortex impacts at TOI 4 (Figure 5.4d) as a CW vortex is being shed. The occurrence of the large value of  $Cl'(+)$  is expected, as the attached CW vortex is at its largest size, and therefore, exerts maximum pull along the top surface of the cylinder. In addition to exerting this pull, the large CW vortex shifts the path of the impinging vortex. In the absence of attached vortices, the impinging vortex will engulf the cylinder, simultaneously producing similar pull along the top and base of the cylinder. However, the large CW vortex that is now shifted towards the upstream edge of the cylinder interacts with and resists the impinging vortex. The path of the impinging vortex is shifted downward, and it merges with the attached CCW vortex attached to the base of the cylinder. The merged and now-large CCW vortex exerts a strong downward pull along the base of the cylinder, causing the large  $Cl'(-)$ .

#### 5.2.4 Summary of Interaction between Impinging and Attached Vortices

The interaction between impinging and attached vortices responsible for the variation in the maximum force coefficient amplitude with vortex TOI is attributed to two factors. For impinging vortex radius  $r_c = 1 \cdot D$ , the free stream VSC is delayed by  $T^*/4$  as the impinging vortex

approaches the cylinder. The attached vortices enlarge during the delayed shedding.  $C_d^{'+}$  and  $C_d^{'-}$  are large if the attached CW and CCW vortices (respectively) are large when the impinging vortex impacts.  $C_l^{'+}$  and  $C_l^{'-}$  are both large if the attached CW vortex is large when the impinging vortex impacts.

The preceding discussion of interaction of shed and impinging vortices is only directly applicable to a system having the same parameters. Any number of parameters may influence the interaction: Reynolds number, impinging vortex size, maximum velocity of the impinging vortex relative to bulk velocity, etc. However, the phenomenon has been identified and explained, and the procedure and methodology developed herein can be applied to study any vortex-structure system.

### **5.3 Influence of Relative Vortex-to-Cylinder Size on Cylinder Loading**

Section 5.3 utilizes Grid 2 to study the influence of the relative vortex-to-cylinder size on the resulting cylinder loading amplitude. The impinging vortex critical radius is incremented from  $1 \cdot D$  to  $100 \cdot D$  while the maximum vortex tangential velocity  $V_{\theta, \max}$  is fixed, and the trend in maximum cylinder loading amplitude is documented. Additionally, the influence of impinging vortex critical radius on the variation in cylinder loading with vortex impact time (Discussed in Section 5.2) is evaluated. The study performed herein specifically identifies the vortex core radii for which vortex impact time ceases to influence the maximum cylinder loading amplitude and the maximum cylinder loading amplitude becomes asymptotic, meaning subsequent increase of the critical radius does not influence the maximum cylinder loading amplitude. The phenomenon controlling the convergence of maximum force coefficient amplitudes to asymptotic values are then illustrated and explained.

### 5.3.1 Methodology and Approach

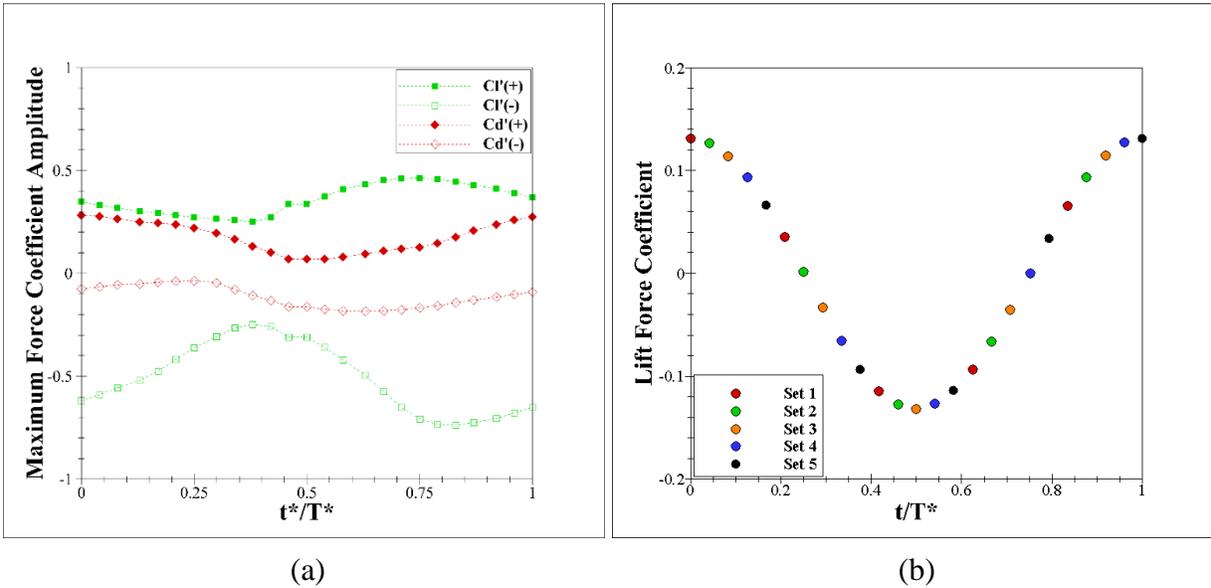
Section 5.2 demonstrates that different impinging vortex impact times with respect to the free stream vortex shedding cycle produce variation in the resulting maximum cylinder loading amplitude. The forthcoming study accounts for this variation by presenting mean values of the maximum force coefficient amplitudes as well as maximum and minimum values of each maximum force coefficient amplitude for each impinging vortex size. The previous study in Section 5.2 utilizes twenty-five vortex impact times to define mean values for the maximum force coefficient amplitudes for impinging vortex critical radii of  $1 \cdot D$  to  $3 \cdot D$ . The forthcoming study spans critical radii of  $1 \cdot D$  to  $100 \cdot D$ , and it is not feasible to perform so twenty-five computer simulations for each impinging vortex size. Fortunately, an alternative procedure for computing the mean values of the maximum force coefficient amplitudes is developed as shall be discussed subsequently.

Through iterative investigation, which is basically a guess-and-check approach, it has been found that mean values for the maximum force coefficient amplitudes can be computed using five simulated vortex impacts spaced at  $\Delta t^* = 1$  interval rather than twenty-five simulated vortex impacts spaced at  $\Delta t^* = 0.23$  interval. Additionally, the first of the five impacts need not start at the beginning of the VSC, but rather may be an arbitrary time. Validation of the use of five versus twenty-five simulated vortex impacts to define the mean values of the maximum force coefficient amplitudes is demonstrated subsequently.

Maximum force coefficient amplitude curves for impinging vortex core radius  $r_c = 1 \cdot D$  impacting the cylinder at twenty-five discrete times during one vortex shedding cycle, which are defined previously in Figure 5.5b, are re-dimensioned using reference velocity of two and illustrated in Figure 5.8a. The maximum force coefficient amplitudes are now separated into five

groups spaced at approximately  $\Delta t^* = 1$  interval as indicated by colored circles in Figure 5.8b.

Mean values for the maximum force coefficient amplitudes are computed using each of the five data sets and also using all twenty-five TOI as a single data set; summary and comparison of the six mean maximum force coefficient sets is provided in Table 5.4.



**Figure 5.8:** (a) Maximum force coefficient amplitudes for impinging vortex ( $r_c = 1 \cdot D$ ) impact at 25 TOI and (b) Separation of 25 TOI into 5 sets of 5.

**Table 5.4:** Comparison of mean maximum force coefficient amplitudes computed with 5 sets of 5 TOI with mean maximum force coefficient amplitudes computed using all 25 TOI as a set.

Set	Mean Values			
	$Cl'$		$Cd'$	
	(+)	(-)	(+)	(-)
1	0.356	-0.506	0.178	-0.116
2	0.364	-0.513	0.177	-0.115
3	0.357	-0.506	0.178	-0.112
4	0.357	-0.506	0.175	-0.115
5	0.355	-0.506	0.176	-0.116
All (25)	0.358	-0.508	0.177	-0.115
Max Error	1.67%	1.05%	0.81%	1.15%

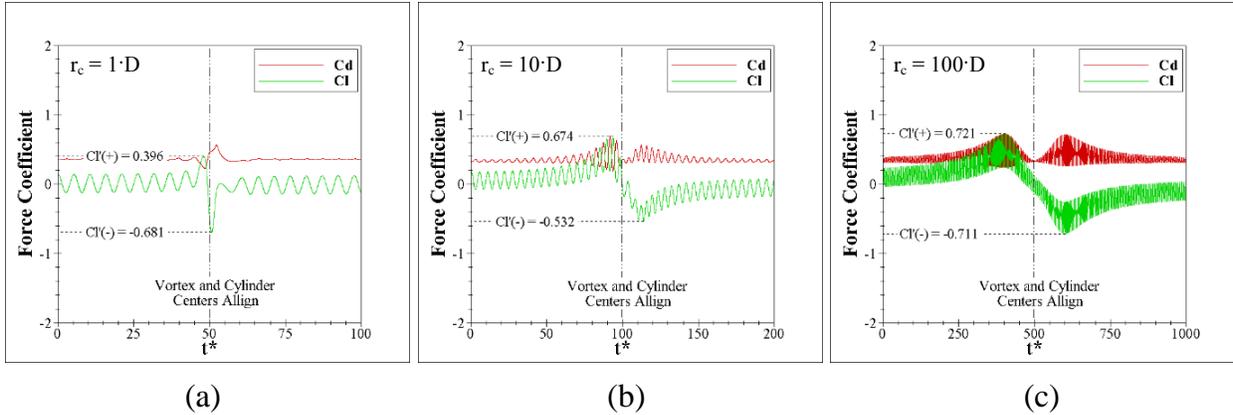
Mean maximum force coefficient amplitudes computed using the 5-TOI data sets are within 1.67% of means computed using all 25 TOI as a data set. This error is small, and the

accuracy of the 5-TOI approximation is sufficient for the present study. Note that all five of the 5-TOI groups yield practically the same mean maximum force coefficient amplitudes. This demonstrates that regardless of the time ( $t^*$ ) that the first vortex impact occurs, four subsequent impacts spaced at  $\Delta t^* \approx 1$  interval are sufficient to establish the mean values of the maximum force coefficient amplitudes.

A procedure has been established to document the trend in maximum cylinder loading amplitude with impinging vortex-to-cylinder size. Specifically, mean values of the maximum cylinder loading amplitude shall be computed from five simulated vortex impacts where the vortex impact time is progressively incremented by  $\Delta t^* = 1$  between simulations. In addition to the mean values of the maximum force coefficient amplitudes, the greatest and least values of each amplitude from the five simulations shall be reported as envelopes to the mean values. This allows the influence of the impinging vortex size on the variation on cylinder loading with time of vortex impact to be documented as well.

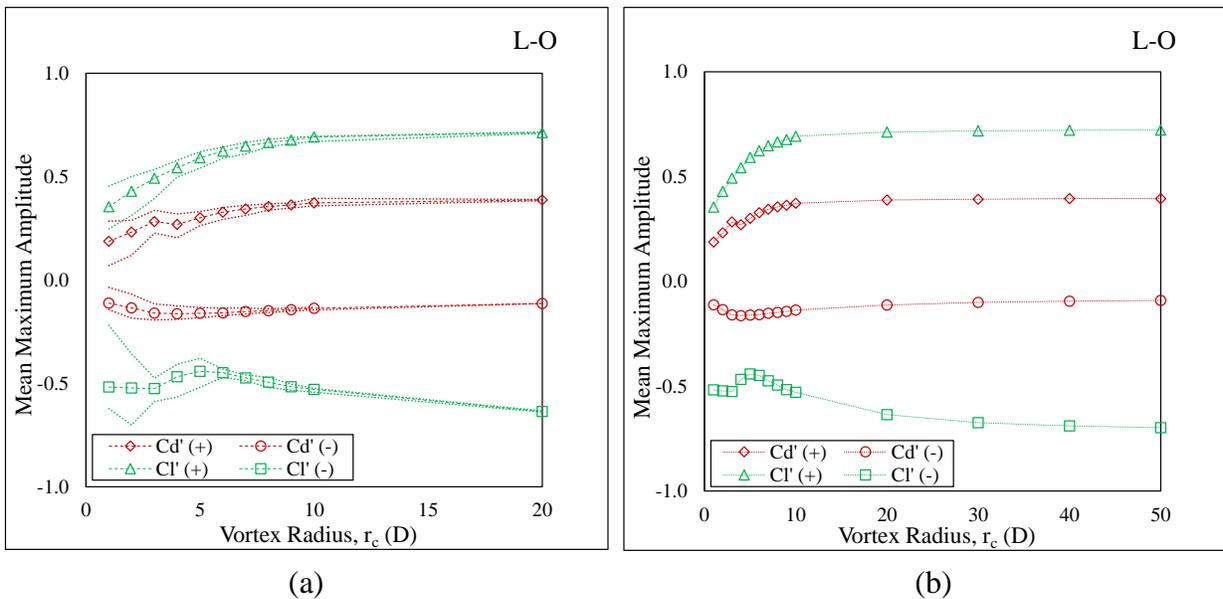
### 5.3.2 Trend in Maximum Cylinder Loading with Impinging Vortex Size

The critical radius of the impinging vortex is incremented from  $r_c = 1 \cdot D$  to  $100 \cdot D$ , and five computer simulations are used to compute mean values for the maximum force coefficient amplitude for each impinging vortex size. Typical force coefficient time histories produced by impinging vortices having critical radii of  $r_c = 1 \cdot D$ ,  $10 \cdot D$ , and  $100 \cdot D$  are illustrated in Figures 5.9a – 5.9c. Generally speaking, the cylinder loading transitions from pulse-like to symmetric.



**Figure 5.9:** Typical time histories for impinging vortex radii (a)  $1 \cdot D$ , (b)  $10 \cdot D$ , and (c)  $100 \cdot D$ .

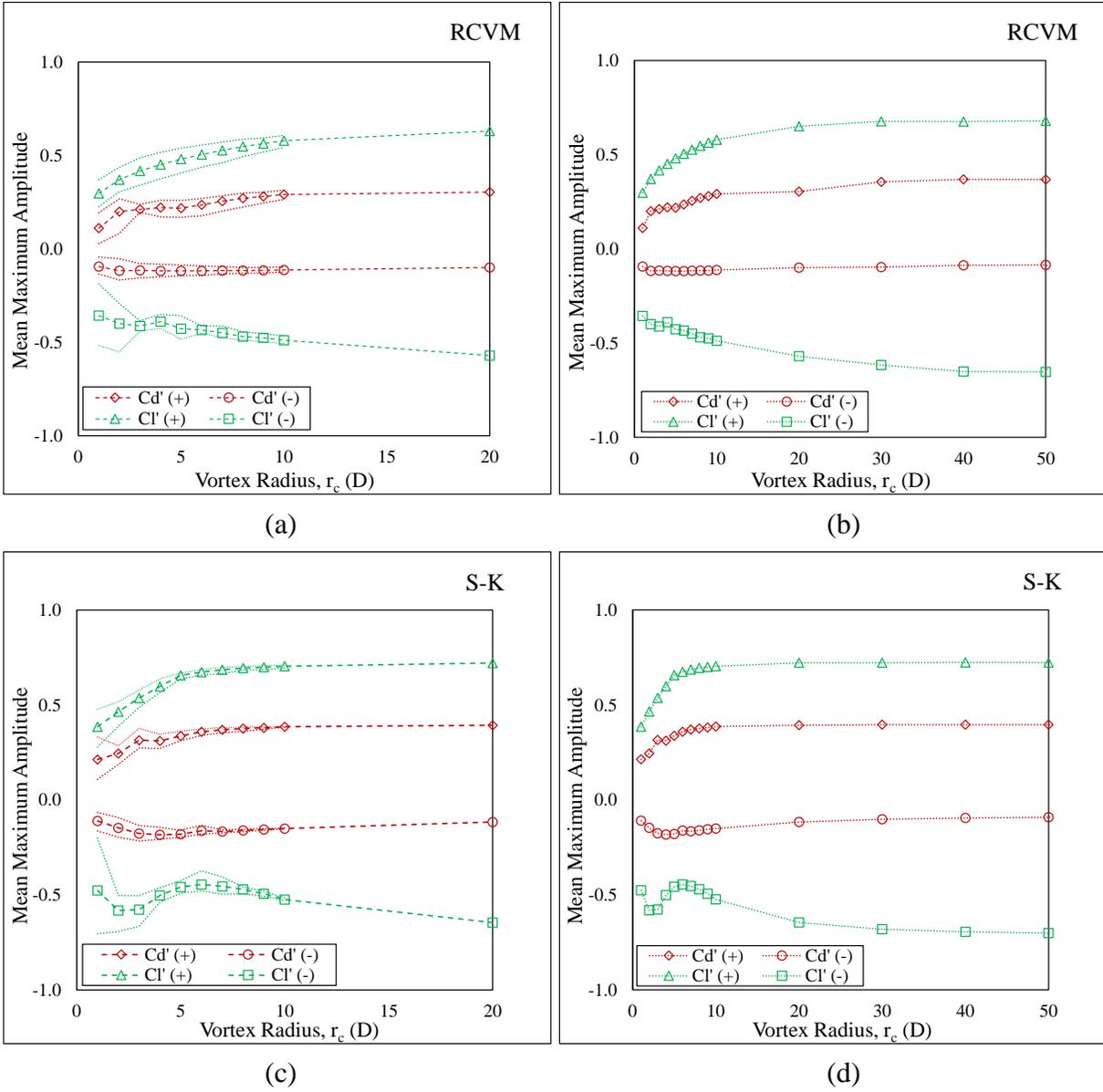
Figures 5.10a and 5.10b summarize the mean maximum force coefficient amplitudes, as well as the variation therein due to vortex impact time, for impinging vortex critical radii of  $r_c = 1 \cdot D$  to  $50 \cdot D$ . Simulations for critical radii of up to  $100 \cdot D$  have been conducted but are not reported herein because maximum force coefficient amplitudes reach asymptotic values for impinging vortex critical radius of approximately  $50 \cdot D$ . Envelopes in Figure 5.10a mark the greatest and least values of each maximum force coefficient amplitude from the five computer simulations and illustrate the variation therein with vortex impact time.



**Figure 5.10:** Mean maximum force coefficient amplitudes from five simulations (envelopes mark the variation with vortex impact time) (a) for  $1 \leq r_c/D \leq 20$  and (b) for  $1 \leq r_c/D \leq 50$ .

The envelopes bounding the mean values of the maximum force coefficient amplitudes in Figure 5.10a narrow rapidly as the impinging vortex size increases. This narrowing is quantified using  $Cl'(-)$ , for which the difference in the greatest and least amplitudes falls from 183.60% to 36.99% to 3.47% for impinging vortex radii of  $1 \cdot D$ ,  $5 \cdot D$ , and  $10 \cdot D$  respectively. The difference in the greatest and least values for all maximum force coefficient amplitudes is less than 1.5% for impinging vortices larger than  $r_c = 20 \cdot D$ . Diminishing variation in cylinder loading amplitude with vortex impact time implies that the impinging vortex controls vortex shedding from the cylinder prior to impacting it. Consequently the vortex impact time with respect to the free stream vortex shedding cycle is no longer influences the cylinder loading amplitude.

The preceding study, which was conducted using the L-O vortex profile, is now repeated using the RCVM and S-K vortex profiles; Figures 5.11a – 5.11d summarize the study findings. Once again, the variation in the maximum force coefficient amplitudes with vortex impact time decays rapidly as the size of the impinging vortex is increased as shown in Figures 5.11a and 5.11c. The maximum force coefficient amplitude mean values for the RCVM and S-K vortex profiles converge to asymptotic values more slowly (Figure 5.11a) and more quickly (Figure 5.11b), respectively, than do the mean maximum force coefficient amplitudes for the L-O vortex profile (Figure 5.10b). The amplitudes produced by the RCVM profile converge more slowly because the TVP is sharper, hence the vortex must be larger to effectively expose the cylinder surface to the maximum tangential velocity. Conversely, the S-K profile's amplitudes become asymptotic more rapidly than the amplitudes from the L-O profile because the S-K profile is flatter and effectively exposes the cylinder surface to the maximum tangential velocity at a smaller size.



**Figure 5.11:** Mean maximum force coefficients and envelopes as function of impinging vortex size for (a-b) RCVM and (c-d) S-K vortex tangential velocity profiles.

Table 5.5 summarizes the asymptotic maximum force coefficient amplitudes for all three impinging vortex profiles. The maximum amplitudes are relatively similar, with the maximum difference in amplitudes computed using different profiles is less than 7.0%. Generally speaking, the maximum amplitude of all force coefficients progressively increases as the vortex TVP transitions from the sharper RCVM profile to the flatter S-K profile.

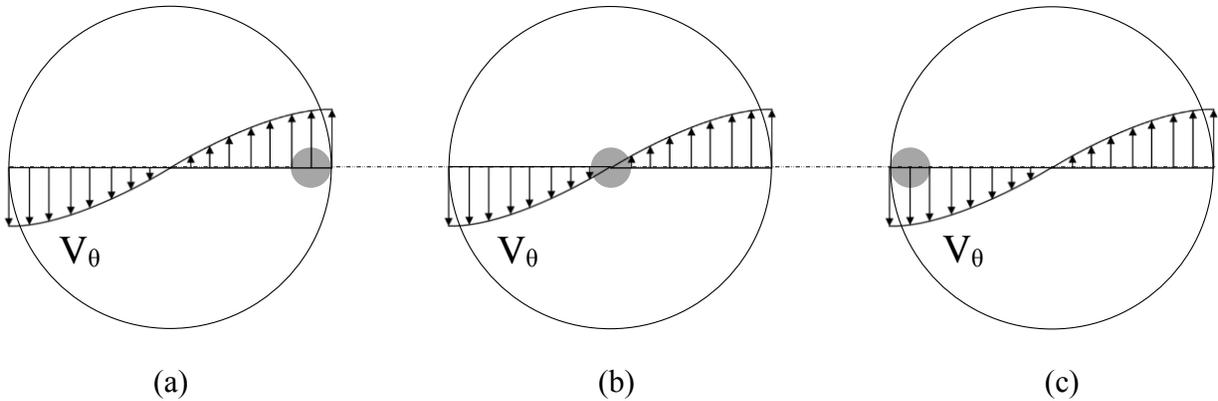
**Table 5.5:** Summary of asymptotic maximum force coefficient amplitudes for RCVM, L-O, and S-K vortex tangential velocity profiles.

TVP	Cd		Cl	
	(+)	(-)	(+)	(-)
RCVM	0.368	-0.085	0.679	-0.652
L-O	0.394	-0.091	0.721	-0.698
S-K	0.396	-0.091	0.723	-0.701

Prior to preceding, it is stressed that the proceeding conclusions regarding the influence of the impinging vortex size on variation in maximum force coefficient amplitude with vortex impact time and convergence of maximum force coefficient amplitudes to asymptotic values are only directly apply to the considered case where  $V_{\theta, \max} = \alpha \cdot r_c = 1$ . Increasing  $\alpha$  increases the sharpness of the vortex profile, and increasing the ratio  $V_{\theta, \max}/U_\infty$  results in the vortex having greater control over vortex shedding from the cylinder prior to impacting it at a smaller size. Consequently, it is likely that maximum force coefficient amplitudes produced by vortices having high  $V_{\theta, \max}/U_\infty$  will become independent of vortex impact time with respect to the free stream VSC at smaller sizes. This is simply a postulation, but the reader as well as subsequent investigators of these topics should at least be aware of how varying system parameters may influence the previously-stated conclusions.

Figures 5.10b, 5.11b, and 5.11d illustrate convergence of mean maximum force coefficient amplitudes to asymptotic values as the vortex's critical radius is increased.  $Cl'(+)$  and  $Cl'(-)$  are the first and last, respectively, to reach their asymptotic values. Figures 5.9a – 5.9c show that  $Cl'(+)$  and  $Cl'(-)$  occur prior to and after the vortex and cylinder centers align. More specifically,  $Cl'(+)$  and  $Cl'(-)$  occur when the cylinder falls within the leading and trailing edges of the vortex respectively. Figures 5.12a and 5.12c respectively illustrate the relative position of the vortex to the cylinder at these times. The vortex's tangential velocity produces maximum upward and downward velocity to the cylinder surface, producing the maximum positive and

negative  $Cl$  amplitudes. The temporal order,  $Cl'(+) then  $Cl'(-)$ , is set by the rotational direction of the impinging vortex which is counter-clockwise (CCW) in the present study.$



**Figure 5.12:** Schematic of the cylinder (a) within the leading edge of the vortex, (b) aligning with the vortex center, and (c) within the trailing edge of the vortex.

As the vortex size is increased, the tangential velocity gradient across the cylinder surface ( $\Delta V_\theta/D$ ) decreases. The positive lift force coefficient maximum amplitude  $Cl'(+)$ ,  $Cd'(+)$ , and  $Cd'(-)$  reach asymptotic values for  $r_c \geq 20 \cdot D$ ; however,  $Cl'(-)$  does not reach its asymptotic value until  $r_c \geq 50 \cdot D$ . It is generally concluded that the convergence of  $Cl'(+)$ ,  $Cd'(+)$ , and  $Cd'(-)$  occurs when  $\Delta V_\theta/D$  becomes sufficiently small.  $Cl'(-)$  requires a larger impinging vortex to reach its asymptotic value because the impinging vortex must be sufficiently large to allow the cylinder's wake to rotate smoothly and without disruption, as shall be illustrated and discussed in the subsequent section.

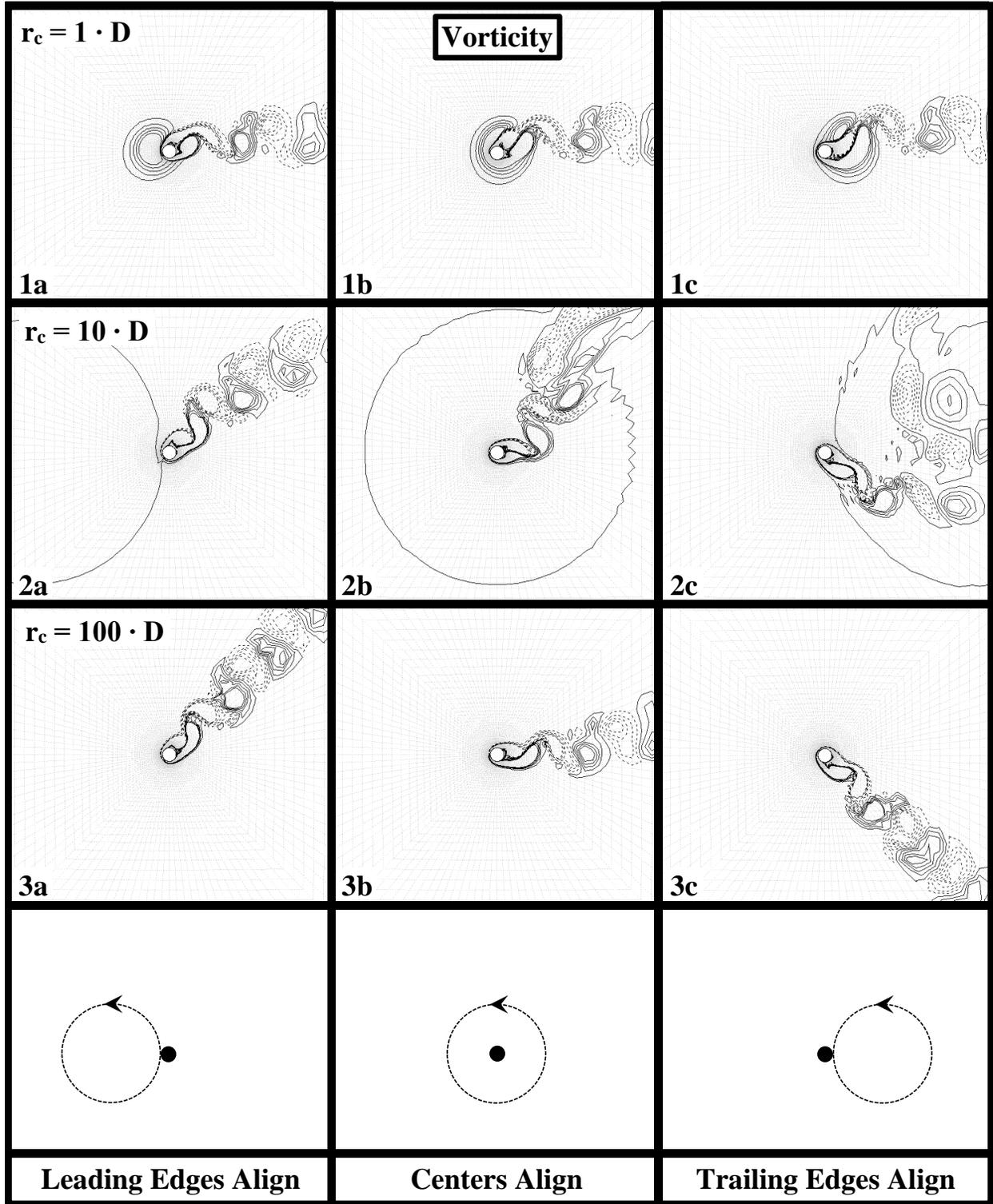
### 5.3.3 Visualization of Cylinder Loading Trend with Increasing Vortex Size

Vorticity contour plots in Figure 5.13 correspond to the force coefficient time histories in Figures 5.10a – 5.10c. Rows (1-3) correspond to impinging vortex radii of  $1 \cdot D$ ,  $10 \cdot D$ , and  $100 \cdot D$ , and columns (a-c) correspond to the three “expected” relative positions of the vortex to the cylinder that are illustrated by the schematics in row 4.

When  $r_c = 1 \cdot D$ , the impinging vortex impacts the cylinder, merges with the CCW vortex attached to the base of the cylinder, and is then shed in the cylinder's wake (Figures 5.13.1a-5.13.1c). The position of the attached vortices is rotated CCW by the impinging vortex, but the cylinder's wake remains horizontal. Vortices shed in the cylinder's wake travel in the direction of the stream over the cylinder, hence, it is evident that the  $r_c = 1 \cdot D$  vortex does not significantly alter vortex shedding from the cylinder prior to impact. The time with respect to the free stream vortex shedding cycle when the impinging vortex impacts the cylinder is significant because the impinging vortex does not control vortex shedding prior to impacting the cylinder hence the structure of the attached vortices will be different for impact at different times.

When  $r_c$  is increased to  $10 \cdot D$ , the cylinder's wake is shifted significantly CCW as the impinging vortex approaches the cylinder (Figure 5.13.2a). The time that the vortex impacts the cylinder with respect to the free stream vortex shedding cycle has little influence on the maximum force coefficient amplitudes because  $r_c$  is sufficiently large for the impinging vortex to control the vortex shedding cycle prior to impacting the cylinder. When the vortex engulfs the cylinder and the cylinder's wake, the velocity gradient within the vortex disrupts the wake (Figure 5.13.2b). The trailing edge of the vortex is distorted significantly due to interaction with the disrupted wake as shown in Figure 5.13.2c. Distortion of the trailing edge of the vortex is the reason  $Cl'(-)$  converges later than the other maximum force coefficient amplitudes.

Finally, when  $r_c$  is  $100 \cdot D$ , the velocity gradient within the vortex is sufficiently small so that the direction of the wake is gradually shifted without disruption (Figures 5.13.3a – 5.13.4c). The vortex is not disrupted by the wake, therefore,  $Cl'(-)$  reaches its asymptotic value. The cylinder's wake is shifted  $45^\circ$  CCW and clockwise (CW) when the cylinder is within the leading (Figure 5.13.3a) and trailing (Figure 5.13.3c) edges of the vortex respectively. This is because



**Figure 5.13:** Vorticity contour plots of the vortex-cylinder interaction for impinging vortex of radius  $1 \cdot D$  (Row 1),  $10 \cdot D$  (Row 2), and  $100 \cdot D$  (Row 3).

$V_{\theta} = U_{\infty}$  and contributes only vertical velocity to the stream over the cylinder at these times, hence the direction of the resultant velocity is  $45^{\circ}$  from the horizontal. When the vortex and cylinder centers align, the net tangential velocity on the cylinder is zero, meaning the flow over the cylinder is the free stream at  $Re = 150$ , which produces a horizontal wake (Figure 5.13.3b).

#### 5.3.4 Summary of Cylinder Loading Trend with Increasing Vortex Size

The radius of the impinging vortex has been incremented from  $1 \leq r_c/D \leq 100$ , and the trend in cylinder loading with respect to impinging vortex size has been documented. For the present system ( $V_{\theta, \max} = U_{\infty} = 1$ ), vortex impact time with respect to the free stream vortex shedding cycle no longer influences maximum force coefficient amplitudes when  $r_c/D \geq 20$ . Maximum force coefficient amplitudes reach asymptotic values for  $r_c/D \geq 50$ , meaning subsequent increase in vortex size does not influence the maximum amplitude of cylinder loading.

It is emphasized that the vortex sizes identified in the previous paragraph only pertain to the specific vortex considered in the study. The broad conclusion of the study is that structure loading by a vortex becomes asymptotic when the three criterion are met:

1. The vortex becomes sufficiently large to control vortex shedding from the structure prior to impacting it.
2. The velocity gradient within the vortex becomes sufficiently small so that the cylinder wake can smoothly shift without being distorted.
3. The vortex becomes sufficiently large to expose the entire surface of the structure to  $V_{\theta, \max}$ .

Numerous parameters such as the vortex translational velocity, the maximum tangential velocity of the vortex, the free stream Reynolds number, etc. may influence the vortex size beyond which vortex impact time ceases to influence the maximum structure loading amplitude and for which maximum force coefficient amplitudes become asymptotic. However, phenomenon which

control the convergence of the maximum force coefficient amplitudes have been identified, and methodology has been established to study the trend in maximum loading of a structure by a vortex as a function of the relative size of the impinging vortex to the loaded structure.

#### **5.4 Influence of Vortex Path on Structural Loading**

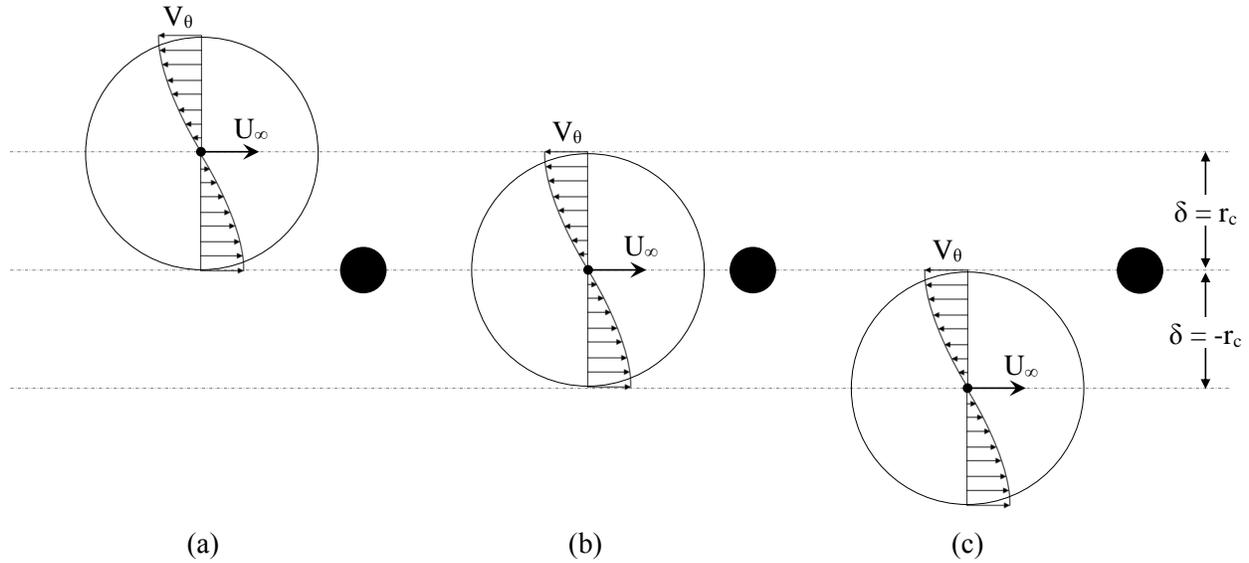
*Section 5.3* establishes that an impinging vortex having critical radius  $r_c = 50 \cdot D$  is large enough to produce asymptotic cylinder loading. To this point, all simulations have been of “direct” vortex impact where the vortex and cylinder centers fall on the same horizontal line. However, an impinging vortex may not always impact the loaded structure directly, so it is necessary to evaluate the influence of y-direction shift of the vortex path on the resulting structural loading. The outcome of this study answers the question of whether or not a rotational flow produces greater loading than an equivalent-velocity stream flow.

##### 5.4.1 Methodology and Approach

A vortex having a critical radius  $50 \cdot D$  is large enough to produce asymptotic cylinder loading, therefore, this fixed vortex size is used for the present study. Direct simulations conducted to this point have  $\delta = 0$  y-direction shift of the impinging vortex path. Now the impinging vortex path is incremented through the interval  $-4 \leq \delta/r_c \leq 4$  so that the impinging vortex travels below and above the cylinder. Maximum positive and negative force coefficient values are extracted following each simulation. These values are then compared with maximum force coefficients produced by a free stream having equivalent maximum velocity to the vortex within the free stream ( $U_{eq} = V_{\theta, max} + U_{\infty}$ ).

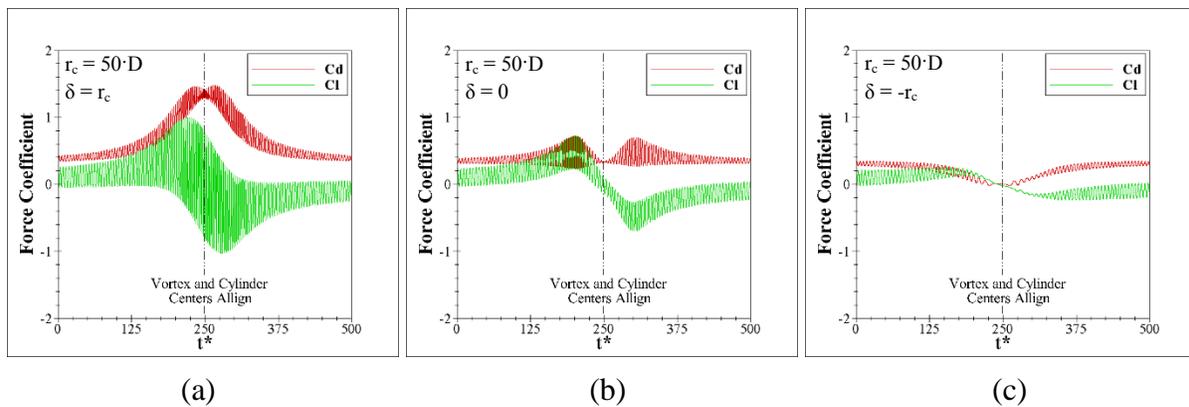
##### 5.4.2 Trend in Structural Loading with Vortex Path Shift

The schematics in Figures 5.14a – 5.14c illustrate the vortex travelling above the cylinder  $\delta = r_c$ , impacting the cylinder directly  $\delta = 0$ , and travelling below the cylinder  $\delta = -r_c$ .



**Figure 5.14:** Schematic of vertical shift of vortex's path by (a)  $\delta = r_c$ , (b)  $\delta = 0$ , and (c)  $\delta = -r_c$ .

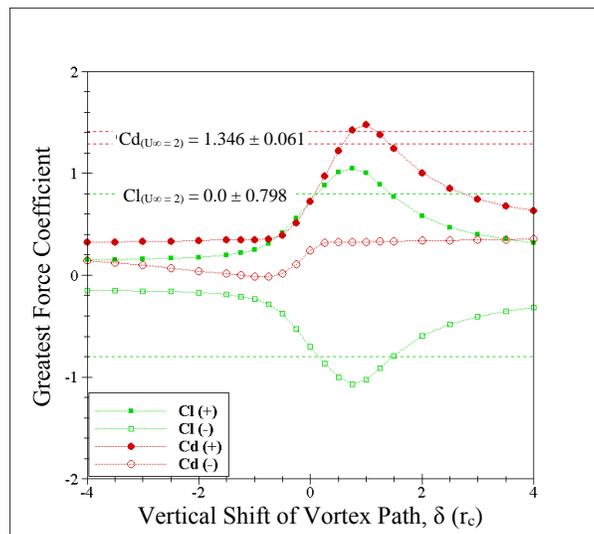
When the vortex impacts the cylinder directly as illustrated in Figure 5.14b, it primarily applies a vertical velocity to the cylinder surface. As the vortex's path is shifted in the  $y$ -direction, it increasingly applies horizontal velocity to the cylinder surface. The present vortex rotates CCW, hence its tangential velocity augments the free stream when it travels above the cylinder (Figure 5.14a) and subtracts from the free stream when it travels below the cylinder (Figure 5.14c).



**Figure 5.15:** Force coefficient time histories for impinging vortex having  $r_c$  of  $50 \cdot D$  and (a)  $\delta = r_c$ , (b)  $\delta = 0$ , and (c)  $\delta = -r_c$ .

Force coefficient time histories corresponding to the three vortex paths illustrated in Figures 5.14a – 5.14c are illustrated in Figures 5.15a – 5.15c. The cylinder loading is much

greater and much less when the vortex travels above (Figure 5.15a) and below (Figure 5.15c) the cylinder, respectively, than when it impacts the cylinder direction (Figure 5.15b). Now the path of the impinging vortex is shifted over the range  $-4 \leq \delta/r_c \leq 4$ , and maximum force coefficients are extracted from each simulation. These are summarized as function of vertical path shift in Figure 5.16 and compared with maximum force coefficients produced by a free stream of equivalent maximum velocity ( $U_{eq} = V_{\theta,max} + U_{\infty}$ ). Note that Figure 5.16 reports maximum force coefficient values rather than maximum force coefficient amplitudes as reported in the preceding Section 5.3.



**Figure 5.16:** Maximum force coefficients for impinging vortex of  $r_c = 50 \cdot D$  and vertical path shift of  $-4 \leq \delta/r_c \leq 4$ . Horizontal lines indicate maximum force coefficients for an  $Re = 300$  equivalent-velocity ( $U = U_{\infty} + V_{\theta,max}$ ).

The summary of maximum force coefficients in Figure 5.16 corroborates the previous statement that the impinging vortex produces greater cylinder loading when it travels above- and less cylinder loading when it travels below- than when it impacts the cylinder directly. The vortex within the free stream produces greater force coefficients than the force coefficients produced when the cylinder is immersed in an equivalent-velocity free stream (Free stream force coefficient are indicated by dashed, horizontal lines). Table 5.6 quantifies and compares the

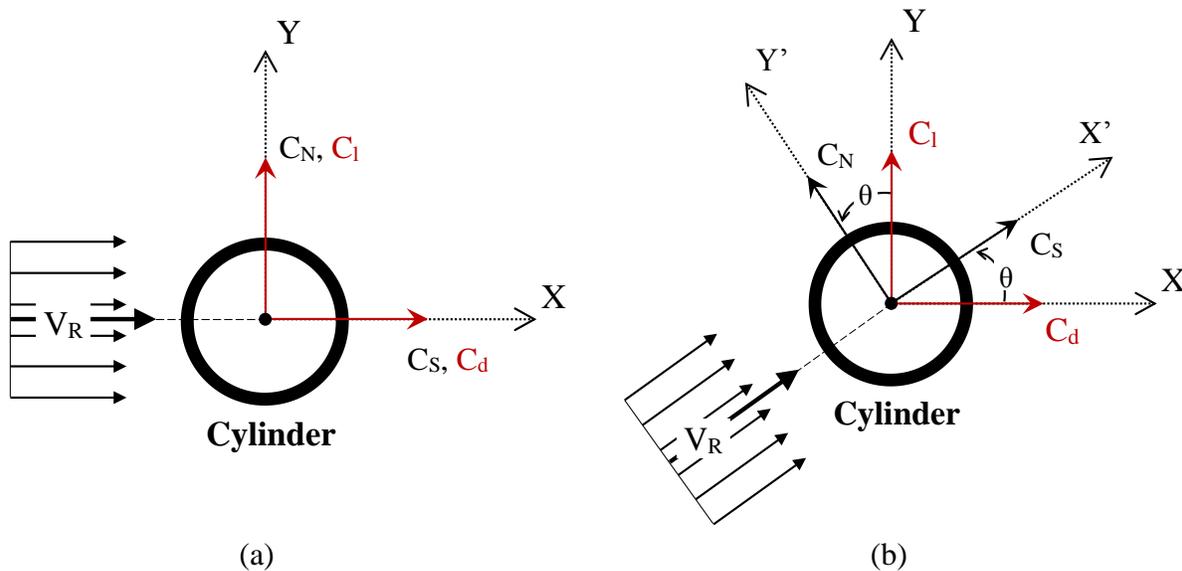
maximum force coefficients produced by the vortex and the free stream. Maximum drag and lift force coefficients produced by the impinging vortex are under-predicted (UP) by the equivalent-velocity free stream force coefficients by up to 4.80% and 34.07% respectively.

**Table 5.6:** Comparison of maximum force coefficients produced by the vortex within the free stream with those produced by a  $Re = 300$  equivalent-velocity free stream.

	Free Stream	Vortex	$\delta (r_c)$	UP
$C_{d_{max}}$	1.407	1.475	1.00	4.80%
$C_{l_{max}}$	0.798	1.069	0.75	34.07%

### 5.4.3 Why the Vortex Produces Greater Loading than the Free Stream

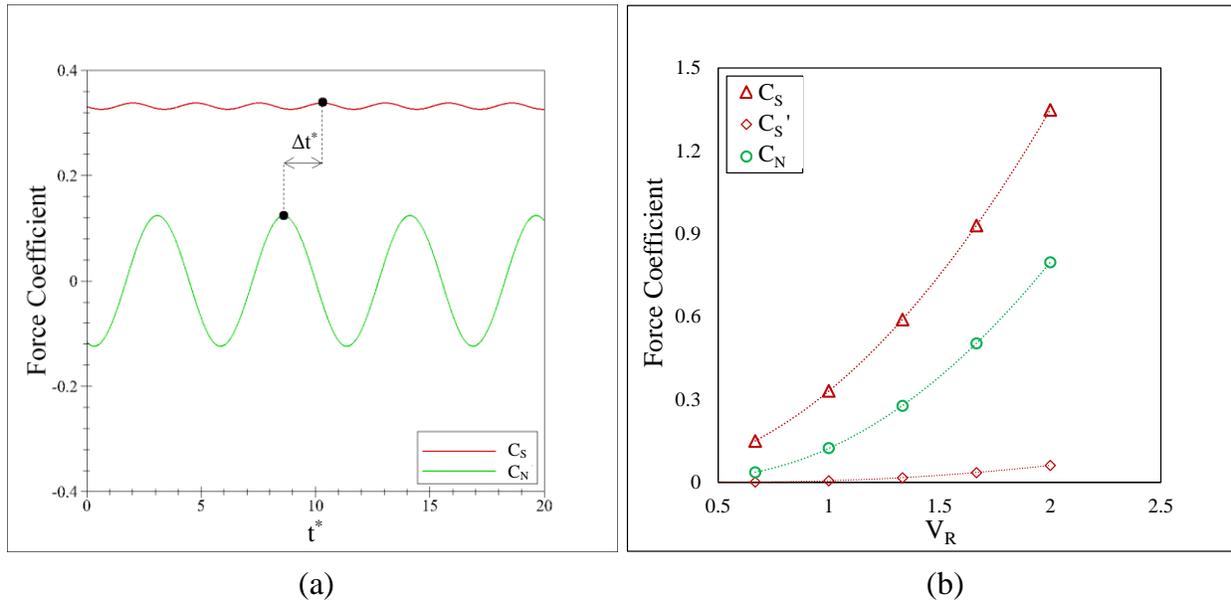
Results from computer simulations reported in the previous subsection demonstrate that a vortex having maximum tangential velocity  $V_{\theta,max}$  travelling within a free stream having velocity  $U_\infty$  can produce greater cylinder loading than an equivalent-velocity  $U_{eq} = V_{\theta,max} + U_\infty$  free stream. This finding is not intuitive because the vortex and the equivalent-velocity free stream both produce the same maximum resultant velocity  $V_R$  on the cylinder; why does the rotational nature of the vortex result in greater cylinder loading? The answer to this question is that both



**Figure 5.17:** Forces produced on a cylinder (a) a free stream and (b) a stream incident from any direction.

the magnitude of  $V_R$  and its angle of incidence  $\theta$  on the cylinder control the maximum cylinder loading amplitude. The forthcoming study expounds this fact to demonstrate that combinations of  $V_R$  and  $\theta$ , rather than just the highest values of  $V_R$ , produce maximum cylinder loading.

Figure 5.17a illustrates a cylinder immersed in a free stream having resultant velocity  $V_R$ . The stream produces stream-wise ( $C_S$ ) and cross-stream ( $C_N$ ) cylinder loading components that respectively act in- and normal to the stream direction. Typical notation is to assign the X-axis along the stream direction and the Y-axis normal to the stream direction; the X-direction force is referred to as drag ( $C_d$ ) and the Y-direction force is referred to as lift ( $C_l$ ).



**Figure 5.18:** (a) Typical free stream force coefficient time histories and (b) free stream force coefficient amplitudes as functions of  $V_R$ . Note that plot (b) is analogous to plotting  $C_d$  and  $C_l$  for Reynolds numbers of 100 to 300, but alternative notation is used here for consistency.

Figure 5.18a is a typical force coefficient time history of cylinder loading by a free stream. The stream-direction force  $C_S$  has a large mean component and small amplitude, while  $C_N$  has mean value of zero and large amplitude. The literature commonly reports  $C_S$  and  $C_N$  (They are referenced  $C_d$  and  $C_l$ ) as function of the free stream Reynolds number. The present study fixes the cylinder diameter and the fluid kinematic viscosity, so the force coefficient

amplitudes are strictly functions of  $V_R$ . Figure 5.18b summarizes force coefficient amplitudes for  $0.67 \leq V_R \leq 2.0$ , which is the range ( $Re \leq 300$ ) for which the present model is valid. Curve fits of the three cylinder loading components illustrated in Figure 5.18b are provided by Eqs. (5.1-3).

$$C_S' = 0.03 \cdot V_R^2 - 0.04 \cdot V_R + 0.01 \quad (5.1)$$

$$C_S = 0.36 \cdot V_R^2 - 0.05 \cdot V_R + 0.03 \quad (5.2)$$

$$C_N = 0.31 \cdot V_R^2 - 0.26 \cdot V_R + 0.07 \quad (5.3)$$

The harmonic component of the stream-wise cylinder loading  $C_S'$  is much smaller than  $C_S$  or  $C_N$  as shown in Figure 5.18b. Furthermore,  $C_S'$  and  $C_N$  are out of phase as illustrated by  $\Delta t^*$  in Figure 5.18a. Inclusion of  $C_S'$  in subsequent discussion is unnecessary and would introduce unnecessary complication. Moving forward, it is therefore assumed that the cylinder loading produced by the free stream consists only of  $C_S$  and  $C_N$ .

When a vortex is added to the free stream,  $V_R$  changes continuously due to the changing proximity of the vortex to the cylinder. Figure 5.17b illustrates the variation by defining the incidence angle ( $\theta$ ) of  $V_R$  as function of its horizontal ( $U$ ) and vertical ( $V$ ) components. As  $\theta$  grows, the directions in which  $C_S$  and  $C_N$  act are shifted around the cylinder surface as indicated by  $X'$  and  $Y'$ . The incident angle  $\theta$  is defined by Eq. (5.4), and Eqs. (5.5) and (5.6) are used to resolve  $C_S$  and  $C_N$  into the drag and lift force coefficients respectively.

$$\theta = \tan^{-1}(V/U) \quad (5.4)$$

$$C_d = C_S \cdot \cos(\theta) - C_N \cdot \sin(\theta) \quad (5.5)$$

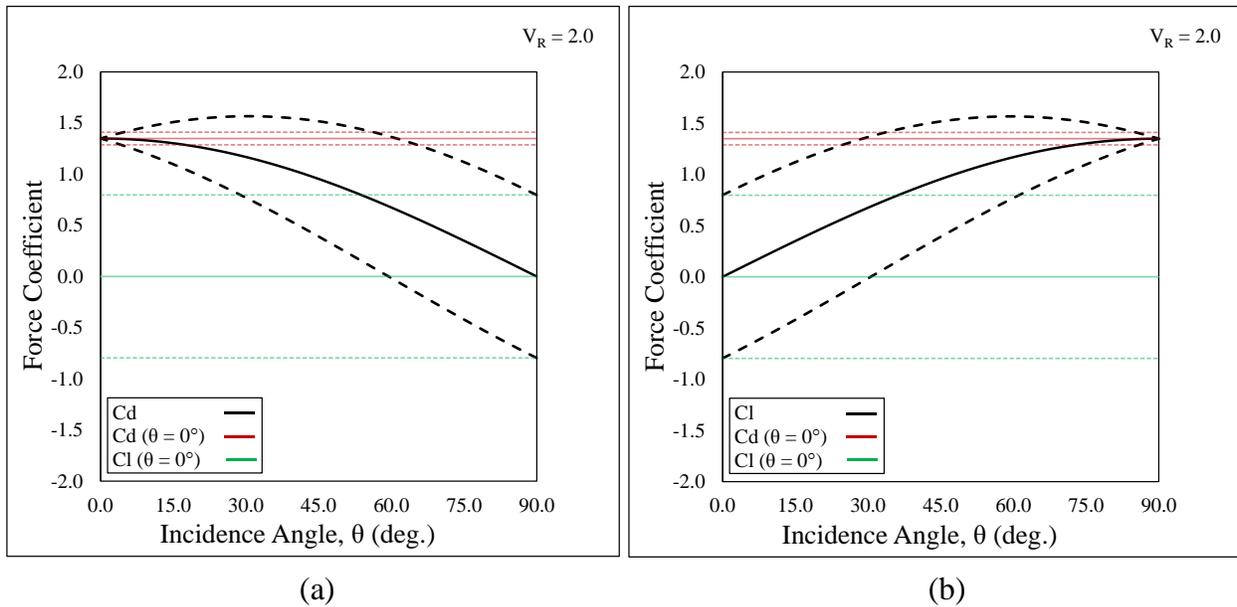
$$C_l = C_S \cdot \sin(\theta) + C_N \cdot \cos(\theta) \quad (5.6)$$

The maximum resultant velocity used in the present study  $V_R = V_{\theta, \max} + U_\infty = 2.0$  is now fixed and used to demonstrate the influence of  $\theta$  on the cylinder loading amplitude. Figures 5.19a and 5.19b respectively illustrate the mean (solid lines) and harmonic (dashed lines) components

of the drag and lift force coefficients as function of  $\theta$ ; the maximum drag and lift force coefficients produced by a  $V_R = 2.0$  and  $\theta = 0^\circ$  free stream are also provided for comparison.

Note that Figure 5.19a and 5.19b replicate each other if the x-axis is reversed.

Figure 5.19a illustrates that the incidence angle range  $4.5^\circ \leq \theta \leq 56^\circ$  produce greater drag force coefficients than are produced by the free stream. The drag force coefficient reaches a maximum value of 1.567, which is 11.12% greater than the maximum free stream drag force coefficient 1.407, for  $\theta = 30.5^\circ$ . Figure 5.19b illustrates that any  $V_R$  incidence angle  $\theta > 0^\circ$  produces greater lift force coefficients than are produced by the free stream. The maximum lift force coefficient value of 1.567, which is 96.60% greater than the free stream value of 0.789, is reached for  $\theta = 59.5^\circ$ .

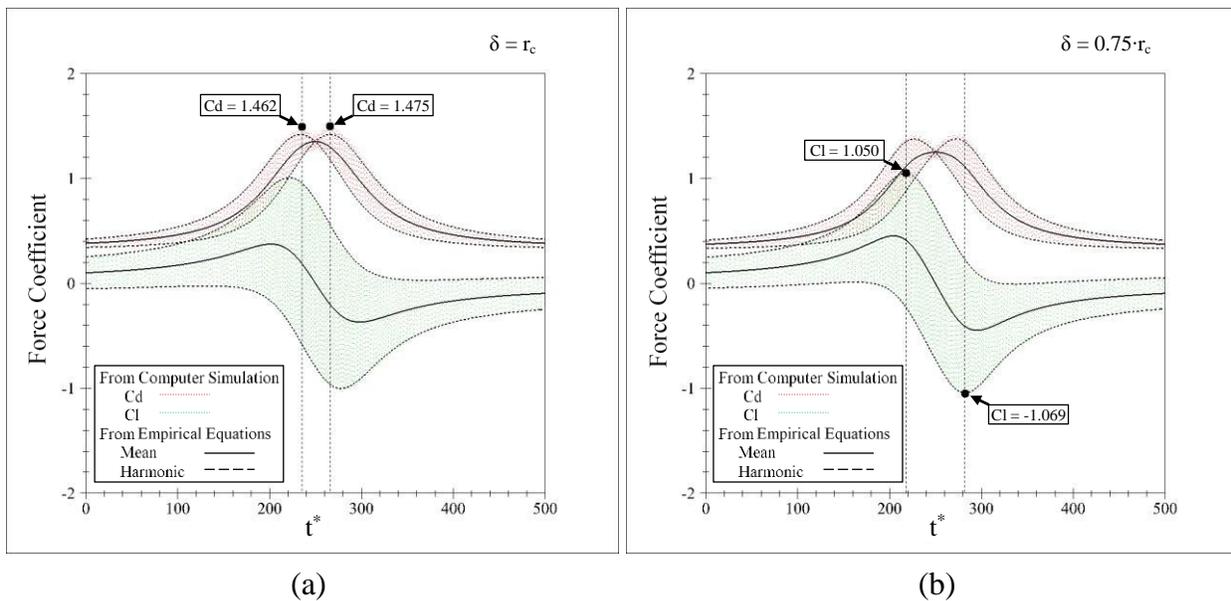


**Figure 5.19:** Maximum (a) drag and (b) lift force coefficient amplitude as a function of resultant velocity incidence angle.

The preceding example demonstrates that, for a fixed  $V_R$ , a range of  $\theta$  produce greater cylinder loading than an equivalent-velocity free stream. However, during vortex loading,  $V_R$  and  $\theta$  change continuously. The stream over the cylinder may be incident at  $\theta$  that produces maximum cylinder loading, such as  $30.5^\circ$  for  $C_d$ , but  $V_R$  may be too low to produce substantial

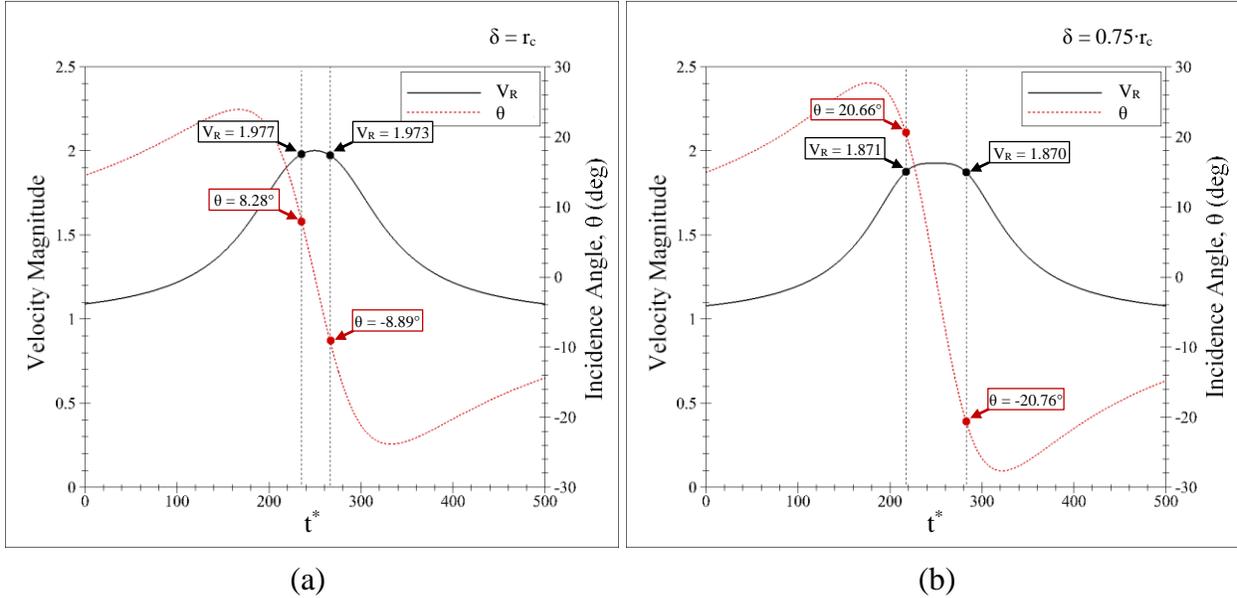
loading. The converse scenario may occur, where  $V_R$  is high, but  $\theta$  is not within the range that produces high force coefficients. The present study validates the use of Eqs. (5.2-6) by comparing maximum cylinder loading defined using known  $V_R$  and  $\theta$  with cylinder loading from the computer simulations which produces maximum force coefficient amplitudes.

The vortex-loading scenarios which produce maximum drag and lift force coefficients are when the vortex path is shifted by  $\delta = r_c$  and  $0.75 \cdot r_c$  respectively. Figures 5.20a and 5.20b compare force coefficient time histories from computer simulation those defined using Eqs. (5.2-6) and demonstrate that the equations provide excellent replication of the cylinder loading from computer simulation. Boxed callouts in both figures indicate the maximum values of the drag and lift force coefficients that occur for the specified loading scenarios.



**Figure 5.20:** Comparison of force coefficient time histories from computer simulation and defined using Eqs. (5.2-6) for impinging vortices travelling at (a)  $\delta = r_c$  and (b)  $0.75 \cdot r_c$ .

Figures 5.21a and 5.21b provide the time histories of  $V_R$  magnitude and  $\theta$  corresponding to the force coefficient time histories in Figures 5.20a and 5.20b. Callouts indicate  $V_R$  and  $\theta$  combinations that produce the maximum force coefficient amplitudes. The maximum cylinder loading is not produced when  $V_R$  is greatest, but rather occurs for maximizing combination of  $V_R$



**Figure 5.21:** Time histories of resultant  $V_R$  magnitude and incidence angle for (a)  $\delta = r_c$  and (b)  $0.75 \cdot r_c$ .

and  $\theta$ . Table 5.7 summarizes the data from Figures 5.20 and 5.21 and compares maximum force coefficients computed using empirical equations Eqs. (5.2-6), based upon  $V_R$  and  $\theta$  combinations indicated in Figure 5.21, with maximum force coefficients from computer simulation that are indicated in Figure 5.20. Empirical Eqs. (5.2-6) reproduce maximum drag and lift force coefficient values from computer simulation to within 4.07% and 2.06% respectively. It is worth noting that if the contribution of the harmonic component of the stream-wise forcing  $C_S'$  is added, the respective values of  $C_{dm,eqs}$  and  $C_{lm,eqs}$  are 1.474 and -1.065, which are within 0.10% and 0.37% of the maximum force coefficients from computer simulation.

The purpose of the preceding study is to explain why a vortex travelling within a free stream can produce greater cylinder loading than a free stream which has equivalent maximum velocity to the vortex within the free stream. It is shown that flow over a cylinder produces forces in the stream direction and normal to the stream direction. The amplitudes of these forces are dependent upon the magnitude of the incident stream velocity  $V_R$ , and the direction in which they act is set by the stream incidence angle  $\theta$ . In summary, the vortex within the free stream

produces greater loading because it is able to produce combinations of  $V_R$  and  $\theta$  that produce greater cylinder loading than a free stream, having the same maximum velocity, at  $0^\circ$  incidence angle.

**Table 5.7:** Comparison of maximum force coefficients computed using Eqs. (5.2-5.6) and maximum force coefficients from computer simulation.

$\delta = r_c$		$\delta = 0.75 \cdot r_c$	
From Equations (5.2-6)			
$V_R$	1.977	$V_R$	1.87
$\theta$	8.28	$\theta$	-20.76
$Cd_{m,eqs}$	1.415	$Cl_{m,eqs}$	-1.047
From Computer Simulation			
$Cd_{m,sim}$	1.475	$Cl_{m,sim}$	-1.069
Percentage Difference			
4.07%		2.06%	

#### 5.4.4 Visualization of Vortex-Cylinder Interaction Influenced by Vortex Path Shift

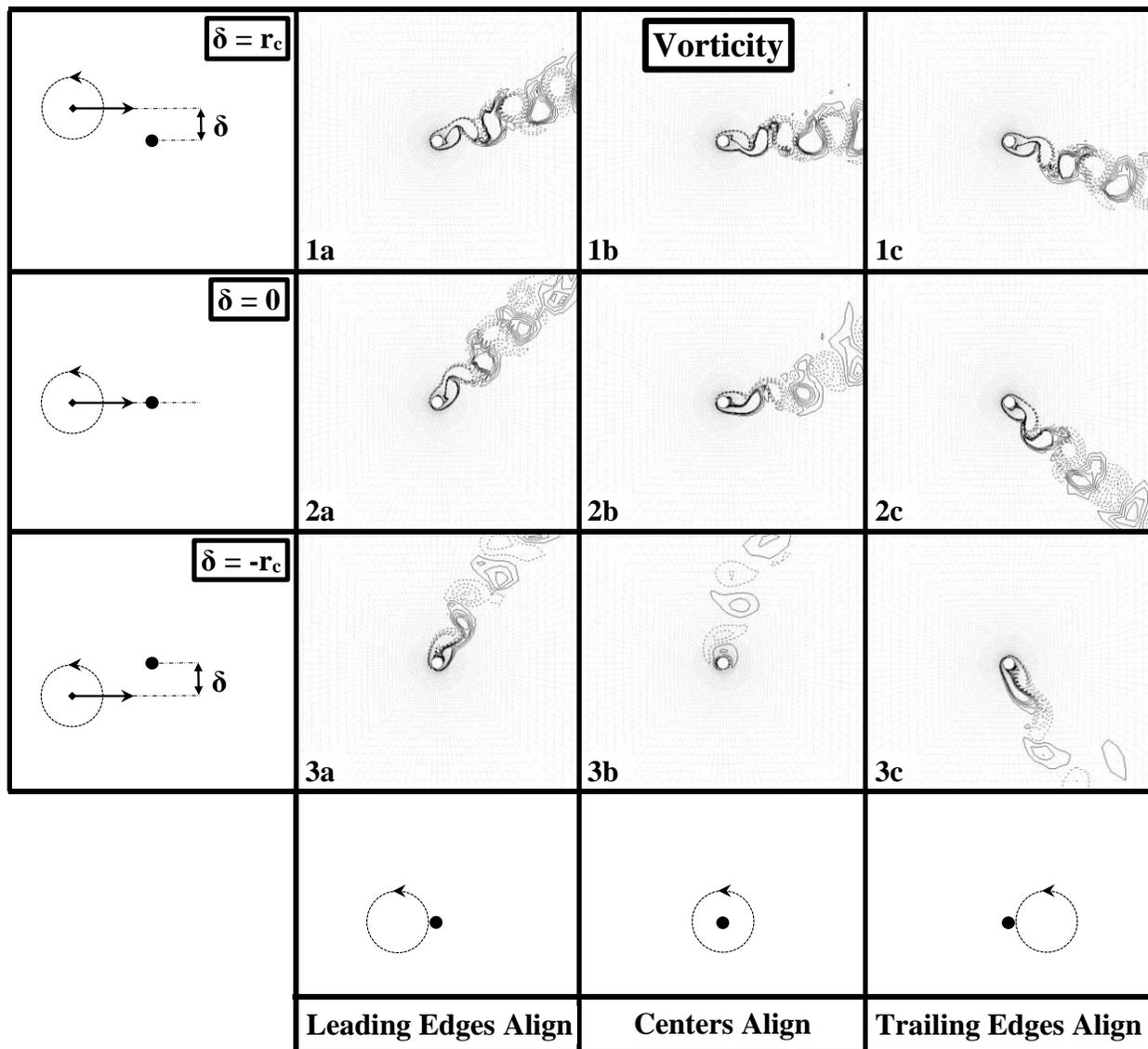
Figure 5.22 uses vorticity contour plots to illustrate the cylinder loading scenarios corresponding to the force coefficient time histories in Figures 5.15a – 5.15c. Rows 1 – 3 respectively illustrate the vortex traveling above the cylinder ( $\delta = r_c$ ), impacting the cylinder directly ( $\delta = 0$ ), and traveling below the cylinder ( $\delta = -r_c$ ). Columns 1 – 3 correspond to the three relative positions of the vortex to the cylinder that are illustrated by the schematics in Row 4.

Cylinder loading is greatest when the impinging vortex travels above the cylinder (See Row 1 of Figure 22). The vortex's tangential velocity adds to and effectively doubles the velocity of the stream passing over the cylinder; this increased velocity produces a large mean drag force and large amplitude lift force. As the vortex approaches the cylinder, its tangential velocity adds a vertical velocity component to the stream over the cylinder as evidenced by the vertical shift of the cylinder's wake in Figures 5.22.1a and 5.22.1c. Consequently, both the

amplitude of the drag force coefficient and the mean value of the lift force coefficient are increased. This is the physical mechanism through which the vortex produces greater loading than the equivalent velocity free stream, as discussed in the previous subsection.

The impinging vortex's tangential velocity primarily adds a vertical velocity component to the stream over the cylinder when the vortex impacts the cylinder directly (See Row 2 of Figure 22). This is shown by 45° counter-clockwise and clockwise shifts in the cylinder wake that are illustrated in Figures 5.22.2a – 5.22.2c respectively. The force coefficient time history in Figure 5.15b shows that the amplitude of  $C_d$  and mean value of  $C_l$  are high for direct vortex impact. However, the maximum resultant velocity is relatively low because the vortex's tangential velocity primarily adds a vertical velocity component to the free stream; consequently, cylinder loading is less than that produced by the equivalent-velocity free stream.

Cylinder loading by the impinging vortex is least when the impinging vortex travels below the cylinder as illustrated in Row 3 of Figure 5.22. The vortex rotates counter-clockwise, hence its tangential velocity effectively counteracts the loading effects produced by the stream that it travels within. Vortex shedding from the cylinder slows and eventually stops as the Reynolds number characterizing the flow over the cylinder falls below  $Re_{cr} \approx 47$ . This is indicated by the non-periodic segment of the force coefficient time histories centered around  $t^* = 250$  in Figure 5.15c. The reduced vortex shedding frequency can be seen when comparing the vorticity contours for  $\delta = -r_c$  (Figures 5.17.3a – 5.17.3c) with those for  $\delta = 0$  and  $\delta = r_c$ .



**Figure 5.22:** Vorticity contour plots of the vortex-cylinder interaction for impinging vortex of radius  $50 \cdot D$  and vertical offset of  $\delta = r_c$  (Row 1),  $\delta = 0$  (Row 2), and  $\delta = -r_c$  (Row 3).

## 5.5 Summary and Conclusions

Structural loading by vortices occurs at practically all relative vortex-to-structure size scales. Direct impact of a vortex with a slender, cylindrical structure is simulated to evaluate the influence of the relative vortex-to-structure size on the structural loading amplitude that is produced. The vortex's maximum tangential velocity and the velocity of the free stream that it travels within are fixed and equivalent ( $V_{\theta, \max} = U_{\infty}$ ). The vortex's critical radius is incremented

from one to one-hundred times the structure's diameter, and maximum force coefficient amplitudes used to illustrate the structural loading amplitude trend. Phenomenon which influence the structural loading trend are illustrated and explained using velocity and vorticity contour plots. Finally, the impinging vortex's path is shifted from the structure's centerline so that it travels along paths both above and below the structure. Maximum force coefficient values are utilized to document the influence of vortex path shift  $\delta$  on the structural loading. Maximum force coefficients produced by the vortex are compared with maximum force coefficients produced by the equivalent velocity free stream to assess the influence of the rotational nature of the vortex on the maximum structural loading that it can produce. Based upon the content presented in Chapter 5, the subsequently-outlined conclusions have been reached.

1. A small vortex produces a wide spectrum of different structural loading amplitudes when it impacts a structure that is already shedding vortices at different times with respect to the vortex shedding cycle.
  - a. The actual maximum force coefficient amplitudes that an  $r_c = 1 \cdot D$  vortex produces may be under-predicted by 84.99%, 197.04%, 310.42%, and 407.15% [ $Cl'$  (+),  $Cl'$  (-),  $Cd'$  (+), and  $Cd'$  (-)] if only a single simulation is performed.
  - b. The variation in structural loading with vortex impact time is caused by interaction between the impinging vortex and different attached vortices.
2. The variation in maximum force coefficients with vortex impact time is negligible for  $r_c \geq 20 \cdot D$  because large vortices control vortex shedding from the structure pre-impact.
3. Maximum force coefficient amplitudes produced by a directly-impacting, fixed-maximum tangential velocity vortex are asymptotic for  $r_c \geq 50 \cdot D$ .
  - a. The impinging vortex must be large enough to:

- i. Control vortex shedding from the structure prior to impacting it.
  - ii. Have a sufficiently small tangential velocity gradient to allow the structure's wake to shift without distortion.
  - iii. Be large enough to effectively expose the entire structure to the maximum tangential velocity.
- 4. Maximum drag and lift force coefficients produced by impacting  $r_c \geq 50 \cdot D$  vortices are respectively 94.77% and 10.66% less than force coefficients produced by the equivalent-velocity free stream.
  - a. Forces produced on the structure are a function of the square of the incident resultant velocity. The maximum stream velocity over the structure for direct vortex impact is horizontal  $U_\infty = 1$  and vertical  $V_{\theta, \max} = 1$ , so the maximum resultant velocity on the structure is  $(2)^{0.5}$  which is much less than the  $U_\infty = 2$  free stream's resultant velocity  $(4)^{0.5}$ .
- 5. The impinging vortex's tangential velocity profile has little influence on maximum force coefficients produced by the impacting vortex.
  - a. The Vatistas  $n = 1$  vortex produces the greatest maximum force coefficient amplitudes, and maximum force coefficient amplitudes produced by the Vatistas  $n = 1$  and Vatistas  $n = 100$  respectively are within 1.0% and 7.0% less.
- 6. The CCW-rotating, impinging vortex respectively produces greater and less structural loading when it travels above and below the structure than when it impacts directly because its tangential velocity respectively augments and subtracts from the free stream velocity.

7. The CCW-rotating impinging vortex produces maximum drag and lift force coefficients that are respectively 3.80% and 34.07% greater than maximum force coefficients produced by the equivalent-velocity free stream.

## CHAPTER 6: DYNAMIC AMPLIFICATION OF TORNADO WIND LOADS

### 6.1 Overview

Atmospheric vortices such as tornados and hurricanes are transient phenomenon, and the capability of their induced wind loads to dynamically excite structures is debated. Hurricane vortices translate slowly and are many kilometers in diameter, hence it can reasonably be inferred that they are incapable of dynamically exciting a practical structure. However, tornado vortices range greatly in size and may translate rapidly. Consequently, the capability of tornados to dynamically amplify structure response needs to be systematically assessed.

The literature contains several investigations of the capability of tornado wind loads to dynamically excite structures. The time history of the tornado wind speed on the structure is defined, and then empirical equations are used define the corresponding forcing time history. The forcing is then applied to various single- and multi-story structures, producing dynamic structure response amplification of 1.735 (Dutta et al., 2002) to 4.0 (Wen, 1975). However, their simulations are performed using forcing time histories developed for straight-line wind, so the aerodynamic effects of the structures' shape and the rotating vortex wind field are absent.

All preceding investigations report than tornado wind loads are capable of dynamically amplifying the loaded structure's response. However, their collective shortcoming is that they fail to develop a generalized assessment of the dynamic loading threat from tornados, but rather assess the response of a particular structure to loading by a particular tornado. Parameters of tornado vortices as well as real-world structure fundamental periods vary widely. The fundamental period of a structure can be computed with reasonable accuracy during design; computation of adequate design loadings necessitates the development of a methodology to assess the possible dynamic amplification of the structure's response to tornado wind loads.

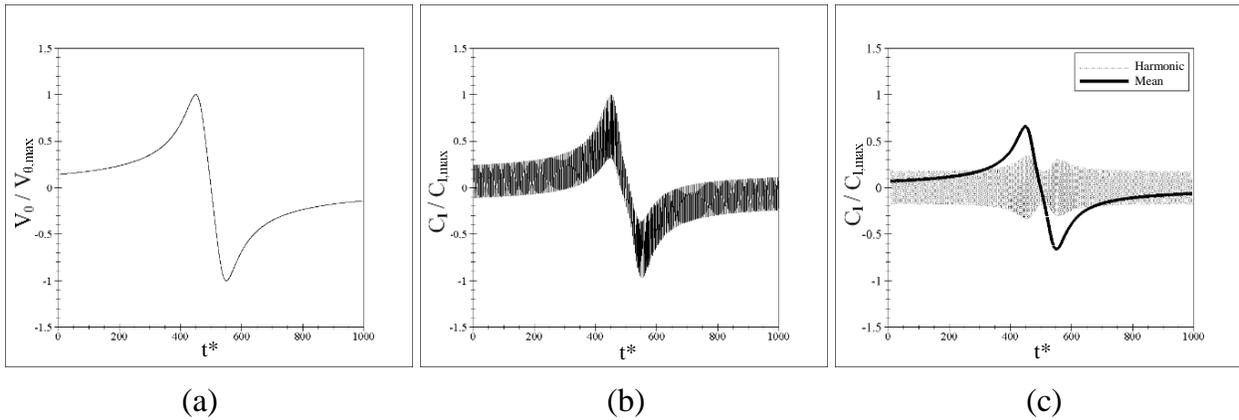
The overarching goal of this chapter is to define the possible dynamic amplification of a structure having specified fundamental period  $T_n$  to tornado wind loads. The dynamic load factor (DLF) concept is used to generalize computation of the dynamic structure response amplification because a Fourier transform of the forcing is unable to distinguish the vortex loading frequency  $f_v$  as illustrated in Figure 6.16 and discussed in Section 6.5.5. The subsequent sections provide general discussion of tornado wind loads prior to introducing and explaining the DLF concept. Construction of generalized DLF curves for an applied forcing profile is demonstrated. The DLF concept is then extended to define the vortex loading period  $T_v$  as a function of the vortex's parameters. Finally, documented tornado vortex parameters are used to define the possible dynamic response amplification for a structure having specified  $T_n$ .

## **6.2 The Vortex's Tangential Velocity**

As a vortex crosses a structure, its tangential velocity rapidly changes the sign and magnitude of the component of the velocity incident on the structure normal to the vortex's path. Consequently, the cross-stream or lift force is the primary dynamic loading concern ((Tan, 1975) and (Seniwongse, 1977)). Numerous studies ((Wen, 1975), (Tan, 1975) and (Seniwongse, 1977)) also concur that the dynamic loading threat is greatest when the vortex impacts the cylinder directly because this case produces greatest variation in the cross-stream wind field. Therefore, the present study uses lift force coefficient time histories from direct impact of the cylinder by the vortex as the vortex wind loading.

Figure 6.1a illustrates the time history of the cross-stream velocity component incident on the cylinder surface as the impinging vortex passes over it. The corresponding lift force coefficient time history is illustrated in Figure 6.1b, and Figure 6.1c illustrates the lift force coefficient decomposed into its mean and harmonic components. The mean component of the lift

force coefficient is produced by surface shear and pressure due to the incident vortex wind field, and the harmonic component is produced by vortex shedding from the cylinder. Vortex shedding rarely occurs when wind speeds exceed 15 m/s due to excessive turbulence in the air; furthermore, gustiness and wind-field variation in tornados prevent vortex shedding (Giosan and, Eng, 2013). Therefore, the present study defines the vortex forcing profile as the mean component of  $C_l$ .



**Figure 6.1:** Time histories of (a) cross-stream velocity component, (b) lift force coefficient, and (c) decomposed lift force coefficient.

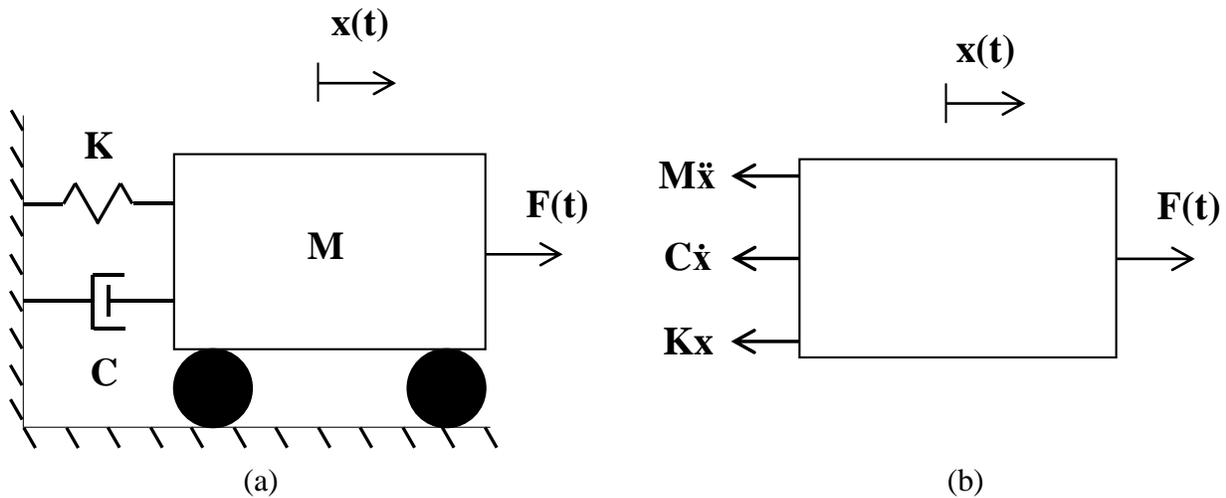
The decomposition of  $C_l$  illustrated in Figure 6.1c is performed by curve fitting the upper and lower bounds of  $C_l$  illustrated in Figure 6.1b and using them to define their mean, which is then subtracted to get the harmonic component. This procedure provides sufficient accuracy when the impinging vortex is large ( $r_c = 50 \cdot D$  for Figure 6) so  $C_l$  does not change substantially between successive peaks of the harmonic component. However, this curve-fitting procedure becomes increasingly-erroneous as the impinging vortex size becomes smaller ( $r_c \leq 10 \cdot D$ ), and it is not possible to simply extract the mean lift force component to analyze as forcing.

The vortex's proximity to the cylinder continuously changes the velocity of the stream over the cylinder, consequently, the frequency and amplitude of the harmonic component of  $C_l$  change as well. This means Fourier analysis cannot be used to identify and allow removal of the

harmonic component of CI. Therefore, the forthcoming study applies the entirety of CI as forcing to a single degree of freedom (SDOF) response model, and uses the dimensionless dynamic load factor (DLF) to identify the associated vortex loading period  $T_v$ . The duration  $T_v$  is defined as the fundamental structure period  $T_n$  for which the DLF reaches a peak value. The subsequent section introduces and develops the SDOF response model used throughout the following study and outlines the computation of the DLF.

### 6.3 Single Degree of Freedom Response Model

The SDOF system modeled herein is the spring-mass-damper system illustrated in Figure 6.2a, and the corresponding free-body diagram is illustrated in Figure 6.2b. The system's equation of motion Eq. (6.1) is attained by summing the x-direction forces acting on the system. Eq. (6.1) is written in Newtonian notation, where dots imply differentiation with respect to time. Variables  $M$ ,  $K$ , and  $C$  are the mass, stiffness, and damping coefficient of the cylinder respectively. Displacement and forcing on the system are denoted  $x(t)$  and  $F(t)$  respectively.



**Figure 6.2:** (a) Physical representation of the SDOF response model and (b) free body diagram of the SDOF system.

$$\sum F_x \rightarrow M\ddot{x} + c\dot{x} + Kx = F(t) \quad (6.1)$$

### 6.3.1 Reduction of System Order

The two subsequent subsections respectively outline the development of dimensional and dimensionless single degree of freedom response models. The dimensional model is utilized through Section 6.4 because the forcing is defined by the user and has units of force. The dimensionless model is utilized from Section 6.5 because dimensionless force coefficients from computer simulation are applied as forcing.

#### **6.3.1.1 Dimensional Response Model**

The SDOF systems position, velocity, and acceleration are solved at each time step using Matlab's ® ODE 45 solver. ODE 45 solves systems of first order, ordinary differential equations (ODE's), hence Eq. (6.1) must be reduced from a second order ODE to two first order ODEs. However, it is convenient to first rewrite Eq. (6.1) in terms of the dimensionless damping ratio  $\zeta$  and the system's angular frequency  $\omega_n$  which are respectively defined in Eqs. (6.2) and (6.3). Substitution of  $\zeta$  and  $\omega_n$  into Eq. (6.1) produces Eq. (6.4) which is the convenient form of the SDOF system's EOM that will be used herein.

$$\zeta = C \cdot (2 \cdot M \cdot \omega_n)^{-1} \quad (6.2)$$

$$\omega_n = \sqrt{K \cdot M^{-1}} \quad (6.3)$$

$$\ddot{x} + 2\zeta\omega_n\dot{x} + \omega_n^2x = F(t) \cdot M^{-1} \quad (6.4)$$

The second order EOM Eq. (6.4) is reduced to two first order ODE's by introducing state variables  $q_1$  and  $q_2$ , which respectively represent the displacement and velocity of the SDOF system and are defined in Eqs. (6.5a) and (6.5b).

$$q_1 = x \quad \text{and} \quad q_2 = \dot{x} \quad (6.5a)$$

$$\dot{q}_1 = \dot{x} \quad \text{and} \quad \dot{q}_2 = \ddot{x} \quad (6.5b)$$

The definitions of the state variables are now introduced into the equation of motion Eq. (6.4), and the equation of motion is solved for the acceleration term  $\ddot{x}$  as shown in Eq. (6.6a).

Recognizing that  $q_2$  is the time derivative of  $q_1$  as defined in Eq. (6.6b), the system of Eqs. (6.6a) and (6.6b) can now be solved simultaneously at each time step to resolve the SDOF system's position, velocity, and displacement.

$$\dot{q}_2 = -2\zeta\omega_n q_2 - \omega_n^2 q_1 + F(t) \cdot M^{-1} \quad (6.6a)$$

$$\dot{q}_1 = q_2 \quad (6.6b)$$

Finally, Eqs. (6.6a) and (6.6b) are placed in the vector format that is taken by Matlab's ODE 45 solver as shown in Eq. (6.7).

$$\bar{f}(\bar{q}(t), t) = \begin{bmatrix} \dot{q}_1 \\ \dot{q}_2 \end{bmatrix} = \begin{bmatrix} 0 & 1 \\ -\omega_n^2 & -2\zeta\omega_n \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} + \begin{bmatrix} 0 \\ F(t)/M \end{bmatrix} \quad (6.7)$$

### 6.3.1.2 Dimensionless Response Model

The computer model utilized in this study is nondimensionalized, and the structural loading produced by vortex impact with the structure is computed as dimensionless force coefficients rather than dimensional forces. It is convenient to re-dimension the state variable vector given in Eq. (6.7) into a dimensionless format so that the force coefficients from the computer model can directly applied as forcing. The dimensionless groups proposed in Selvam and Govindaswamy (2001) are utilized herein.

The dimensional equation of motion for the SDOF system is defined previously in Eq. (6.4). Dimensionless position  $x^*$  and time  $t^*$  are defined in Eqs. (6.8) and (6.9) respectively.

$$x^* = x \cdot D^{-1} \quad (6.8)$$

$$t^* = t \cdot U_\infty \cdot D^{-1} \quad (6.9)$$

Dimensionless velocity  $\dot{x}^*$  and acceleration  $\ddot{x}^*$  are respectively defined in Eqs. (6.10) and (6.11) by taking the first and second derivatives of  $x^*$  with respect to  $t^*$ .

$$\dot{x} = \frac{dx}{dt} = \frac{d(D \cdot x^*)}{d(t^* \cdot D \cdot U_\infty^{-1})} = U_\infty \cdot \dot{x}^* \quad (6.10)$$

$$\ddot{x} = \frac{d^2x}{dt^2} = \frac{d}{dt} \left( \frac{dx}{dt} \right) = \frac{dt^*}{dt} \cdot \frac{d}{dt^*} \left( \frac{d(D \cdot x^*)}{dt^*} \cdot \frac{dt^*}{dt} \right) = \frac{U_\infty^2}{D} \cdot \ddot{x}^* \quad (6.11)$$

Now the definitions of  $x^*$ ,  $\dot{x}^*$ , and  $\ddot{x}^*$  are substituted into Eq. (6.4). After multiplying through by  $D \cdot U_\infty^{-2}$  to simplify the expression, the equation of motion Eq. (6.4) is redefined as Eq. (6.12).

$$\ddot{x}^* + \frac{2\zeta\omega_n D}{U_\infty} \dot{x}^* + \frac{\omega_n^2 D^2}{U_\infty^2} x^* = \frac{F(t)D}{MU_\infty^2} \quad (6.12)$$

Dimensional force  $F(t)$  is replaced with the dimensionless force coefficient definition  $C_F(t) = 2F(t) \cdot (\rho A U_\infty^2)^{-1}$ . Mass and density are grouped into the dimensionless mass term  $R_m$  defined in Eq. (6.13). Substitution of the definitions of  $C_F(t)$  and  $R_m$  into Eq. (6.12) produces the equation of motion defined in Eq. (6.14).

$$R_m = \rho \cdot D \cdot A \cdot M^{-1} \quad (6.13)$$

$$\ddot{x}^* + \frac{2\zeta\omega_n D}{U_\infty} \dot{x}^* + \frac{\omega_n^2 D^2}{U_\infty^2} x^* = \frac{C_F(t)R_m}{2} \quad (6.14)$$

Finally, the dimensional reference velocity  $U_\infty$  is eliminated by introducing the dimensionless reference velocity  $U_\infty^*$  which is defined in Eq. (6.15). The dimensionless reference velocity  $U_\infty^*$  is substituted into Eq. (6.14) yielding the fully dimensionless equation of motion for the SDOF system Eq. (6.16).

$$U_\infty^* = U_\infty \cdot (\omega_n \cdot D)^{-1} \quad (6.15)$$

$$\ddot{x}^* + \frac{2\zeta}{U_\infty^*} \dot{x}^* + \frac{1}{U_\infty^*} x^* = \frac{C_F(t)R_m}{2} \quad (6.16)$$

The second order, dimensionless equation of motion Eq. (6.16) is now reduced to two first order ODE's using the state variables that are respectively defined as  $q_1$  and  $q_2$  in Eqs. (6.17) and (6.18).

$$q_1 = x^* \quad \text{and} \quad q_2 = \dot{x}^* \quad (6.17)$$

$$\dot{q}_1 = \dot{x}^* \quad \text{and} \quad \dot{q}_2 = \ddot{x}^* \quad (6.18)$$

Equations (6.17) and (6.18) are now placed in the vector format required for Matlab's ODE 45 solver. The resulting expression Eq. (6.19) is the dimensionless state representation of the dimensioned equation of motion Eq. (6.7) and allows direct application of force coefficient time histories from computer simulation  $C_F(t)$  as forcing.

$$\bar{f}(\bar{q}(t), t) = \begin{bmatrix} \dot{q}_1 \\ \dot{q}_2 \end{bmatrix} = \begin{bmatrix} 0 & 1 \\ -U_\infty^* & -2\zeta U_\infty^* \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} + \begin{bmatrix} 0 \\ 0.5C_F(t)R_m \end{bmatrix} \quad (6.19)$$

### 6.3.2 RK4 Formulation and Validation

The SDOF system response is solved via fourth order Runge-Kutta (RK4) at each time step. Four weighted terms are used to approximate the state variables (Specifically displacement and velocity of the cylinder) at each time step. The “k” terms or “Kutta coefficients” are defined in Eqs. (6.20-23) and are intermediate solutions to  $\bar{f}(\bar{q}(t), t)$  at each time step. The state variables are computed using the weighted summation of Kutta coefficients defined Eq. (6.24).

$$\bar{k}_1 = \bar{f}(\bar{q}^t, t) \quad (6.20)$$

$$\bar{k}_2 = \bar{f}(\bar{q}^t + \Delta t/2 \cdot \bar{k}_1, t + \Delta t/2) \quad (6.21)$$

$$\bar{k}_3 = \bar{f}(\bar{q}^t + \Delta t/2 \cdot \bar{k}_2, t + \Delta t/2) \quad (6.22)$$

$$\bar{k}_4 = \bar{f}(\bar{q}^t + \Delta t \cdot \bar{k}_3, t + \Delta t) \quad (6.23)$$

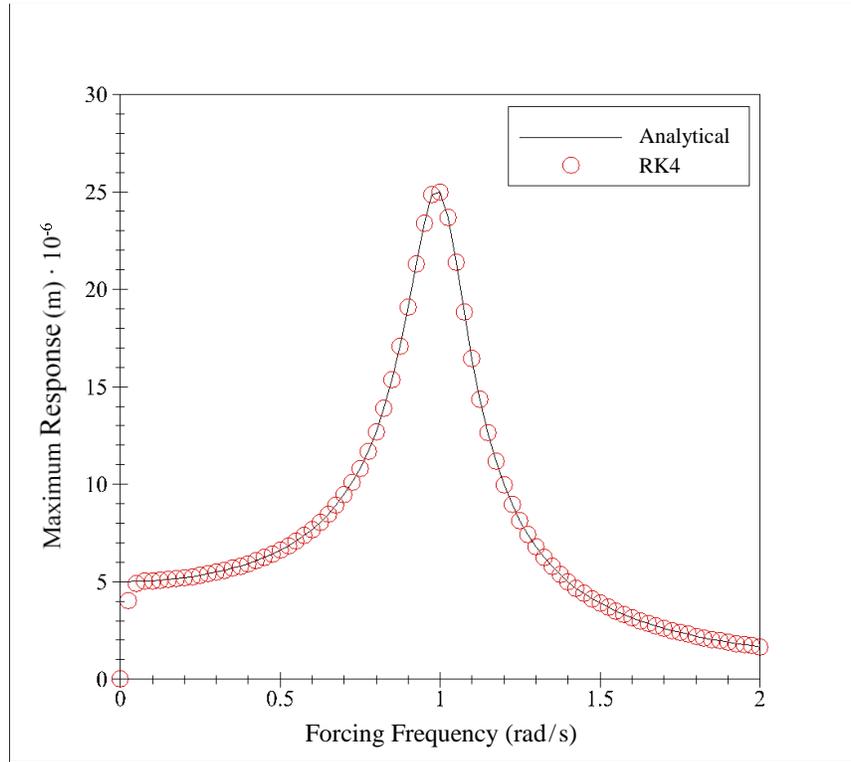
$$\bar{q}^{t+\Delta t} = \bar{q}^t + \Delta t/6 \cdot [\bar{k}_1 + 2\bar{k}_2 + 2\bar{k}_3 + \bar{k}_4] \quad (6.24)$$

The SDOF response model is integrated into a computer model that is now validated by computation of a response spectrum. The mass, stiffness, and damping ratio of the system are  $M = 200,000$  kg,  $K = 200,000$  N/m, and  $\zeta = 0.1$  respectively. The applied forcing is a sine wave, which is selected because an analytical solution is available for a SDOF structure's response to sinusoidal loading, having amplitude and frequency of  $A_F = 1.0$  N and  $0 \leq \omega_F \leq 2$  rad/s

respectively. The analytical solution to the maximum structure response as function of the structure's fundamental frequency is given in Eq. (6.25).

$$A_s = \frac{A_F}{M} \cdot \left( \sqrt{(\omega_n^2 - \omega_F^2)^2 + (2\zeta\omega_n\omega_F)^2} \right)^{-1} \quad (6.25)$$

**Figure 6.3** compares the response spectrum defined by Eq. (6.25) with the response spectrum computed by the computer model.



**Figure 6.3:** Comparison of the response spectra defined analytically by Eq. (6.25) and that defined numerically by the RK4 numerical solution.

The response spectra computed via RK4 solution of the computer model replicates that defined by the analytical solution defined by Eq. (6.25). The maximum response occurs at the damped natural frequency  $\omega_d = 0.995$  rad/s defined by Eq. (6.2) which for practical purposes is the same as the structure's undamped natural frequency.

$$\omega_d = \omega_n \cdot \sqrt{1 - \zeta^2} \quad (6.26)$$

Having now confirmed the accuracy of the SDOF response model, the model can be applied to assess dynamic amplification of structure response to vortex loading. The dimensionless DLF shall be used to perform this study, hence the following section explains the concept, computation, and application of the DLF.

#### 6.4 The Dynamic Load Factor

Consider the application of arbitrary forcing  $F(t)$  to a structure having stiffness  $K$ , where the amplitude of the forcing is  $F_o$ . If each discrete value of  $F(t)$  is applied as static structure loading, the maximum static displacement of the structure is defined by Eq. (6.27).

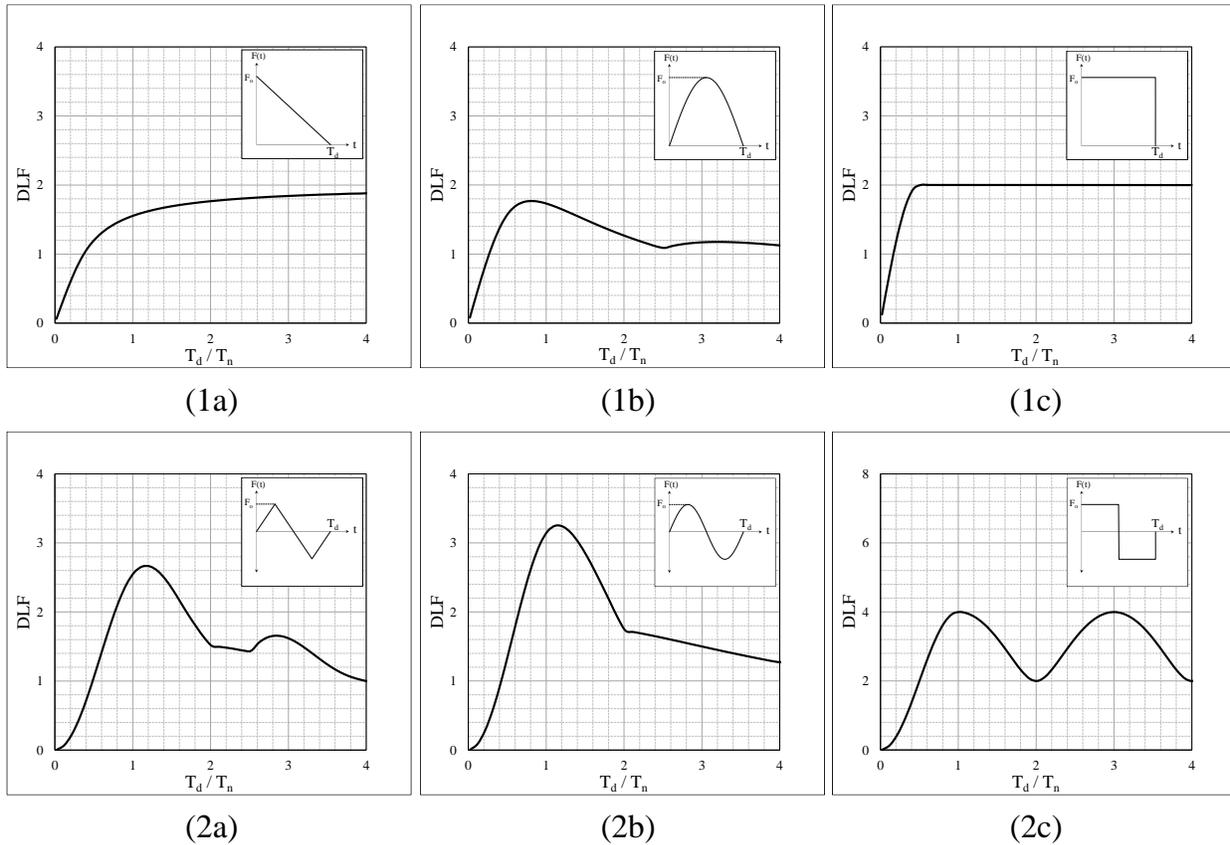
$$x_{o,st} = F_o/K \quad (6.27)$$

Now when the complete forcing time history  $F(t)$  is applied to the structure, the time history of the structure's response is  $x(t)$ . The dynamic amplification of the applied forcing at each time instant is defined by normalizing the instantaneous displacement using the maximum static displacement  $x_{o,st}$  as shown in Eq. (6.28).

$$DLF(t) = x(t)/x_{o,st} \quad (6.28)$$

It is common practice to present the maximum dynamic response amplification produced by a specific forcing profile using dimensionless DLF curves (see structural dynamics texts such as Paz and Leigh (2004) and Chopra (2005)). An un-damped ( $\zeta = 0$ ), SDOF system having fundamental period  $T_n$  is dynamically loaded over the period  $T_d$ . The response of the SDOF structure is computed, and the maximum value of  $DLF(t)$  is reported as the dynamic amplification of the applied forcing when the forcing is applied to a structure having the specified fundamental period  $T_n$ . This procedure is repeated as the fundamental period of the loaded structure is incrementally increased. Subsequently, the maximum values of  $DLF(t)$  for each value of  $T_n$  are reported in a single curve, which is rendered dimensionless by plotting the

DLF associated with each fundamental structure period as function of the ratio  $T_d/T_n$ . DLF curves for some common forcing profiles are shown in Figure 6.4.



**Figure 6.4:** DLF curves for some common forcing profiles ( $\zeta = 0$ ).

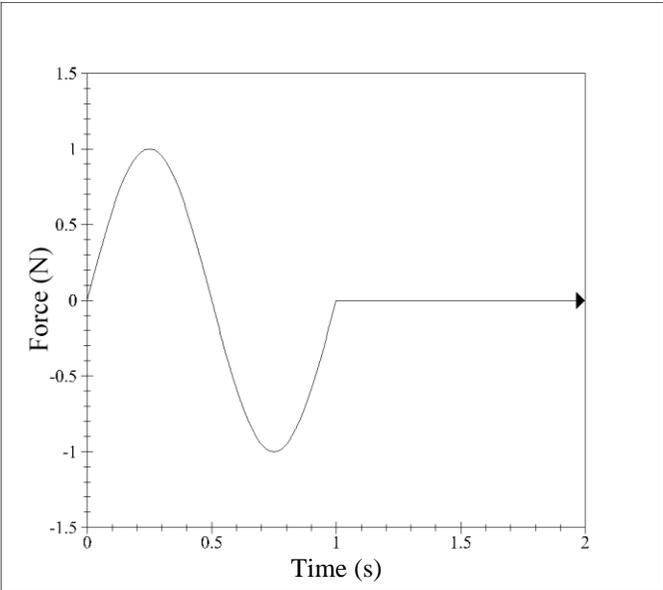
Defining DLF curves for forcing profiles is analogous to defining aerodynamic force coefficients (Such as  $C_d$  and  $C_l$ ) as functions of Reynolds number for various body shapes. The Reynolds number combines parameters characterizing flow over the body, and the similarity principal explains that any combination of system parameters producing the same characterizing Reynolds number will produce the same type of flow around the body. This allows force coefficients to be defined as a function of the characterized system rather than for each of the individual system variables. Similarly, when a structure is dynamically loaded by a specified forcing profile, there are practically infinite possible combinations of forcing amplitude, forcing application period, and structure fundamental period. However, once the curve  $DLF(T_d/T_n)$  is

defined for the specified forcing profile, the dynamic amplification of the maximum static structure response defined previously in Eq. (6.28) can be easily read.

6.4.1 Construction of DLF Curves

This subsection demonstrates the procedure for constructing DLF curves for a specified forcing profile. Special focus is given on explaining and illustrating the benefit of using the dimensionless DLF to evaluate dynamic amplification of structure loading. Subsequently, the DLF concept is used to define the vortex load-application period  $T_v$  and to assess the dynamic amplification of vortex-induced loading.

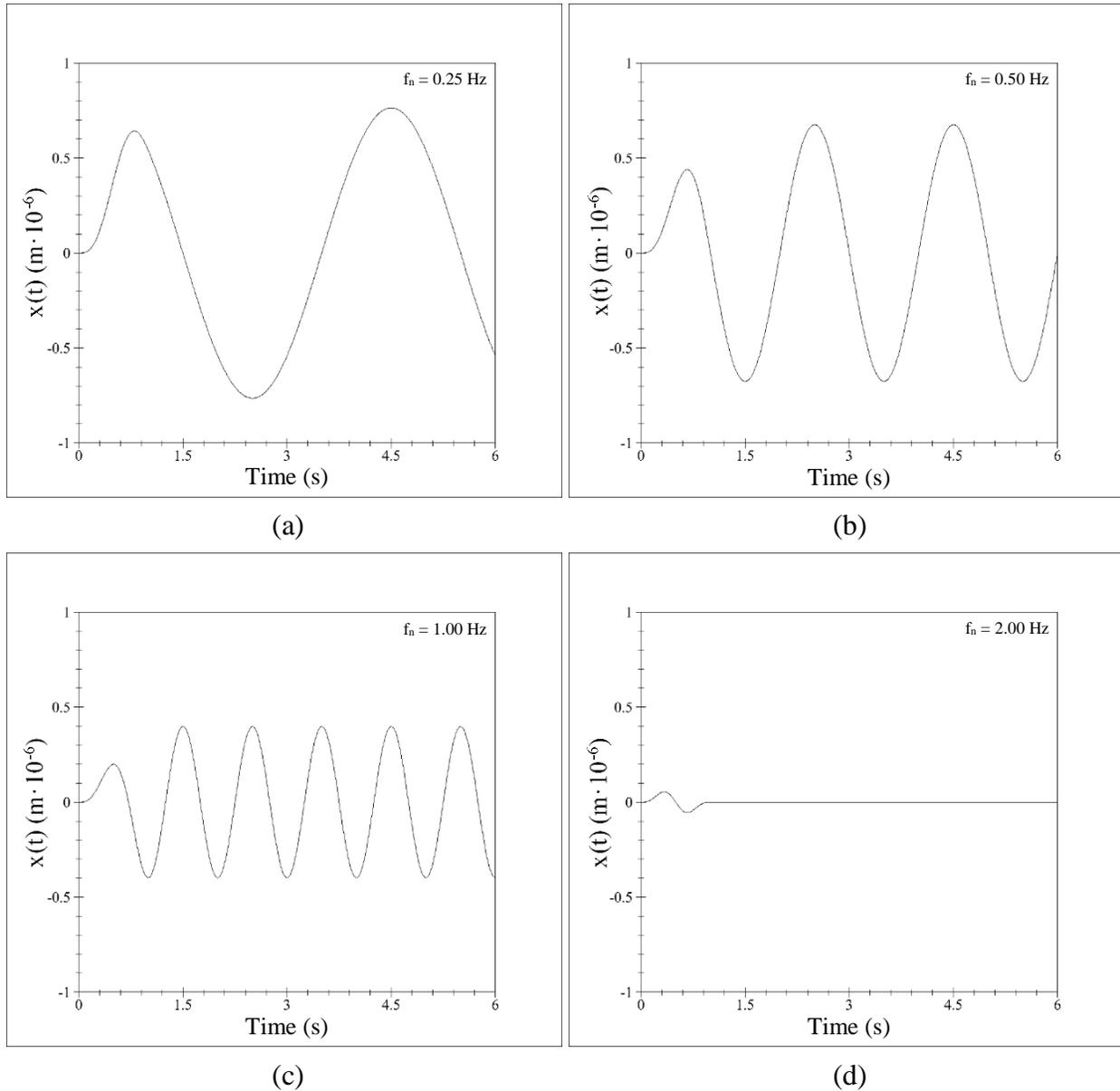
The considered forcing profile is the single-period sinusoid illustrated in Figure 6.5. The forcing has amplitude  $F_o = 1$  N and load application period  $T_d = 1$  s. The forcing is applied to an undamped  $\zeta = 0$  structure having fixed mass  $M = 200,000$  kg. The responses of four separate structures, each of which having a different fundamental frequency  $f_n = T_n^{-1}$ , the stiffness and frequency of each structure are summarized in Table 6.1. For each structure, the fundamental frequency is specified and the corresponding structure stiffness is computed as  $K = (f_n/2\cdot\pi)^2\cdot M$ .



**Figure 6.5:** Sinusoid impulse applied as forcing.

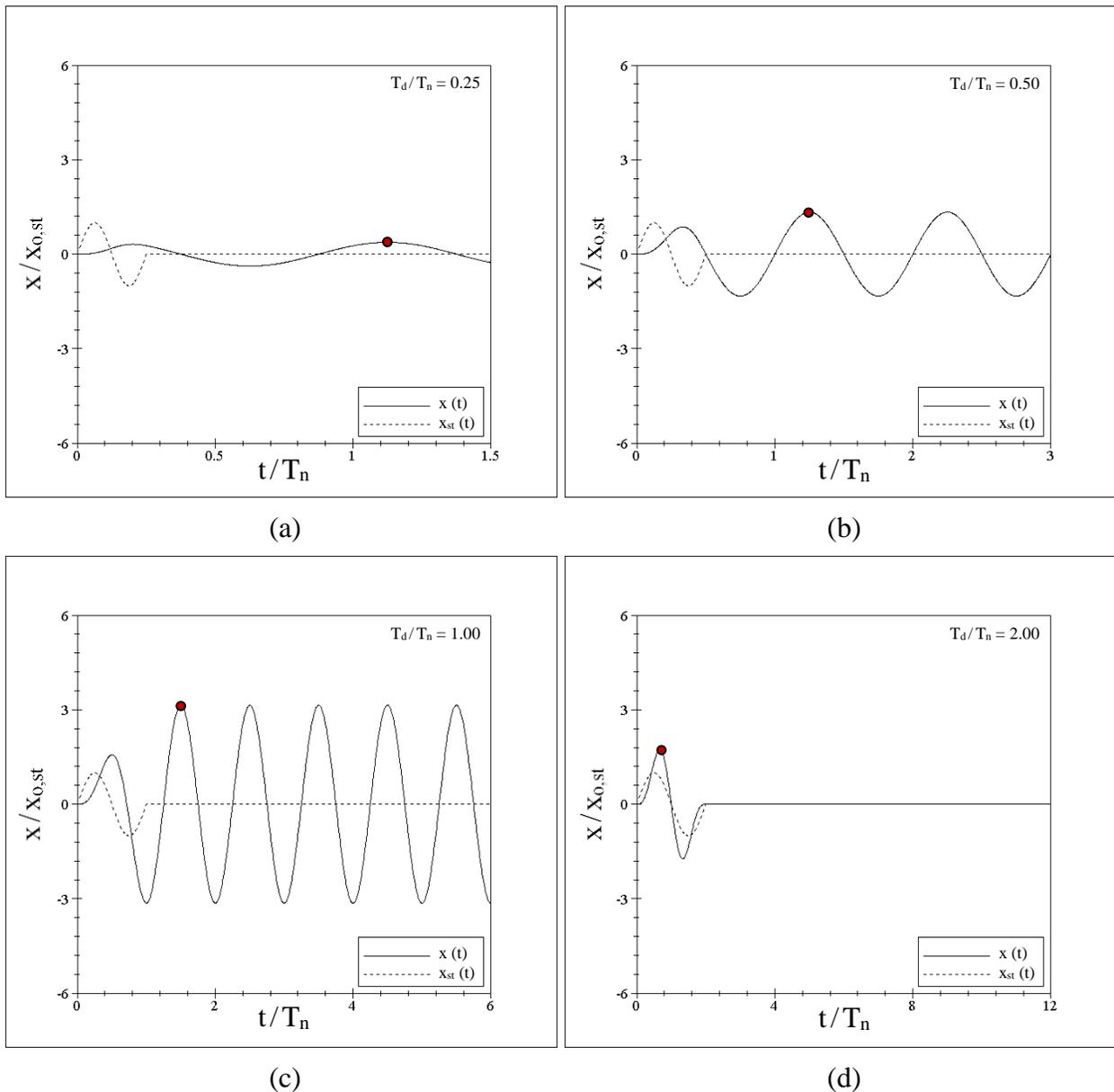
**Table 6.1:** Parameters for the four considered SDOF structures.

Structure	$f_n$ (Hz)	K (N/m)	$T_d/T_n$
1	0.25	493,480	0.25
2	0.50	1,973,900	0.50
3	1.00	7,895,700	1.00
4	2.00	31,583,000	2.00



**Figure 6.6:** Responses of the four SDOF structures defined in *Table 6.1* to the sinusoidal forcing shown in *Figure 6.5*.

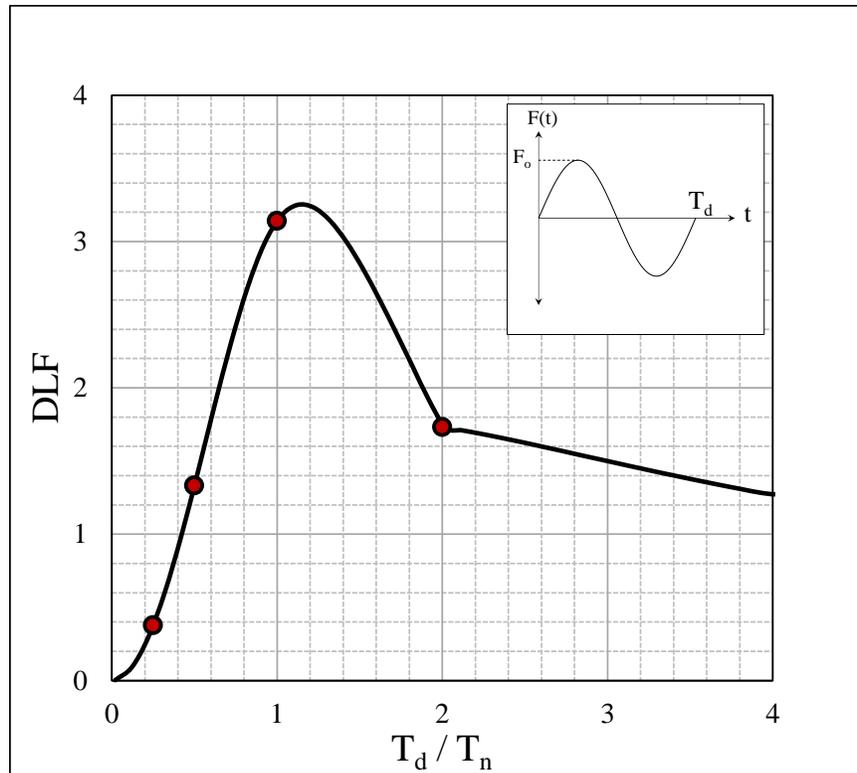
The sinusoid of forcing is applied to each of the four structures defined in Table 6.1 producing the responses illustrated in Figure 6.6. The amplitude of the structure's response generally decreases progressing from Figure 6.6a to 6.6d due to the increasing stiffness of the structure. However, analyzing the structure responses in this manner provides no meaningful insight regarding structure response amplification due to the dynamic nature of the sinusoidal forcing.



**Figure 6.7:** Dimensionless response of the four SDOF structures to the sinusoid forcing.

Dynamic amplification of the structure's response is illustrated in a more meaningful manner by plotting  $DLF(t)$  instead of  $x(t)$ . This approach effectively eliminates the influence of the different structures' stiffness and conveys only the dynamic loading effect. The maximum static deflection  $x_{o,st}$  previously defined in Eq. (6.27) is defined using each structures' stiffness and used to normalize the time history of the structure response (thereby defining  $DLF(t)$ ). Time is also converted to dimensionless format, following Chopra's (1995) convention, by normalizing time with respect to the loaded structure's  $T_n$ . The four resulting response time histories are presented in Figure 6.7. Solid lines plot the DLF time history, and the dashed line illustrates the SDOF time history if the discrete values of the sinusoid of forcing were applied as static loads.

Figures 6.7a – 6.7d, provide far better illustration of the dynamic response amplification than Figures 6.6a – 6.6d. All four structures experience greater displacement than they would if a static force equivalent to the sinusoid's amplitude were applied. Generally speaking, the DLF increases as  $T_d/T_n$  approaches unity and decreases moving away from unity. The maximum DLF value for each structure is indicated by a red circle. Now a range of fundamental structure periods is selected, and structures having discrete  $T_n$  spanning that range are defined. The sinusoid forcing is applied to the SDOF system for each value of  $T_n$ , and the maximum DLF value is extracted. The DLF curve for a single sinusoid forcing profile is defined by plotting the DLF values for each value of  $T_n$  as a function of the ratio of the application period of the sinusoid to the fundamental period of the structure as shown in Figure 6.8. DLF values marked by red circles correspond to the values identified in Figure 6.7.

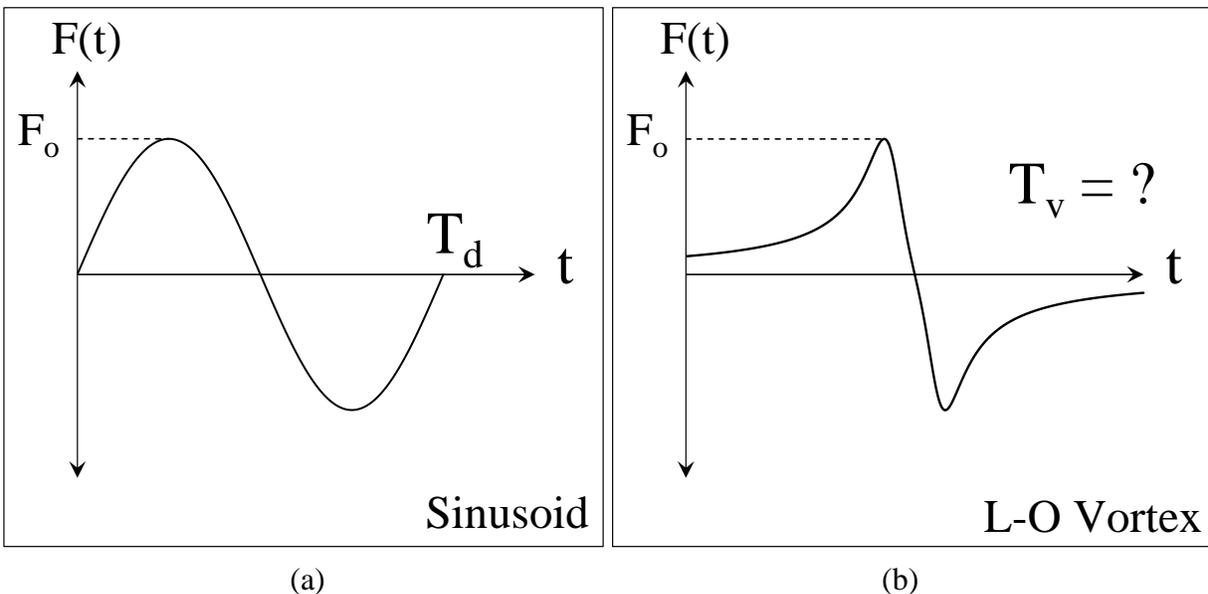


**Figure 6.8:** DLF curve for a single period of sinusoidal loading.

Figure 6.8 illustrates that when the period of the applied sinusoid and the fundamental period of the structure are similar, the sinusoidal profile of the applied loading results in a more than three-fold increase in the structure's deflection than if the load of equivalent magnitude  $F_0$  were applied statically. It is important to note that the dynamic nature of the forcing profile may substantially increase structure deflection even if  $T_d$  is less than or greater than  $T_n$ . This example illustrates the construction methodology and benefit of DLF curves. Specifically, if the profile of the impulsive forcing and the ratio of the load application period to the fundamental structure period are known, the associated dynamic amplification of the structure response can be deduced regardless of the individual values of  $T_d$  or  $T_n$ .

## 6.5 Definition of the Vortex Loading Period

The previous section discusses the DLF concept and illustrates the procedure for computing the DLF curve for a specified forcing profile. Generally stated, the DLF is a scalar value that increases or decreases the maximum structure response that the dynamically-applied forcing would produce if the maximum forcing amplitude were applied statically. Use of DLF curves requires knowledge of only the profile of the applied forcing and the ratio  $T_d/T_n$  to define the dynamic amplification of  $x_{o,st}$ . The fundamental structure period  $T_n$  can be computed during design or measured in existing structure; however, defining  $T_d$  for vortex forcing presents a challenge. Unlike the periodic sinusoid forcing profile shown in Figure 6.9a, the vortex's cross-stream forcing profile shown in Figure 6.9b does not become zero at a discrete time. Therefore, there is no intuitive definition of the vortex load application period  $T_v$  ( $T_d$  and  $T_v$  are analogous, but  $T_d$  defines generic forcing while  $T_v$  is specific to vortex forcing).



**Figure 6.9:** (a) Sinusoid forcing profile having application period  $T_d$  and (b) vortex forcing profile having unknown application period  $T_v$ .

Generalization of the dynamic amplification of tornado wind loading necessitates definition of  $T_v$ . Previous studies of structure response to dynamic tornado wind loads study only

the response of a specific structure to a specific wind field rather than arriving at general conclusions. Parameters of vortices that load structures vary greatly, hence it is necessary to develop a methodology to compute  $T_v$  as function of the vortex's parameters. The subsequent sections utilize the DLF concept, applying forcing from two-dimensional computer simulation as forcing, to define  $T_v$  as function of the impinging vortex's parameters.

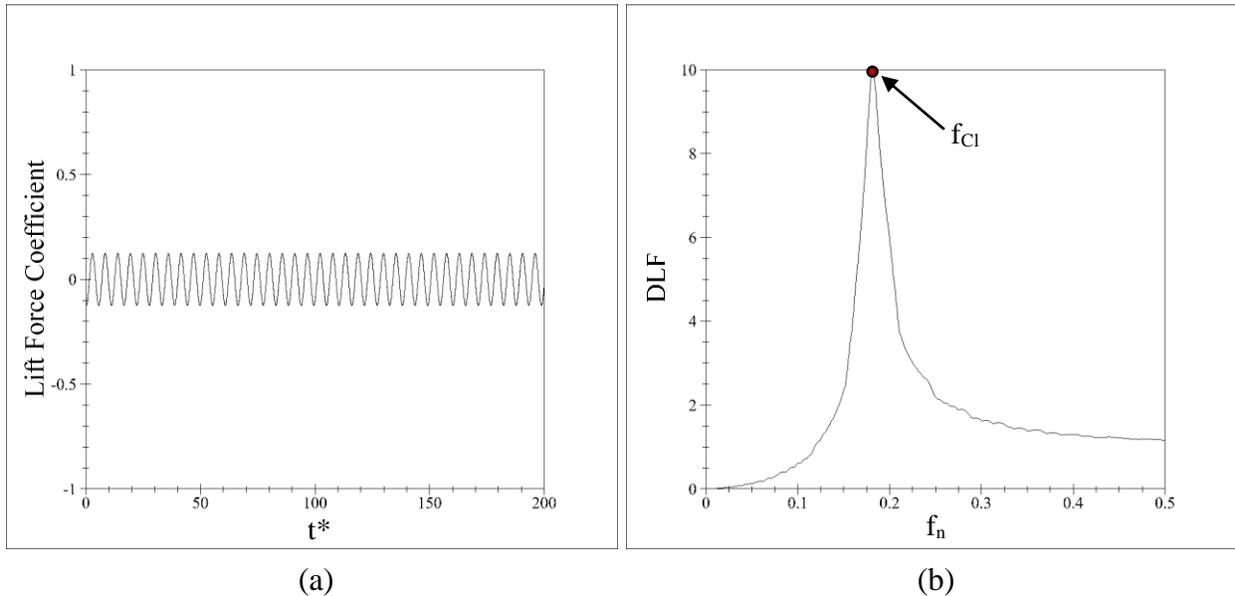
### 6.5.1 Methodology

Lift force coefficients from computer-simulated vortex loading of a rigid circular cylinder are applied as forcing to the SDOF response model. The DLF concept is applied to compute the fundamental structure period for which the structure's response experiences greatest dynamic amplification, and this is defined as the vortex load-application period  $T_v$ . The computer simulations are performed at  $Re \leq 300$ , hence the lift force coefficient is composed of mean and harmonic components. Giosan and Eng (2013) explain that vortex shedding does not occur when wind speeds exceed 15 m/s due to excessive turbulence in the air. This means the harmonic component of  $C_l$  is not a realistic component of tornado wind loading; therefore, the present study defines the vortex forcing profile as the mean component of  $C_l$ .

When the SDOF response model is forced with the  $C_l$  time history, it is expected that the mean component should produce dynamic response amplification at the vortex loading frequency  $f_v = T_v^{-1}$ . However, the presence of the harmonic component of  $C_l$  adds a complication, specifically that it should produce dynamically-amplified structure response at the vortex shedding frequency  $f_{Cl}$ . As the vortex approaches the cylinder, its tangential velocity augments the velocity of the stream over the cylinder and consequently increases the vortex shedding frequency. The impinging vortices studied herein are relatively large and slow-moving, hence it is speculated that the vortex loading frequency is notably less than the vortex shedding

frequency ( $f_v < f_{Cl}$ ). Therefore, after noting the peak in the DLF curve at  $f_{Cl}$  produced by vortex shedding, the dynamic amplification produced by the mean component of  $C_l$  should appear as a distinct, lower-frequency peak at  $f_v$ .

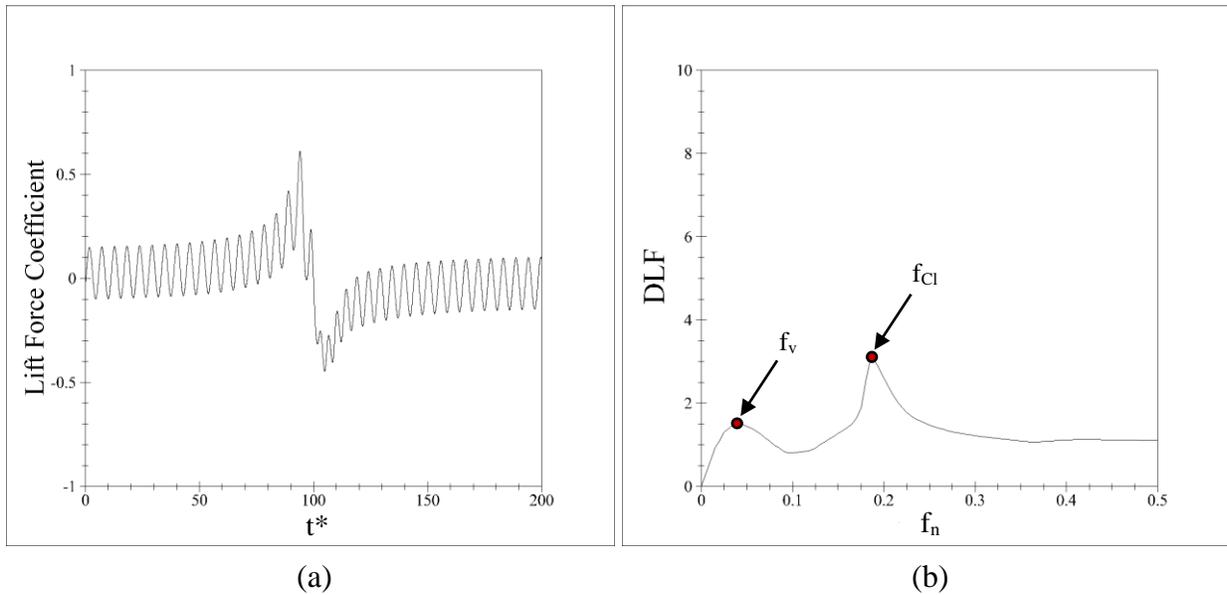
Flow over the cylinder immersed in the free stream is simulated producing the force lift force coefficient time history illustrated in Figure 6.10a. This lift force coefficient time history is then applied as forcing to the SDOF response model; breaking from common convention, the damping ratio of the system is set to  $\zeta = 5\%$  to avoid resonant response when the structure fundamental frequency  $f_n \approx f_{Cl}$ . Figure 6.10b plots the DLF curve for the lift force coefficient time history; the distinct peak at the vortex shedding frequency  $f_n \approx f_{Cl}$  is marked by the callout. The maximum DLF value of 9.99 is practically equivalent to the theoretical maximum value  $DLF = (2 \cdot \zeta)^{-1} = 10.0$  for a system damped at  $\zeta = 5\%$ .



**Figure 6.10:** Free stream lift force coefficient (a) time history and (b) DLF curve.

Vortex impact with the cylinder is now simulated, and the resulting lift force coefficient time history is illustrated in Figure 6.11a. The lift force coefficient time history as forcing to the SDOF response model, allowing construction of the DLF curve illustrated in Figure 6.11b. The

harmonic component of  $C_l$  still produces the distinct DLF curve peak at  $f_{Cl}$ . However, there is an additional distinct, lower frequency peak in the DLF curve indicated by the callout  $f_v$ . This is the vortex loading frequency and is the fundamental structure frequency which experiences the maximum dynamic response amplification due to the vortex loading.



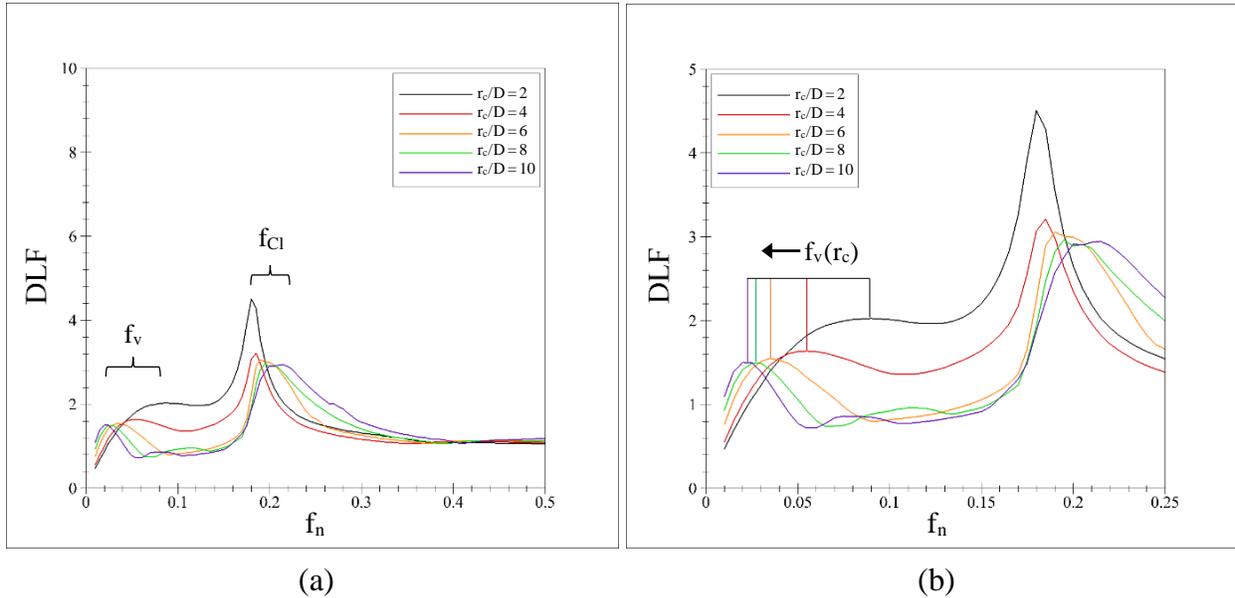
**Figure 6.11:** Vortex loading lift force coefficient (a) time history and (b) DLF curve.

The vortex loading frequency  $f_v$  is now defined as the fundamental structure frequency that exhibits the greatest dynamic response amplification to the mean component of  $C_l$ . The subsequent subsections use the L-O vortex profile and define  $T_v = f_v^{-1}$  as function of the vortex's critical radius  $r_c$  and translational velocity  $U_\infty$ . Finally, cylinder loading by RCVM and S-K vortices is simulated to establish the influence of the vortex's tangential velocity profile on  $T_v$ .

### 6.5.2 Influence of Vortex Size on Vortex Loading Period

The first parameter studied in defining the vortex loading period is the size, specifically the critical radius  $r_c$ , of the impinging vortex. The maximum vortex tangential velocity and vortex translational velocity are fixed and equivalent ( $V_{\theta, \max} = U_\infty = 1.0$ ), and the size of the impinging vortex is increased from  $r_c = 2 \cdot D$  to  $10 \cdot D$ . Lift force coefficient time histories from

each simulation are applied as forcing to the SDOF response model, and the resulting DLF curves are plotted in Figure 6.12a. Figure 6.12b is a zoomed view of the peaks in the DLF curves.

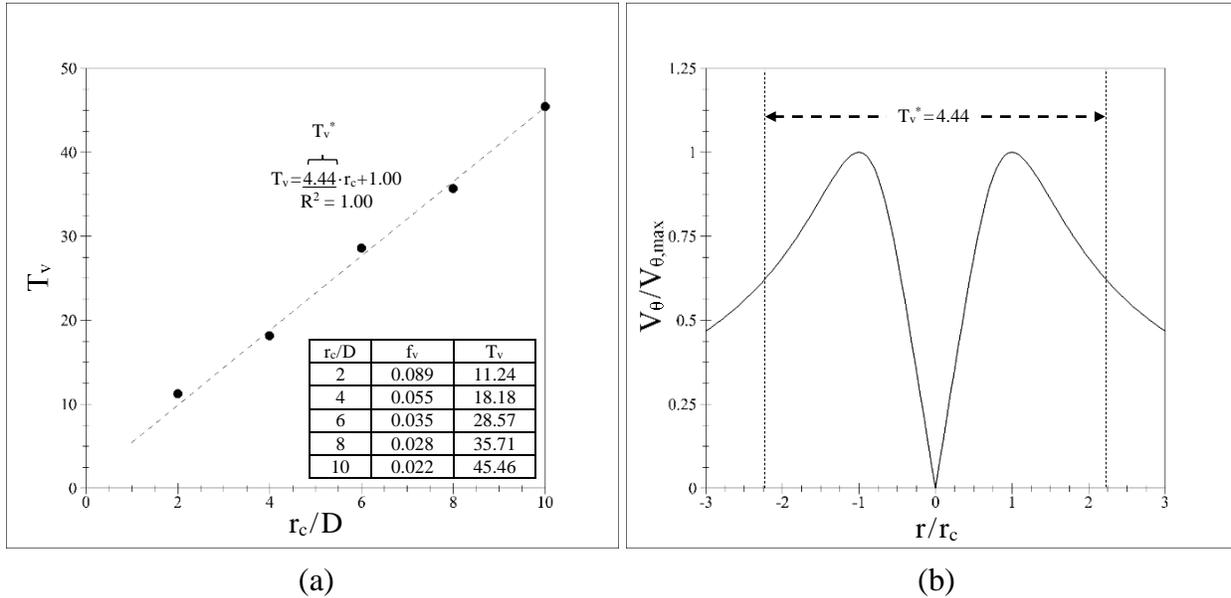


**Figure 6.12:** (a) Lift force coefficient DLF curves and (b) zoomed view of  $f_v$  illustrating the progressive decrease in  $f_v$  with increasing  $r_c$ .

Two general observations are made from the DLF curves in Figures 6.12a and 6.12b. First, the DLF curve peaks associated with  $f_{Cl}$  progressively move right-ward and broaden as  $r_c$  increases. This is because the larger impinging vortex progressively augments the velocity of the stream over the cylinder pre-impact, hence  $f_{Cl}$  progressively increases. Second,  $T_v = f_v^{-1}$  progressively increases with  $r_c$  as shown in Figure 6.12b. Intuitively, this is because a larger vortex interacts with the cylinder for longer time, hence the load application period is longer.

Vortex loading frequency values (marked by vertical lines in Figure 6.12b are extracted and inverted to give the vortex loading period for each considered  $r_c$  value. The vortex load application period  $T_v$  is then plotted as function of  $r_c$  in Figure 6.13a, revealing that  $T_v(r_c)$  is a linear trend. The slope of  $T_v(r_c)$ , is now defined as the dimensionless vortex loading period  $T_v^*$ . The dimensionless vortex loading period  $T_v^*$  corresponds to the dimensionless region of the

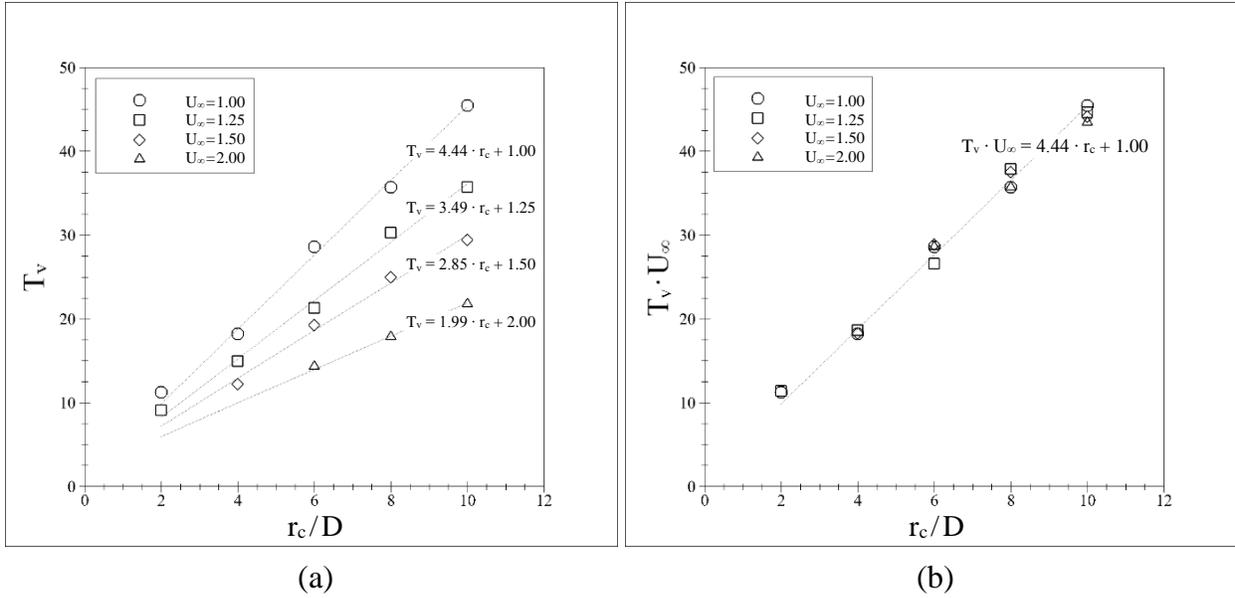
vortex's tangential velocity profile that is illustrated in Figure 6.13b for the L-O vortex. It is postulated that  $T_v$  is the time required for the portion of the vortex's tangential velocity profile defined by  $T_v^*$  to cross the loaded structure.



**Figure 6.13:** (a) Vortex loading period as function of vortex critical radius for  $r_c = 2 \cdot D$  to  $10 \cdot D$  and (b) dimensionless region of the L-O vortex's tangential velocity profile corresponding to  $T_v$ .

### 6.5.3 Influence of Vortex Translational Velocity on Vortex Loading Period

Now the influence of the impinging vortex's translational velocity on the vortex loading period is evaluated. The vortex's maximum tangential velocity remains fixed at  $V_{\theta, \max} = 1.0$ , and the vortex's translational velocity is incremented from  $U_\infty = 1.0$  to  $2.0$ . For each considered translational velocity, the vortex size is incremented from  $r_c = 2 \cdot D$  to  $10 \cdot D$ . Lift force coefficient time histories from each simulation are applied as forcing to the SDOF response model, and  $T_v$  is extracted for each. The vortex loading period decreases when  $r_c$  decreases or  $U_\infty$  increases, hence  $f_v$  approaches  $f_{c1}$  and there is no distinct peak in the DLF curve for some combinations of small  $r_c$  and large  $U_\infty$  (which is why the data set for  $U_\infty = 2.0$  only has three data points). Figure 6.14a summarizes  $T_v(r_c)$  for each considered translational velocity.



**Figure 6.14:** (a) Vortex loading period as function of vortex translational velocity and (b) consolidation of vortex loading period curves using Eq. (6.29).

The trend  $T_v(r_c)$  is linear for all considered  $U_\infty$ , and, as one would expect, increasing the vortex's translational velocity decreases the associated value of  $T_v$ . The postulation raised in the previous subsection, specifically that  $T_v$  is the time required for the portion of the dimensionless vortex profile defined by  $T_v^*$  to cross the loaded structure is defined by Eq. (6.29).

$$T_v = T_v^* \cdot r_c \cdot U_\infty^{-1} \quad (6.29)$$

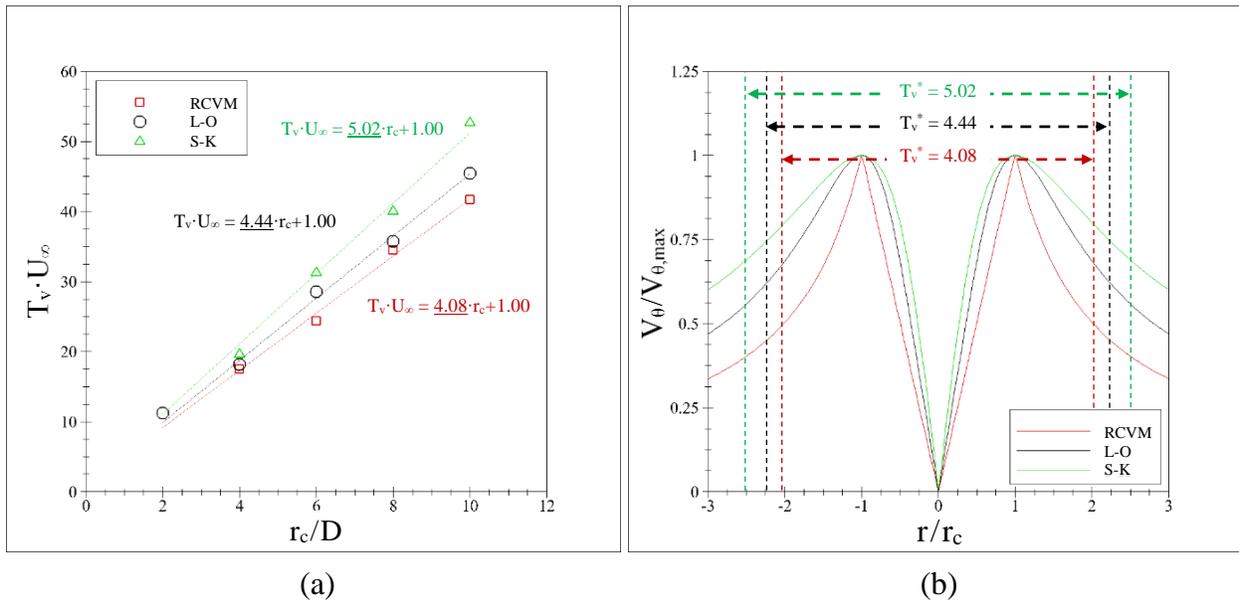
The data plotted in Figure 6.14a are now re-plotted in Figure 6.14b using the definition of  $T_v$  defined by Eq. (6.18). The  $T_v(r_c)$  curves compress to the trend line previously defined in Figure 6.13a (Defined using  $U_\infty = 1$ ). This confirms the postulation that  $T_v$  is the time required for the dimensionless region of the vortex profile defined by  $T_v^*$  to cross the loaded structure. Eq. (6.2) establishes the generalized relationship between  $T_v$  and the two vortex parameters  $r_c$  and  $U_\infty$ .

#### 6.5.4 Influence of Vortex Profile on Vortex Loading Period

Chapter 3 reports an extensive review of analytical and measured vortex tangential velocity profiles. Measured tornado vortex profiles range from smooth (S-K vortex) to sharp and discontinuous (RCVM vortex), meaning a tornado vortex's forcing profile may vary greatly.

Dynamic amplification of a structure's response depends upon the applied forcing profile, hence it is postulated that  $T_v$  is a function of the impinging vortex's tangential velocity profile in addition to its size and translational velocity.

The L-O vortex profile has been used up to this point because it is representative of the “typical vortex”. Now the S-K and RCVM vortex profiles are used to evaluate the respective influences of flatter and sharper vortex profiles on  $T_v$ . Analysis in Chapter 3 concludes that that the Sullivan vortex profile is a better representation of most of the sharper measured vortex profiles than the RCVM profile is. However, the RCVM profile is used here for two reasons: it is a more extreme sharper profile, and some measured vortex profiles do fit it. Therefore, use of the RCVM profile in place of the Sullivan profile affords a better representation of the sharpest possible vortex profile. The methodology outlined in Section 6.5.2 is now repeated for the S-K and RCVM profiles, and the resulting trends  $T_v \cdot U_\infty(r_c)$  are summarized in Figure 6.15a along with the trend for the L-O vortex previously-defined in Figure 6.13a.



**Figure 6.15:** (a) Definition of  $T_v(r_c)$  for the three vortex profiles and (b) dimensionless regions of the three vortex tangential velocity profiles corresponding to  $T_v^*$ .

Figure 6.15a shows that  $T_v$  progressively lengthens (RCVM to L-O to S-K) as the vortex's tangential velocity profile flattens. The definition to  $T_v^*$  is retained and illustrated for all three vortex profiles in Figure 6.15b. The vortex load-application period has now been defined as function of the three primary variables that characterize intense vortices, specifically, size ( $r_c$ ), translational velocity ( $U_\infty$ ), and vortex tangential velocity profile (RCVM, L-O, or S-K). The definition of  $T_v$  in Eq. (6.29) defines the fundamental structure frequency that will experience maximum dynamic response amplification due to cross-stream loading from a vortex.

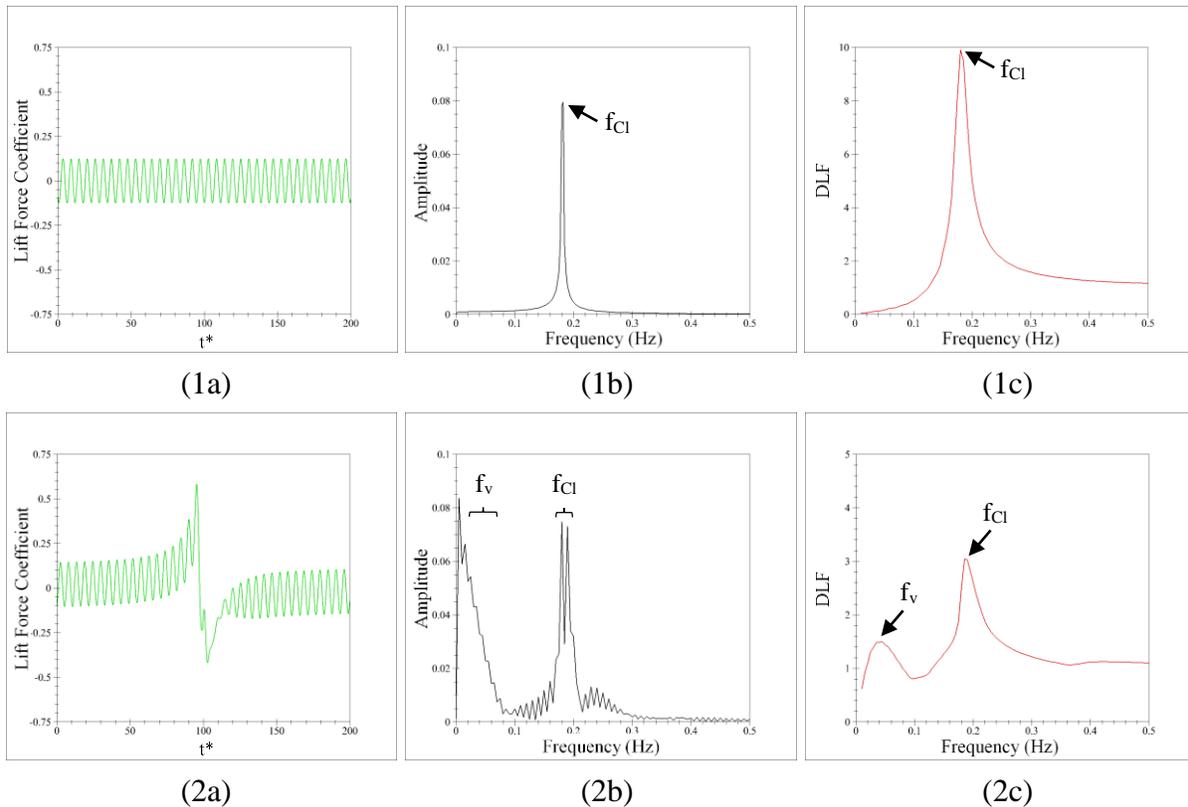
#### 6.5.5 Why Use the DLF Instead of the Fourier transform to Identify $T_v$ ?

The present study utilizes DLF analysis, specifically comparing the response of structures having a spectrum of fundamental periods  $T_n$ , to identify the vortex loading period  $T_v$ . The DLF analysis may seem to be a roundabout approach, as one might think that the discrete Fourier transform (DFT) should directly decompose the forcing into constituent components, directly yielding  $T_v$ . However, the DFT is unable to isolate  $T_v$  from a given vortex forcing time history while the DLF is able to. One postulation for the superior performance of the DLF analysis is that practically any  $T_n$  resolution may be utilized, while  $f_n$  resolution is constrained to integer multiples of  $2/T_{\text{signal}}$ , where  $T_{\text{signal}}$  is the duration of the forcing. If the exact vortex loading period is not considered in the DFT, specifically  $T_v = 1/f_n$ , the component of forcing at the vortex frequency  $f_v = 1/T_v$  will be leaked to the neighboring adjacent frequencies, thereby reducing its resolution. Stull (1988) reports numerous other factors that influence accuracy and resolution of the DFT. Briefly summarized, DLF analysis is used to identify  $T_v$  because the DFT is unable to do so. Numerous parameters of the forcing can influence accuracy of the DFT, but identification of the specific parameters in the vortex forcing is beyond the scope of this study.

The subsequent examples illustrate the inability of the DFT to resolve  $T_v$  utilizing the forcing produced by a free stream and by vortex impact from Section 6.5.1.

Figure 6.16.1a illustrates the lift force coefficient time history from simulated free stream flow over a cylinder. The forcing is harmonic due to shedding of vortices from the cylinder at frequency  $f_{Cl}$ . The vortex shedding frequency  $f_{Cl}$  is accurately captured by both Fourier Transform and DLF analysis as respectively shown in Figures 6.16.1b and 6.16.1c.

Figure 6.16.2a illustrates the lift force coefficient time history from vortex impact with the cylinder. The vortex shedding frequency changes as the vortex approaches the cylinder as evidenced by the numerous constituting frequencies detected on either side of the original vortex shedding frequency identified in Figure 6.16.1b. The vortex loading frequency  $f_v$  that is



**Figure 6.16:** (a) Lift force coefficient time history, (b) corresponding Fourier Transform, and (c) corresponding dynamic load factor curve for (1) simulated free stream flow over a cylinder and (2) simulated vortex impact with a cylinder.

associated with the mean component of the lift force coefficient is an indistinguishable lower frequency  $f_v < f_{Cl}$  as indicated in Figure 6.16.2b. However, when the vortex loading is applied to a response model in DLF analysis,  $f_v$  emerges as the distinct peak indicated in Figure 6.16.2c. It is therefore concluded that DLF analysis rather than Fourier analysis is necessary to identify the vortex loading frequency  $f_v$ .

## **6.6 Use of Empirical Equations to Define Vortex Loading**

### 6.6.1 Overview

Up to this point, lift force coefficient time histories from computer-simulated loading of a circular cylinder by an impinging vortex have been applied as forcing to define  $T_v$ . These simulations are restricted to  $Re \leq 300$  because the current computer model is two-dimensional. Consequently, vortex shedding from the cylinder produces a harmonic loading component that is not realistic of loading by tornado vortices. Additionally, the computer simulations are computationally expensive, requiring approximately 48 hours of CPU time to execute. Finally, the utilized grid and the model's numerical stability limit the range of usable vortex parameters.

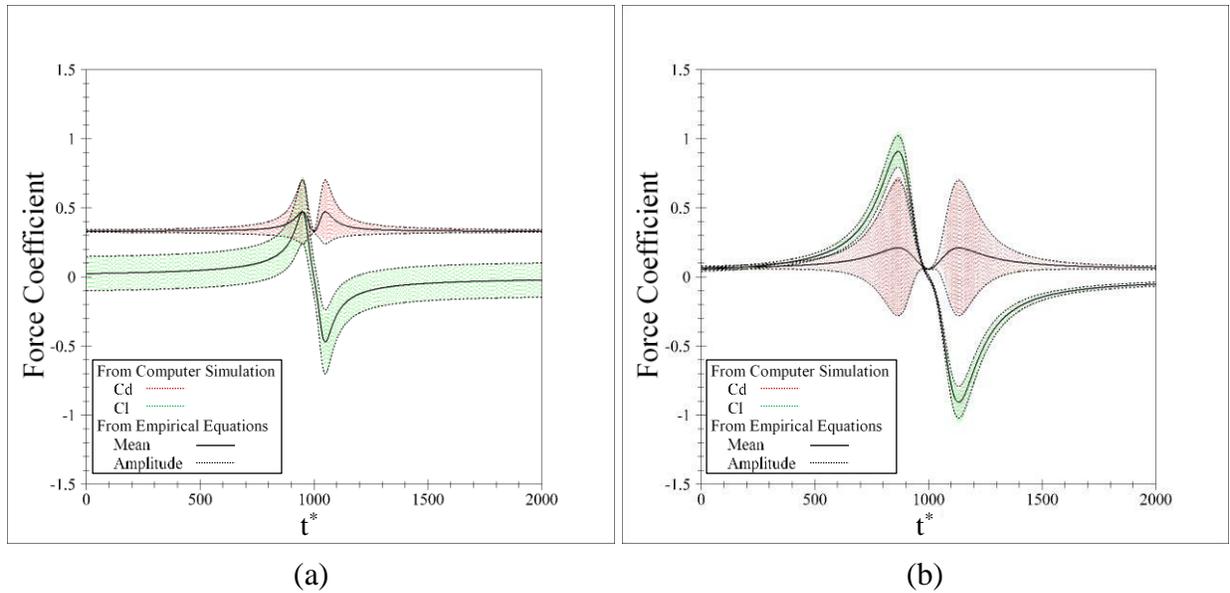
Generalization of the methodology to assess the possible dynamic response amplification of structures to tornado wind loads requires definition of DLF curves for the identified vortex velocity profiles. Tornado wind loads do not produce vortex shedding, hence only the mean component of the lift force coefficient need be applied as forcing. A set of empirical equations Eqs. (5.1-6) are developed in Section 5.4.3 to define the cylinder loading as a function of the velocity incident on the cylinder. The present analysis applies the vortex velocity boundary condition's governing equations Eqs. (4.8a-4.10b) to define the time history of the stream velocity incident on the cylinder, and then uses the empirical equations to define the corresponding force coefficient time histories. The equation-defined forcing is validated by

comparison with force coefficient time histories from computer simulation. Subsequently, it is applied to assess the influence of the vortex velocity ratio on  $T_v$ , a study which could not be performed previously due to numerical instability of the computer model.

### 6.6.2 Validation of Empirical Equation-Defined Forcing

The primary assumptions that are made when using the empirical equations to define the cylinder loading are that the same velocity effectively acts over the entire cylinder surface and that the velocity is the velocity that would act at the location of the cylinder center if the cylinder were not present to disrupt the flow around it. These assumptions are reasonably met when the impinging vortex is much larger than the cylinder. Therefore, the first stage of model validation is to assess the accuracy with which they replicate the cylinder loading from computer simulation. This serves as additional validation to that previously reported in Figure 5.20.

Following the approach used in Section 5.4.3, the validation study is performed with impinging vortices having critical radius  $r_c = 50 \cdot D$ . The maximum resultant velocity  $V_R$  in the domain is restricted to 2.0, because this is the limit to which the empirical equations are defined. The first comparison is for the vortex velocity ratio  $V_{\theta, \max}/U_\infty = 1.0$ , and the second comparison is performed for  $V_{\theta, \max}/U_\infty = 4.3$ . Figures 6.17a and 6.17b respectively illustrate the comparison of force coefficient time histories defined by the empirical equations with those from computer simulation for vortex velocity ratios of 1.0 and 4.3. Colored lines illustrate force coefficient time histories from computer simulation, and black solid and dashed lines respectively illustrate mean and harmonic components of the force coefficient time histories defined using the empirical equations. The equation-defined forcing replicates the computer simulation forcing with excellent accuracy.

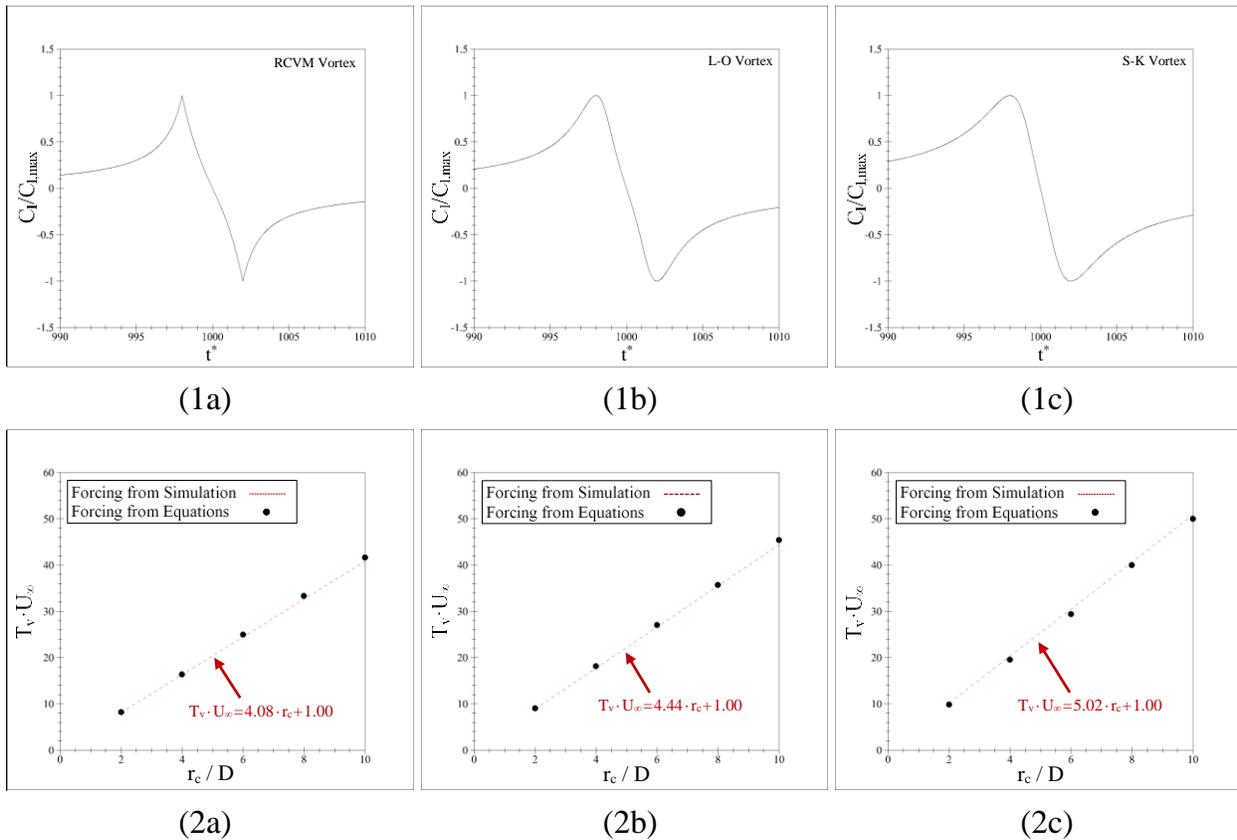


**Figure 6.17:** Comparison of force coefficient time histories from computer simulation and defined by the empirical equations: (a)  $V_{\theta, \max} / U_{\infty} = 1.0$  and (b)  $V_{\theta, \max} / U_{\infty} = 4.3$ .

Empirical equations are able to replicate force coefficient time histories from computer simulation with excellent accuracy when the impinging vortex is much larger than the loaded cylinder. However, as the impinging vortex's size is reduced, the assumption that the entire cylinder surface is exposed to the same velocity becomes increasingly-erroneous. The purpose of this study is to attain the mean component of the lift force coefficient to assess its capability to produce dynamically-amplified structure response. The preceding studies have successfully identified the fundamental structure period that experiences maximum dynamic response amplification. Therefore, now mean lift force coefficient time histories are applied as forcing to the SDOF response model to determine whether or not they excite maximum structure response at the same fundamental periods as do force coefficient time histories from computer simulation.

The empirical equations are used to define the mean lift force coefficient time histories produced by vortices having RCVM, L-O, and S-K profiles, examples of which are illustrated in Figures 6.18.1a – 6.18.1c. Impinging vortex sizes of  $r_c = 2 \cdot D$  to  $10 \cdot D$  are considered for each of the three vortex profiles, while the vortex and stream velocity are fixed  $V_{\theta, \max} = U_{\infty} = 1.0$ . The

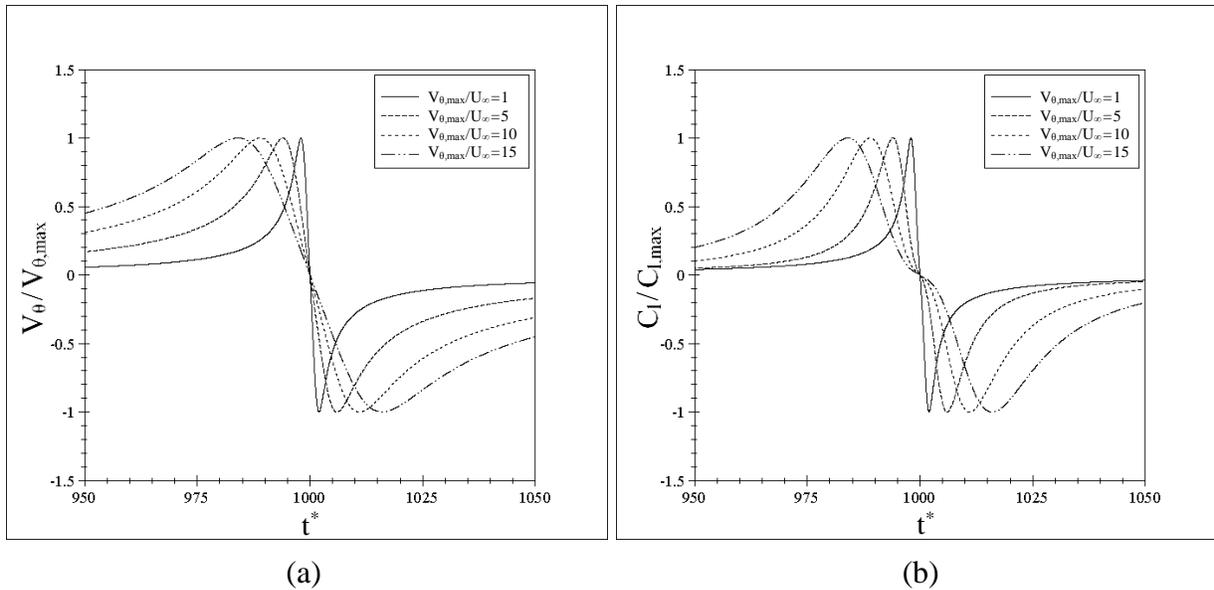
equation-defined forcing is applied to the SDOF response model, and the resulting vortex loading periods are extracted and summarized in Figures 6.18.2a – 6.18.2c. Vortex loading periods defined using the equation-defined forcing are practically identical to those defined by applying vortex loading from computer simulation as forcing. It is therefore concluded that the mean lift force coefficient time histories defined using the empirical equations accurately represents the mean component of the lift force coefficient time history from computer simulation, meaning the empirical equations can now be used to define force coefficient time histories for subsequent analysis.



**Figure 6.18:** (1) Mean lift force coefficient time histories for (a-c) RCVM, L-O, and S-K vortex profiles and (2) comparison of  $T_v$  from DLF of equation-defined forcing (black dots) with Eq. (6.18) (dashed line).

### 6.6.3 Influence of Vortex Velocity Ratio on Vortex Loading Period

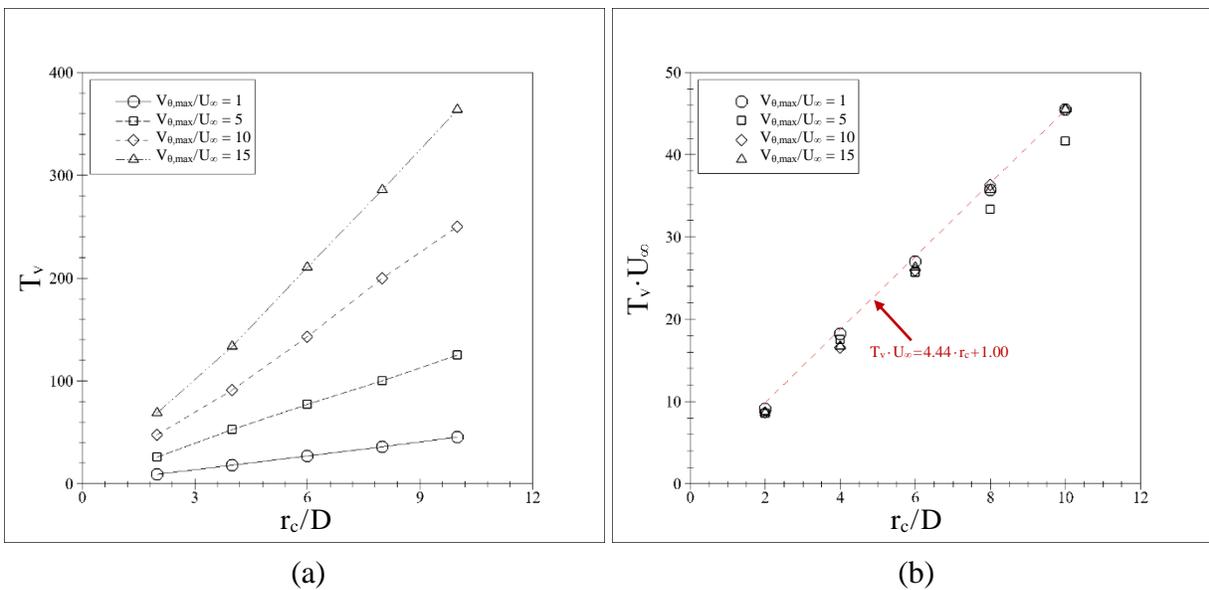
Vortex loading of structures occurs in numerous scenarios, and the vortex velocity ratio  $V_{\theta, \max}/U_{\infty}$  varies greatly depending upon the loading scenario. Blade-vortex interaction is typically characterized by  $V_{\theta, \max}/U_{\infty} \leq 1.0$ , while vortex velocity ratios of documented tornados and hurricanes span  $3.0 \leq V_{\theta, \max}/U_{\infty} \leq 15.0$ . Figures 6.19a and 6.19b respectively illustrated time histories of the normalized tangential velocity incident on the cylinder from the vortex and the resulting lift force coefficient time histories for vortex velocity ratios ranging from  $3.0 \leq V_{\theta, \max}/U_{\infty} \leq 15.0$ . For a velocity ratio of unity, the tangential velocity time history and lift force coefficient time history are similar. However, as  $V_{\theta, \max}/U_{\infty} \rightarrow 15$ , the profile of the lift force coefficient becomes increasingly flatter near  $t^* = 1000$  and deviates from the tangential velocity profile. The profile of the vortex loading controls  $T_v$ ; because the profile visibly change, it is therefore necessary to confirm the validity of the definition of  $T_v$  given by Eq. (6.29).



**Figure 6.19:** Variation in (a) tangential velocity time history and (b) the mean lift force coefficient time history with variation in vortex velocity ratio.

Force coefficient time histories are defined for a L-O vortex having fixed maximum resultant velocity  $V_R = 2.0$  for each of the vortex velocity ratios illustrated in Figure 6.19.

Impinging vortex sizes of  $r_c = 2 \cdot D$  to  $10 \cdot D$  are considered to maintain consistency with the methodology used up to this point. Each forcing profile is applied to the SDOF response model, and the resulting trends  $T_v(r_c)$  are summarized for each vortex velocity ratio in Figure 6.20a. The maximum resultant velocity  $V_R$  is fixed, and the vortex velocity ratio is inversely proportional to  $U_\infty$ ; it follows that the vortex loading period increases with increasing vortex loading period as illustrated in Figure 6.20a. However, Figure 6.20b illustrates that the data in Figure 6.20a compress to the definition of Eq. (6.29).

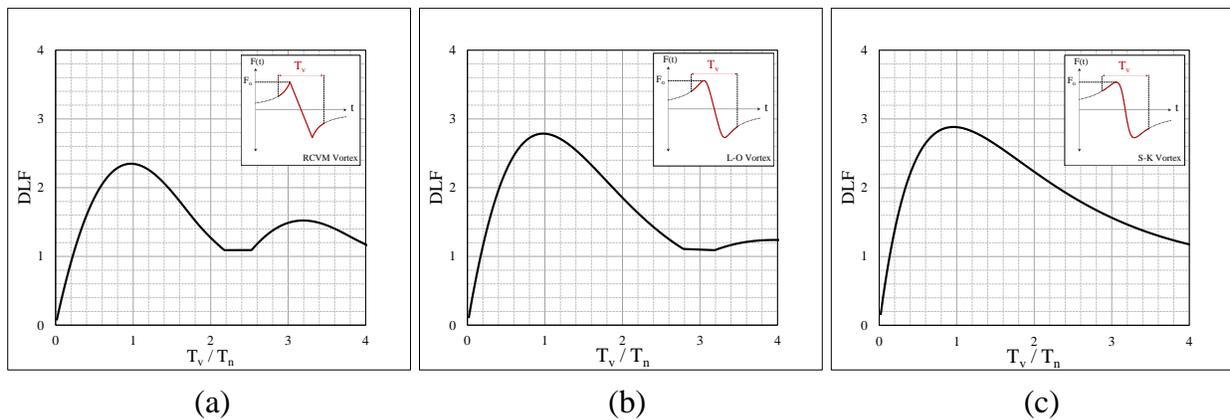


**Figure 6.20:** (a) Vortex loading period curves for considered vortex velocity ratios and (b) comparison of vortex loading periods with the definition given by Eq. (6.29).

The preceding study has served the dual purpose of further validating the definition of the vortex loading period by Eq. (6.29) by further demonstrating the capability of the empirical equations to accurately reproduce cylinder loading from computer simulation. The mean lift force coefficient defined using the empirical equations can now be used with confidence to define DLF curves for vortex-induced loading.

## 6.7 Definition of DLF Curves for Vortex Loading

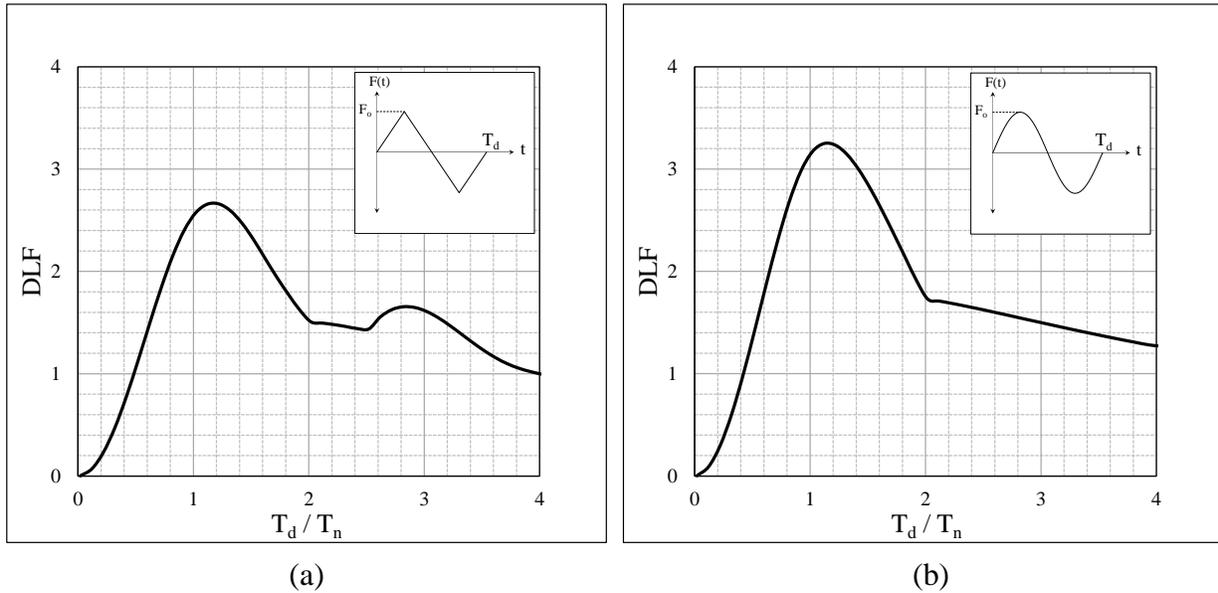
The previous section demonstrates that mean lift force coefficient profiles defined by empirical equations replicate those produced by computer simulation for RCVM, L-O, and S-K vortices. Now these force coefficient time histories are applied as forcing to the SDOF response model, and the procedure outlined in Section 6.4.1 is used to define DLF curves for each of the vortex profiles. For each of the vortex profiles,  $T_v$  is defined using Eq. (6.29). If comparing with Section 6.4.1, recall that  $T_v$  is analogous to  $T_d$ , but simply references to the vortex load application period rather than the load application period of a general dynamic load. Figures 6.21a – 6.21c respectively illustrate the DLF curves for the RCVM, L-O, and S-K vortex profiles.



**Figure 6.21:** DLF curves for vortex loading by vortices having (a) RCVM, (b) L-O, and (c) S-K tangential velocity profiles.

The vortex loading DLF curves change in two primary ways as the vortex profile transitions from sharp (RCVM) to smooth (S-K). First, the maximum value of the DLF curve progressively increases: 2.35 (RCVM)  $\rightarrow$  2.78 (L-O)  $\rightarrow$  2.88 (S-K) as the vortex's profile flattens. Additionally, the pronounced, secondary peaks in the DLF curve (Figure 6.21a) flatten (Figure 6.21b) and disappear (Figure 6.21c) as the vortex profile flattens (RCVM to S-K profile). Both of these trends are observed when DLF curves for the sharp triangle wave and the smooth

sine wave (Figures 6.22a and 6.22b) are compared, providing qualitative validation of the methodology used to define the vortex loading DLF curves Figures 21a – 21c. Now the definition of  $T_v$  given by Eq. (6.29) and the vortex loading DLF curves can be used to define the dynamic amplification of a specified structure's response to loading by a specified vortex.

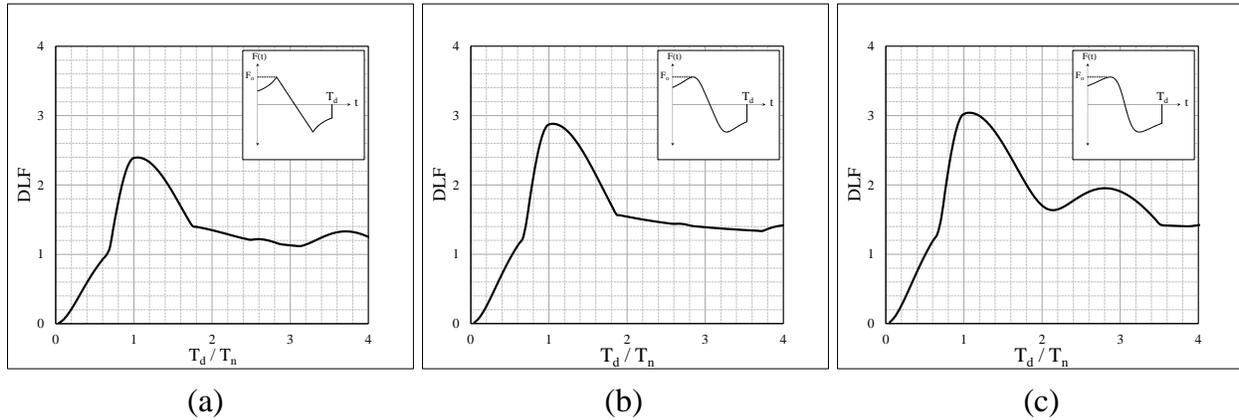


**Figure 6.22:** DLF curves for (a) triangle wave and (b) sine wave forcing profiles.

### 6.7.1 Application of Only $T_v$ as Forcing

An additional study is now conducted where only the region of the vortices' forcing profile defined by  $T_v^*$  as forcing and corresponding DLF curves are constructed. **Figures 6.23a – 6.22c** illustrate DLF curves produced by applying only the portion of the RCVM, L-O, and S-K profiles that constitute the vortex loading period to the SDOF response model. Generally speaking, the maximum amplitudes of these DLF curves are slightly greater than the maximum amplitudes produced when the entire vortex forcing profiles are applied. Additionally, the DLF curves in **Figures 6.23a – 6.23c** are much rougher and more irregular than the DLF curves in **Figures 6.21a – 6.21c**. In reality, application of only the portion of the vortex loading profile defined by  $T_v$  is not realistic of loading by a vortex, where the wind field gradually builds as the

vortex approaches the loaded structure. Instead, it immolates a suddenly-applied load that varies in time before being suddenly removed. Therefore, the DLF curves defined in *Figures 6.20a – 6.20c* are the vortex loading DLF curves that shall be considered in subsequent analysis.



**Figure 6.23:** DLF curves produced by application of only the portion of the vortex forcing profile defined by  $T_v$  for (a) RCVM, (b) L-O, and (c) S-K vortex profiles.

## 6.8 Definition of the Generalized DLF Curve for Tornado Wind Loads

*Section 6.5* develops Eq. (6.29) to compute the vortex load application period  $T_v$  based upon the vortex's size  $r_c$ , translational velocity  $U_\infty$ , and tangential velocity profile. *Sections 6.6 – 6.7* develop DLF curves (*Figures 6.21a – 6.21c*) defining the maximum dynamic response amplification for a structure having specified fundamental period  $T_n$ . Now *Section 6.8* combines these findings along with the documented range of tornado vortex parameters to define the generalized DLF curve for structure response to tornado wind loadings.

### 6.8.1 Tornado Vortex Parameters and Fundamental Structure Periods

The vortex loading period defined in Eq. (6.29) is a function of two vortex parameters,  $r_c$  and  $U_\infty$ , and the vortex's tangential velocity profile, the influence of which is integrated through  $T_v^*$ . It is not possible to define the range of tornado vortex parameters with complete confidence because vortex parameters change continuously throughout the tornado's duration and are notably different between storms. Furthermore, both technological limitations and the random

nature of tornado occurrence limit the documentation of tornado properties. However, the tornado-vortex parameters reported herein are compiled following extensive review of extreme tornado events, and the parameters summarized in Table 6.2 are believed to represent the documented severe-storm parameter range.

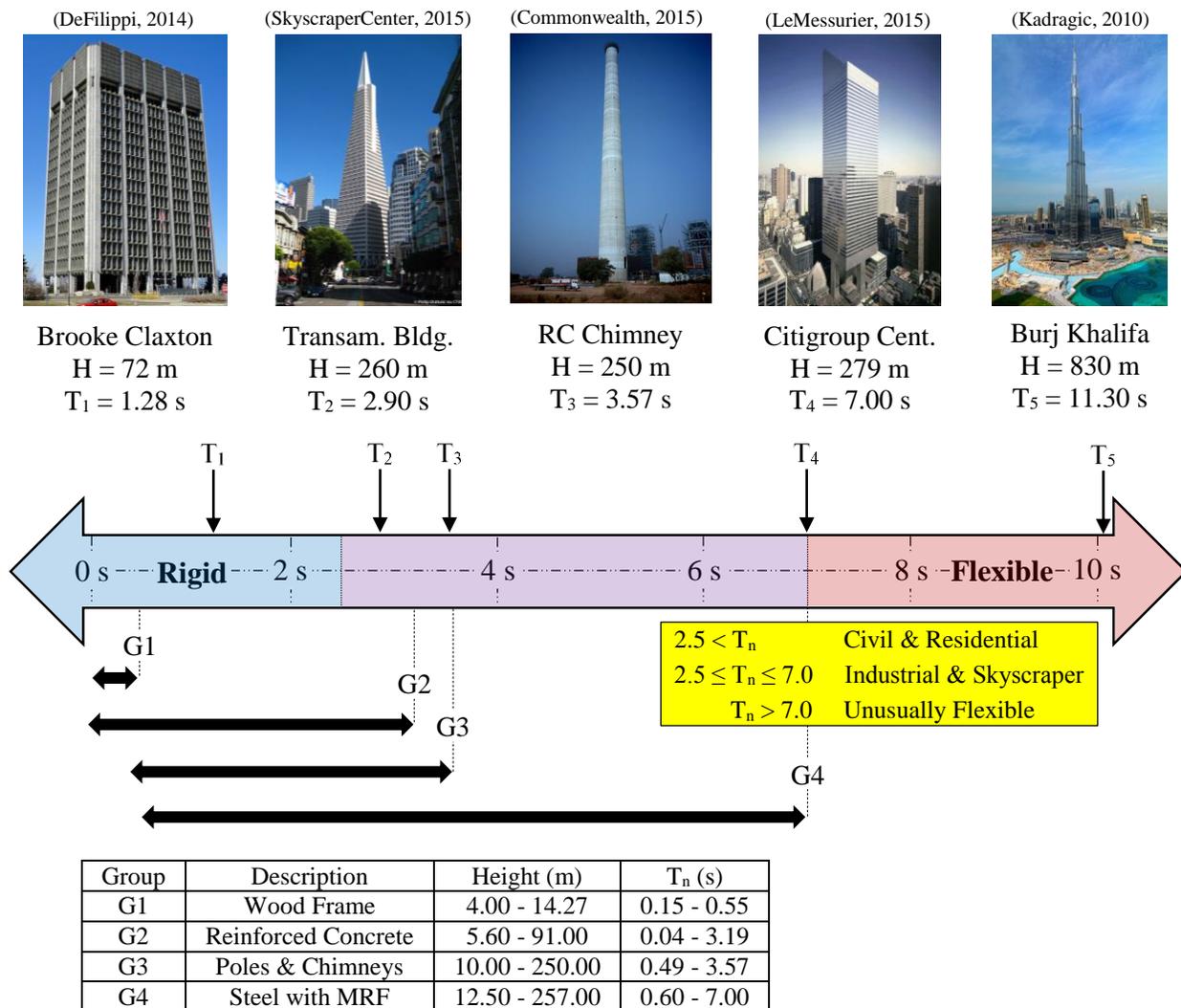
**Table 6.2:** Range of documented tornado vortex parameters for defining  $T_v$  using Eq. (6.18).

Parameter	Units	Minimum	Maximum
$r_c$	m	46.0 (Fujita, 1981)	2008.0 (NOAA, 2013a)
$U_\infty$	$m \cdot s^{-1}$	11.6 (NOAA, 2013b)	32.6 (Fujita, 1973)
$T_v^*$	~~~	4.08	5.02

**Table 6.3:** Fundamental periods for real-world structures.

Structure	Description	$T_n$ (s)	Source
<b>Wood-Frame Structures</b>			
"Typical" Single-Story Residence	Undamaged	0.15 to 0.20	Graf (2008)
...	Damaged	< 0.50	...
4-Story Structure on CC Slab	Undamaged	0.233 to 0.250	Hafeez et al. (2014)
...	Damaged	0.345 to 0.370	...
5-Story Structure on CC Slab	H = 15.24 m	0.40 to 0.55	Thompson (2015)
<b>Civil Structures and Skyscrapers</b>			
Shear Wall Structures	RC, (12 x 8 x 5.6m)	0.035	Balkaya and Kalkan (2003)
...	RC, (27 x 24 x 42m)	0.918	...
Buildings with Load Bearing Walls and Moment-Resisting Frames	RC, (71 x 10.98 x 8.45m)	0.146 to 0.186	Hong and Hwang (2000)
Nuclear Reactor Housing	RC, (51.64 x 17 x 77.1m)	0.877 to 1.449	...
13 Buildings with Load Bearing Walls	RC, Dome-Roofed	0.15 to 0.50	Chopra (2005)
Morrow Point Dam	RC, H = 15 m	0.155 to 0.294	Kuz. and Was. (2006)
37 Moment-Resisting Frame Structures	RC, H = 139 m	0.268 to 0.303	Chopra (2005)
PineFlat Dam	RC, 9 < H < 91 m	0.27 to 3.19	Goel and Chopra (1997)
21 Buildings in LA, CA	RC, H = 122 m	0.288 to 0.306	Chopra (2005)
53 Moment-Resisting Frame Structures	RC and Steel, 20 < H < 100 m	0.397 to 3.704	Todorovska et al. (2005)
Medical Center, Richmond CA	Steel, 12.5 < H < 257 m	0.60 to 6.50	Goel and Chopra (1997)
National Health/Welfare Building	Steel, H ≈ 10 m	0.63 to 0.74	Chopra (2005)
Alcoa Building	RC and Steel, (43 x 27 x 72m)	0.99 to 1.28	Crawford and Ward (1964)
Transamerica Building	Steel, H = 120 m	1.67 to 2.21	Chopra (2005)
Golden Gate Bridge Main Span	Steel, H = 260 m	2.9	...
601 Lexington Ave. (Citigroup Cent.)	L = 1280 m	3.81 to 18.2	...
Burj Khalifa	RC and Steel, H = 279 m	7.0	FEMA (2006)
	H = 830 m	11.3	Baker (2010)
<b>Slender Structures</b>			
Anemometer Pole	Steel Tube, H = 10 m	0.4902	Repetto and Solari (2010)
Industrial Chimneys	Steel Tube, H = 25 m	0.775	Repetto and Solari (2002)
...	Steel Tube, H = 30 m	0.787	...
...	Steel Tube, H = 100 m	2.058	...
...	RC, H = 250 m	3.57	Chopra (2005)
Antenna Pole	Steel Tube, H = 30 m	0.92 to 1.09	Repetto and Solari (2010)
Urban Light Pole	Steel, H = 14 m	1.85	Repetto and Solari (2004)

Table 6.3 compiles measured fundamental periods of real-world structures, which, vary greatly, ranging from rigid, single-story concrete structures ( $T_n = 0.035$  s) to very flexible structures such as the main span of the Golden Gate Bridge ( $T_n = 18.2$  s). It is convenient to define four groups of structures based upon their fundamental periods: “G1 – Wood Frame” ( $0.15 \leq T_n \leq 0.55$ ), “G2 – Reinforced Concrete” ( $0.04 \leq T_n \leq 3.19$ ), “G3 – Poles and Chimneys” ( $0.49 \leq T_n \leq 3.57$ ), and “G4 – Steel Structures” ( $0.60 \leq T_n \leq 7.00$ ). Figure 6.24 illustrates G1-G4 along with examples of structures having fundamental periods within the specified ranges.



**Figure 6.24:** Illustration of the four structure period groups G1-G4 and examples of structures within these ranges.

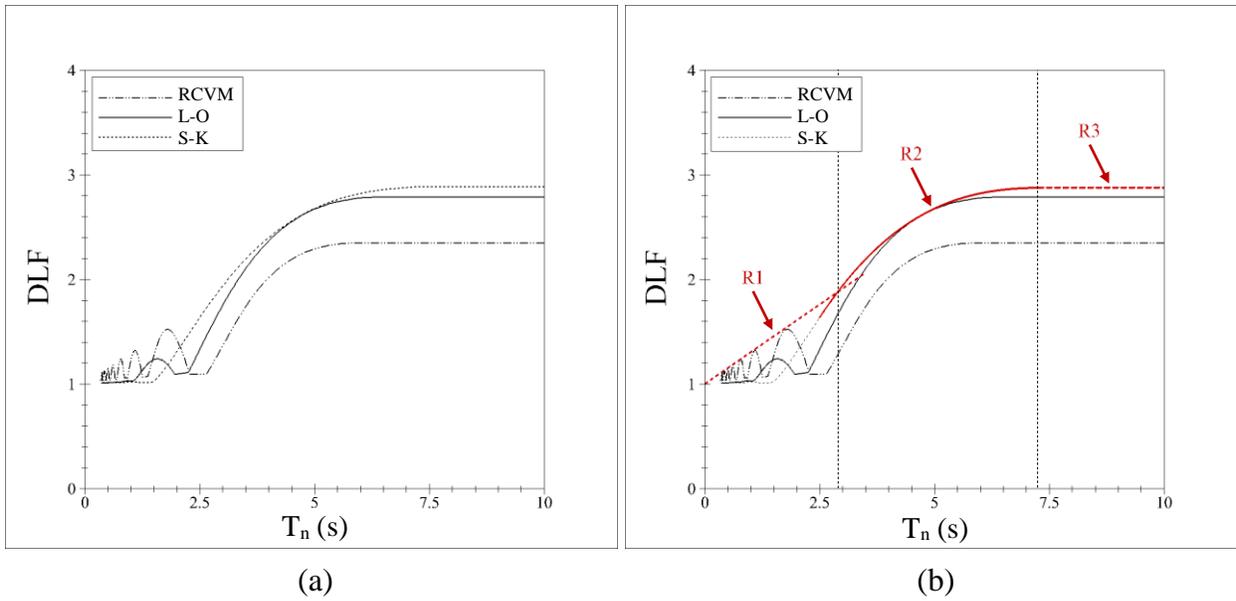
### 6.8.2 Generalized DLF Curve for Tornado Wind Loads

Eq. (6.29) defines that  $T_v$  is proportional to  $r_c$  and  $T_v^*$  while being inversely proportional to  $U_\infty$ . The vortex load application period  $T_v$  is smallest when  $r_c/U_\infty$  is minimized ( $r_{c,\min}/U_{\infty,\max} = 46 \text{ m}/32.6 \text{ m}\cdot\text{s}^{-1} = 1.41 \text{ s}^{-1}$ ) and largest when  $r_c/U_\infty$  is maximized ( $r_{c,\max}/U_{\infty,\min} = 2008 \text{ m}/11.6 \text{ m}\cdot\text{s}^{-1} = 173.10 \text{ s}^{-1}$ ). Documented tornado-vortex parameters span a wide range, and any vortex load application period in the range  $T_{v,\min} < T_v < T_{v,\max}$  is possible. Table 6.4 summarizes  $T_{v,\min}$  for each profile based upon the documented tornado vortex parameters in Table 6.2 and the definition of  $T_v$  provided by Eq. (6.29). The response of any structure for which  $T_n \geq T_{v,\min}$  can experience maximum dynamic amplification of tornado wind loads. Structures for which  $T_n < T_{v,\min}$  require that  $\text{DLF}(T_v/T_n)$  be defined using the DLF curves in Figures 6.21a – 6.21c, where the ratio  $T_v/T_n$  is computed using  $T_v = T_{v,\min}$ .

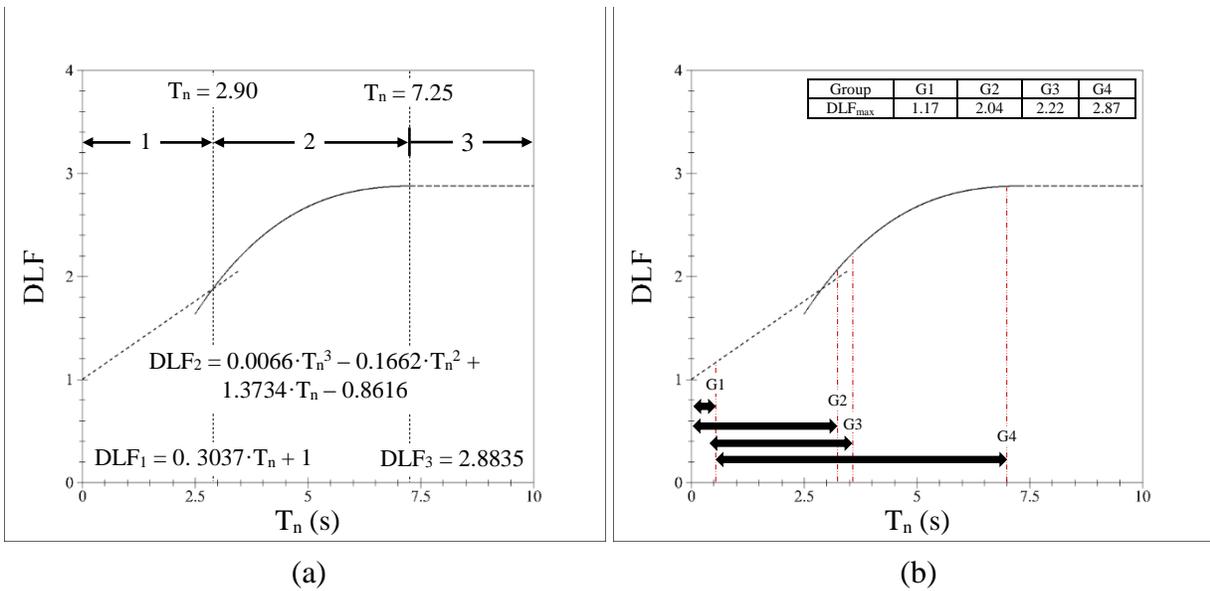
**Table 6.4:** Minimum vortex loading period for each vortex profile.

Profile	$T_v^*$ (dim)	$r_{c,\min}$ (m)	$U_{\infty,\max}$ (m/s)	$T_{v,\min}$ (s)
RCVM	4.08	46.00	32.60	5.76
L-O	4.44	...	...	6.27
S-K	5.02	...	...	7.17

Figure 6.25a plots  $\text{DLF}(T_n)$  curves for each of the three vortex profiles (Recall that these three profiles represent the range of measured tornado-vortex profiles). Now a general DLF curve for tornado wind loading is defined by curve fitting the upper bound of the three DLF curves using the three region (R1-R3) envelope, which is shown by the red line in Figure 6.25b. The three regions are: (R1) a linear region from  $0 < T_n \leq 2.90 \text{ s}$ , (R2) an exponential region from  $2.90 \text{ s} < T_n < 7.25 \text{ s}$ , and (R3) a constant region for  $T_n \geq 7.25 \text{ s}$ . The general tornado-vortex DLF curve is re-plotted in Figure 26a, and the equations for R1-R3 are provided.



**Figure 6.25:** (a) DLF curves for three vortex tangential velocity profiles and (b) 3-region, piecewise curve fit defining the maximum DLF value for each fundamental structure period.



**Figure 6.26:** (a) Generalized maximum DLF curve for tornado wind loads as function of  $T_n$  and (b) illustration of maximum DLF values for structure groups G1-G4.

Figure 26b illustrates where each of the four structure groups (G1-G4) fall along the general tornado-vortex DLF curve. Typical wood frame structures (G1) will experience minimal dynamic amplification of tornado wind loads. However, tall concrete (G2) and steel (G4) structures as well as slender structures such as chimneys and poles (G3) may experience

significant dynamic response amplification to tornado wind loads. The potential for dynamic response amplification to tornado wind loads is relevant and should be weighed in the design and construction of such structures.

### 6.8.3 Practical Assessment of Tornado Dynamic Loading Threat

Figure 6.26a shows that the responses of structures having  $T_n \geq 7.25$  s to tornado wind loads may be dynamically amplified by up to 2.88 times. There are very few practical structures with  $T_n \geq 7.25$  s; however, DLF values of 2.0 are possible for structures with  $T_n$  as low as 3.10 s, which is well within the practical fundamental frequency range for multi-story steel and concrete structures. Furthermore, the fact that tornado-like vortices have been shown to produce up to 1.5 times greater lateral wind loading than straight line wind ((Selvam and Millet, 2005) and (Hann et al., 2010)) must also be considered. Computation of tornado wind loads for tall structures assuming the tornadic wind can be treated as straight-line wind could be quite erroneous as lateral tornado wind loadings may easily be in excess of  $2.0 \cdot 1.5 = 3.0$  times greater than those produced by straight line winds.

However, prior to suggesting that structures need be designed for this extreme level of load amplification, the basis and methodology of this study must be taken into account. Specific assumptions that need to be considered are the tornado vortex parameters and the assumed manner in which the tornado loading is applied to the structure.

The tornado vortex parameters used to defined the minimum vortex loading period  $T_{v,\min}$  are those that minimize  $T_v$  as defined in Eq. (6.18) and are specifically  $U_{\infty,\max} = 32.6 \text{ m}\cdot\text{s}^{-1}$  and  $r_{c,\min} = 46 \text{ m}$ . These parameters ( $U_{\infty,\max}$  and  $r_{c,\min}$ ) are not documented from the same tornado; in fact, the loading periods of the tornados from which they are documented are respectively  $T_v > 100 \text{ s}$  and  $T_v > 12 \text{ s}$ . This is not to say that dynamic loading by tornado winds is not possible, as

there are many documented small, rapidly translating vortices in the literature. Furthermore, tornado core radii dimensions reported in the literature are primarily approximated from field-observation or post-storm damage investigation and likely over-predict the actual core radii. Finally, small vortices orbiting within “multi-vortex” tornados have been documented (Lewellen et al., 2000) and may themselves have greater propensity to dynamically amplify structure response than the larger parent cyclone. In short, dynamic amplification or tornado wind loads requires a very small, rapidly-translating tornado and is not likely a significant concern.

The premise of the present analysis is that the same vortex loading is applied to the entire loaded structure and that the structure does not disrupt the vortex during their interaction. Restating the summary of the previous paragraph, small, rapidly-translating tornados produce a dynamic loading threat. The possible dynamic response amplification increases rapidly with  $T_n$ ; generally speaking, structures that may experience dynamic response amplification are multistory steel and concrete buildings. Such structures have large footprints and cross sections, hence there is question as to whether or not a small tornado can produce uniform loading across the structure’s cross section. Furthermore, three-dimensional simulation reported in Gorecki and Selvam (2015) shows that impact of a vortex with a prismatic structure substantially disrupts the vortex. If the vortex is disrupted, the forcing profile that it produces on the loaded structure will change as well. Once again, this speculation does not remove the concern of dynamic load amplification of tornado wind loads, rather it opens the door for subsequent investigation using three-dimensional computer simulation to account for the disruption of the impinging vortex by the loaded structure.

## 6.9 Summary and Conclusions

The dynamic load factor (DLF) concept has been applied to define the first generalized assessment of the dynamic loading threat of tornado-like wind loads to real-world structures. Two-dimensional loading of a rigid, circular cylinder by an impinging vortex is directly simulated. The resulting cylinder loading is applied as forcing to a single degree of freedom (SDOF) response model, and the vortex loading period  $T_v$  is defined as the fundamental structure frequency at which the dynamic amplification of the SDOF structure's response is greatest. Three impinging vortex tangential velocity profiles (RCVM, L-O, and S-K) are considered because they represent the range of documented tornado vortex tangential velocity profiles. The vortex loading period  $T_v$  is defined as a function of the vortex's critical radius  $r_c$ , translational velocity  $U_\infty$ , and tangential velocity profile. Documented tornado-vortex parameters are surveyed leading to the definitions of the core radii range of  $46 \text{ m} \leq r_c \leq 2008 \text{ m}$  and translational velocity range of  $11.60 \text{ m} \cdot \text{s}^{-1} \leq U_\infty \leq 32.60 \text{ m} \cdot \text{s}^{-1}$ . The possible dynamic amplification of a structure's response to cross-stream tornado wind loads is defined as function of  $T_n$ . Based upon the content presented in Chapter 6, the conclusions outlined below have been reached.

1. The load-application period of a vortex's tangential wind field is " $T_v = T_v^* \cdot r_c \cdot U_\infty^{-1}$ ".
  - a. Documented tornado-vortex parameters:
    - i. Critical radii:  $46 \text{ m} \leq r_c \leq 2008 \text{ m}$
    - ii. Translational velocity:  $11.60 \text{ m} \cdot \text{s}^{-1} \leq U_\infty \leq 32.60 \text{ m} \cdot \text{s}^{-1}$
  - b. Influence of vortex's tangential velocity profile incorporated through dimensionless vortex loading period:
    - i.  $T_v^* = 4.08$  – RCVM vortex profile
    - ii.  $T_v^* = 4.44$  – L-O vortex profile

iii.  $T_v^* = 5.02$  – S-K vortex profile

2. The maximum dynamic response amplification of a structure having fundamental period

$T_n$  to tornado wind loads is:

a.  $2.90 \text{ s} \geq T_n \rightarrow \text{DLF} = 0.3037 \cdot T_n + 1$

b.  $2.90 \text{ s} < T_n < 7.25 \text{ s} \rightarrow \text{DLF} = 0.0066 \cdot T_n^3 - 0.1662 \cdot T_n^2 + 1.3734 \cdot T_n - 0.8616$

c.  $T_n \geq 7.25 \text{ s} \rightarrow \text{DLF} = 2.8835$

3. Dynamic amplification of tornado wind loads:

a. Requires a small, rapidly-translating tornado

b. Is not a concern for typical wood-frame structures and residences

c. Is possible and a concern for tall, flexible structures

## CHAPTER 7: SUMMARY AND CONCLUSIONS

### 7.1 Summary

Structure loading from impacting vortices is a hazardous phenomenon that is encountered in numerous engineering applications. The complex physics governing vortex-structure interaction necessitate study using either physical experiment or computer simulation. Investigation via either methodology is fraught with challenges, hence the amplitude and character (static or dynamic) of the induced loading is at best poorly understood.

Structure loading by vortices occurs across a wide spectrum of relative vortex-to-structure size scales. Rotor- and wing-tip vortices are typically smaller or similar-sized to the components of aerospace vehicles that they impact. Tornado and hurricane vortices range from similar-sized to many times larger than the structures that they load. The primary objective of this thesis is to grow the current knowledge of vortex loading of structures by evaluating the influence of the impinging vortex's size on the amplitude and character of the structure loading that it produces. This knowledge can be applied to better predict air loads on aerospace vehicles and to better design residential and civil structures to withstand wind loadings from tornados and hurricanes.

A two-dimensional computer model is used to directly simulate the impact of an impinging vortex with a slender, cylindrical structure. A rigorous review of analytical and measured vortex tangential velocity profiles (TVPs) is conducted, and analytical models which realistically represent the intense vortices encountered in engineering applications are integrated into the computer model. The vortex's maximum tangential velocity is fixed, and its critical radius is incrementally increased from one to one-hundred times the structure's diameter. The structure loading amplitude trend with respect to impinging vortex size is documented, and

phenomenon that control the trend are illustrated and explained. The vortex's path is then shifted so that it travels both above and below the structure. The trend in structure loading amplitude with respect to this path shift is then documented, and the path shift that produces maximum loading amplitude is identified. Finally, a generalized methodology is developed to assess the dynamic amplification of a structure's response to vortex loading. The dynamic load factor concept is applied to define the duration of the load application period as a function of the impacting vortex's parameters. Documented tornado-vortex parameter ranges are established and used to define the first generalized expression for possible dynamic amplification of tornado wind loads as function of the structure's fundamental period. Documented fundamental periods of existing structures are compiled, and the dynamic loading threat posed by tornados to typical civil and residential structures is assessed.

## **7.2 Conclusions**

The subsequent subsections restate the five primary objectives of these thesis as Objectives 1-5. Each objective is restated, the methodology utilized to achieve each objective is briefly summarized, and the conclusions presented while pursuing each objective are summarized. The conclusions reached herein directly apply only to the considered physical system, which is two-dimensional impact of a vortex with a slender, cylindrical structure having the parameters specified in Section 4.1.1. However, the methodology utilized herein can be used to extend the findings presented subsequently to any physical vortex-structure system and parameter range.

### 7.2.1 Objective 1

❖ *To select physically-realistic analytical vortex tangential velocity profiles (TVPs) for use in computer simulation of structural loading by vortices.*

Physically-realistic analytical vortex TVPs are required to simulate physically-realistic vortex-structure interaction. Measured vortex TVPs are qualitatively similar but differ quantitatively, hence no single analytical vortex TVP replicates all measured vortex TVPs. Measured vortex TVPs were exhaustively surveyed from documented experimental and atmospheric vortices. Analytical vortex TVPs were compiled and normalized for comparison with the measured vortex TVPs. A group of analytical TVPs that adequately represent the spectrum of measured TVPs were identified for use in computer-simulated vortex-structure interaction. The conclusions reached while pursuing Objective 1 are outlined subsequently.

- ✓ The algebraic Vatistas or Wood-White TVPs are ideal for integration into computer models because they can replicate numerous analytical vortex TVPs.
- ✓ Three analytical vortex TVPs represent the spectrum of measured vortex TVPs
  - The normalized S-K (Vatistas  $n = 1$ ) TVP is the upper bound to the measured TVPs.
  - The normalized L-O (Vatistas  $n = 2$ ) TVP replicates the typical measured TVP.
  - The Wood-White  $\lambda = 0.435$  TVP is the lower bound to the measured TVPs.
- ✓ The normalized L-O (Vatistas  $n = 2$ ) TVP is the best representation of the typical measured vortex TVP and should be utilized as the default for computer simulation unless alternative justification is provided.

### 7.2.2 Objective 2

❖ *To identify the phenomenon that cause variation in structural loading when an impinging vortex impacts a structure, which is shedding vortices, at different times and to quantify the possible variation in the maximum force coefficient amplitude.*

An impinging vortex with critical radius  $r_c$  equal to the structure diameter  $D$  produces a spectrum of different structural loading when it impacts the structure at different times with respect to the beginning of the computer simulation. For all computer simulations, the free stream establishes vortex shedding from the cylinder prior to vortex impact with the structure. The relative size and strength of the vortices produced on the structure by the free stream change continuously during vortex shedding. It was postulated that interaction between the impinging vortex and different attached vortex structures causes the variation in structural loading when the impinging vortex impacts the structure at different times.

Impinging vortex impact was simulated at twenty-five discrete times within a single period of the vortex shedding cycle (VSC), and signed maximum drag  $C_d$  and lift  $C_l$  force coefficient amplitudes were extracted and utilized to assess the variation in structural loading amplitude with vortex impact time. Contour plots of vorticity and velocity were used to illustrate the interaction between impinging and attached vortices. A procedure was introduced to ascertain the variation in maximum force coefficient amplitudes with vortex impact time using five computer simulations instead of twenty-five. The influence of the impinging vortex's size on the variation in maximum force coefficient amplitude with vortex impact time was then assessed by incrementing the impinging vortex's critical radius from one to one-hundred times the structure's diameter. The conclusions reached while pursuing Objective 2 are outlined subsequently.

- ✓ Interaction between the impinging vortex and vortices produced on the structure by the free stream causes variation in structural loading amplitude when the impinging vortex impacts the structure at different times with respect to the free stream VSC.
  - Maximum force coefficient amplitudes produced by an  $r_c = 1 \cdot D$  vortex may be under-predicted by 84.99%, 197.04%, 310.42%, and 407.15% [ $Cl'$  (+),  $Cl'$  (-),  $Cd'$  (+), and  $Cd'$  (-)] if only a single vortex impact is simulated.
- ✓ Variation in maximum force coefficients when the vortex impacts at different times is negligible for  $r_c \geq 20 \cdot D$ . Large vortices control vortex shedding from the structure prior to impacting, so the vortex impact time with respect to the free stream VSC is irrelevant.

### 7.2.3 Objective 3

- ❖ *To define the trend in maximum force coefficient amplitudes produced by a directly-impacting vortex when the vortex size is increased at fixed maximum tangential velocity and to explain the phenomenon controlling the trend in maximum force coefficient amplitudes.*

Structural loading by vortices occurs at practically all relative vortex-to-structure size scales, and adequate design of structures to resist vortex loading necessitates understanding how the relative vortex-to-structure size influences the resulting structural loading amplitude. The impinging vortex's maximum tangential velocity was fixed, direct impact of vortices ranging from  $r_c = 1 \cdot D$  to  $100 \cdot D$  with the structure was simulated, and maximum force coefficient amplitudes from the simulations were utilized to document the influence of relative vortex-to-structure size on the structural loading amplitude. The aforementioned study, first performed with the Vatisas  $n = 2$  vortex, was repeated with the Vatisas  $n = 1$  and  $n = 100$  vortices to evaluate if the vortex's TVP independently influences trend. Finally, contour plots of vorticity and velocity were utilized to illustrate underlying phenomenon that control the trend in structural

loading amplitude with respect to the impinging vortex's size. The conclusions reached while pursuing Objective 3 are outlined subsequently.

- ✓ Maximum force coefficient amplitudes increase to asymptotic values for impinging vortex sizes  $r_c \geq 50 \cdot D$ . The impinging vortex must be large enough to satisfy three requirements:
  - Be large enough to control vortex shedding from the structure prior to impacting.
  - Have a small enough tangential velocity gradient to allow the structure's wake to shift without distortion.
  - Be large enough to expose the entire structure surface to maximum tangential velocity.
- ✓ Asymptotic ( $r_c \geq 50 \cdot D$ ) maximum drag and lift force coefficients from direct vortex impact are respectively 94.77% and 10.66% less than maximum force coefficients produced by the equivalent-velocity free stream.
  - Forces on a structure immersed in a fluid stream increase with the square of the stream's resultant velocity. The maximum stream velocity incident on the structure for direct vortex impact is horizontal  $U_\infty = 1$  and vertical  $V_{\theta, \max} = 1$ , so the maximum resultant velocity on the structure is  $(2)^{0.5}$ . An equivalent-velocity free stream with  $U_\infty = 2$  produces the much greater resultant velocity over the structure of  $(4)^{0.5}$ , hence produces greater structural loading.
- ✓ The impinging vortex's TVP does not significantly influence the trend in maximum force coefficient amplitudes with increasing vortex size or the asymptotic maximum force coefficient values.

- The Vatisas  $n = 1$  TVP produces greatest maximum force coefficients, and maximum force coefficients produced by the Vatisas  $n = 2$  and Vatisas  $n = 100$  respectively agree within 1.0% and 7.0%.

#### 7.2.4 Objective 4

❖ *To evaluate the influence of shifting an impacting vortex's path from the structure's centerline on the maximum structural loading produced by the vortex and to determine if the vortex produces greater structural loading than an equivalent-velocity free stream.*

An impinging vortex may approach and impact a structure travelling along a path other than the structure's centerline. The impinging vortex may rotate clockwise (CW) or counterclockwise (CCW), so its path shift with respect to the structures' centerline will cause the vortex's tangential velocity to either augment or subtract from the free stream velocity. The present study incremented the path of an  $r_c = 50 \cdot D$ , CCW-rotating vortex so that it travelled a range of paths both above and below the loaded structure's centerline. Maximum force coefficient values were extracted, used to document the influence of vortex path shift on structural loading amplitude, and compared with maximum force coefficients produced by an equivalent-velocity free stream. The conclusions reached while pursuing Objective 4 are outlined subsequently.

- ✓ The CCW-rotating vortex respectively produces greater and less structural loading when it travels above and below the structure because its tangential velocity respectively augments and subtracts the free stream velocity.
- ✓ The impacting vortex produces maximum drag and lift force coefficients that are respectively 4.80% and 34.07% greater than maximum force coefficients produced by the equivalent-velocity free stream.

### 7.2.5 Objective 5

❖ *To develop a generalized methodology to assess dynamic amplification of structures' response to loading produced by an impacting vortex and to apply the methodology to assess the dynamic loading threat that tornados pose to residential and civil structures.*

The impact of an impinging vortex with a structure is a transient loading event that may produce dynamically-amplified structure response. Previous studies have shown that structures' responses to tornado-like vortex loading may be dynamically amplified, but no generalized, applicable methodology is in place to assess the possible dynamic amplification of a structure's response to vortex loading. Failure to account for dynamic loading effects may result in dramatic under-prediction of loading produced by an impacting vortex, necessitating the development of generalized approach to compute dynamic load amplification from an impacting vortex.

The present study simulated direct vortex impact with a slender structure and analyzed the forcing time history produced on the structure to define the impinging vortex's loading period  $T_v$ . The vortex loading period  $T_v$  could not be clearly distinguished by taking the Fourier transform of the forcing time history. However,  $T_v$  was clearly distinguished by dynamic load factor (DLF) analysis, specifically by analyzing the responses of structures having a spectrum of fundamental frequencies to the forcing time history. The vortex loading period  $T_v$  was defined as a function of three parameters ( $r_c$ ,  $U_\infty$ , and  $T_v^*$ ) which respectively incorporate the influences of the impinging vortex's critical radius, translational velocity, and tangential velocity profile (RCVM, L-O, or S-K). Documented tornado vortex parameters were compiled to establish the known range of tornado vortex core radius and translational velocity. The definition of  $T_v$  along with the documented tornado vortex parameter ranges were then used to define the possible dynamic amplification of a structure having fundamental period  $T_n$  to tornado vortex loading.

Fundamental periods of existing structures were then surveyed, and the dynamic loading threat that tornados pose to real-world residential and civil structures was assessed. The conclusions reached while pursuing Objective 5 are outlined subsequently.

- ✓ A Fourier transform of the forcing time history produced by the vortex is not able to distinguish  $T_v$ , while DLF analysis of the same forcing is able to distinguish  $T_v$ .
  - The current hypothesis is that only a single period of the vortex loading is present in the forcing time history, so Fourier analysis is not able to resolve  $T_v$ .
  - An avenue to future study has been opened to assess why analysis of structures' responses can be used to isolate  $T_v$ , while frequency decomposition via Fourier analysis cannot be used to isolate  $T_v$ .
- ✓ The loading period of a directly-impacting vortex's tangential wind is  $T_v = T_v^* \cdot r_c \cdot U_\infty^{-1}$ .
  - Documented tornado-vortex parameter ranges are:
    - Critical radii:  $46 \text{ m} \leq r_c \leq 2008 \text{ m}$
    - Translational velocity:  $11.60 \text{ m} \cdot \text{s}^{-1} \leq U_\infty \leq 32.60 \text{ m} \cdot \text{s}^{-1}$
  - The influence of the vortex's tangential velocity profile is incorporated through the dimensionless vortex loading period:
    - $T_v^* = 4.08$  – RCVM vortex TVP
    - $T_v^* = 4.44$  – L-O vortex TVP
    - $T_v^* = 5.02$  – S-K vortex TVP
- ✓ The maximum dynamic response amplification (DLF value) of a general structure having fundamental period  $T_n$  to a directly-impacting vortex's tangential wind field is:
  - $2.90 \text{ s} \geq T_n \quad \rightarrow \quad \text{DLF} = 0.3037 \cdot T_n + 1$
  - $2.90 \text{ s} < T_n < 7.25 \text{ s} \quad \rightarrow \quad \text{DLF} = 0.0066 \cdot T_n^3 - 0.1662 \cdot T_n^2 + 1.3734 \cdot T_n - 0.8616$

- $T_n \geq 7.25 \text{ s} \rightarrow \text{DLF} = 2.8835$
- ✓ Generally speaking, dynamic amplification of tornado wind loads:
  - Requires a atypical small, rapidly-translating tornado.
  - Is not likely a concern for typical wood-frame structures and residences.
  - Is possible and a concern for tall, flexible structures.

### 7.3 Primary Contributions to the Scientific Community

The body of work presented in this thesis substantially advances the scientific community's knowledge of structural loading produced by impacting vortices. The four primary contributions from this body of work are listed subsequently.

1. The normalized Vatistas vortex TVP is introduced in Eq. (3.19) and is shown to be capable of replicating the spectrum of measured vortex TVPs in the literature.
2. The concept that interaction between an impinging vortex and vortices attached to a structure causes great variation in structural loading when the impinging vortex impacts the structure at different times with respect to the vortex shedding cycle is identified and explained.
3. The vortex loading period  $T_v$  of an impacting vortex's tangential wind field is defined by Eq. (6.29), as a part of the first generalized methodology to assess dynamic amplification of structures' response to vortex loading.
4. Documented tornado vortex parameters and the vortex loading period  $T_v$  are utilized to define the possible dynamic amplification of loading produced by a tornado's tangential wind field as function of the loaded structure's fundamental period (See Figure 6.26b).

Diligent effort has been made to rapidly disseminate the findings of this thesis. The content has been submitted as four peer-reviewed papers (two have been accepted and the two others are

currently under review) and presented as three technical posters. References for the papers are listed below, and a complete list of the author's publication history is provided in the vitae at the end of the thesis.

Strasser, M.N., Yousef, M.A., and Selvam, R.P., 2015. Defining the Vortex Loading Period and Application to Assess Dynamic Amplification of Tornado-Like Wind Loading, In Review with *J. Fluids and Structures*, Manuscript Number: YJFLS-D-15-00381.

Strasser, M.N. and Selvam, R.P., 2015. The Influence of Vortex Size on the Maximum Loading of a Slender, Cylindrical Structure Impacted by Aerospace to Tornado-Scale Vortices, In Review with *J. Fluids and Structures*, Manuscript Number: YJFLS-2015-126.

Strasser, M.N., & Selvam, R.P., 2015. Selection of a Realistic Vortex Tangential Velocity Profile for Computer Simulation of Vortex-Structure Interaction, *J. Arkansas Academy of Science* **69**, Manuscript Number: JAAS MS69-12.

Strasser, M.N., & Selvam, R.P., 2015. The variation in the maximum loading of a circular cylinder impacted by a 2D vortex with time of impact, *J. Fluids and Structures* **58**, 66-78.

#### **7.4 Limitations of the Present Study**

As stated in the opening paragraphs of Section 7.2, the conclusions reached within the scope of the present study only pertain directly to physical system and parameter ranges considered in the present study. The computer model utilized herein simulates two-dimensional impact of a vortex with a slender, cylindrical structure. For the considered Reynolds number range of less than 300 used throughout this study, vortex shedding from slender, cylindrical structures is known to be primarily two-dimensional in nature. However, it is not currently known whether or not impact by an impinging vortex may produce three-dimensional phenomenon in the vortex-structure interaction.

The assessment of dynamic loading produced by tornado vortices is conducted by applying lift force coefficient time histories from simulated, two-dimensional impact of an impinging vortex with a slender structure to a single degree of freedom system. In reality, a tornado produces time-varying structural loading along all three of the structure's axis. The

structure's response to simultaneous application of forcing in all three directions needs to be considered for a more realistic assessment of the dynamic loading threat that tornados pose. Furthermore, a tornado's wind field varies moving upwards from the ground until the edge of the boundary layer from the ground's surface is reached, meaning the loading produced by the tornado varies as well. The present analysis does not account for this variation, hence it is only physically realistic for impact of a tornado with a tall, slender structure where the variation in forces along the structure's height is minimal.

### **7.5 Suggested Future Work**

At the conclusion of this body of work, several additional topics have been identified as potential avenues for future researchers. These topics are outlined and discussed subsequently in what the author believes to be their order of importance.

Forcing time histories from computer-simulated, direct vortex impact with a slender structure were analyzed to define the loading period of a vortex's tangential wind field  $T_v$  and the possible dynamic amplification of structural loading produced by tornados' tangential wind fields. Dynamic load factor (DLF) analysis was utilized to identify  $T_v$  because a Fourier transform of the forcing time history failed to isolate  $T_v$ . This raises question as to why  $T_v$  can be distinguished from structures' responses in DLF analysis but not from decomposition of the constitutive frequencies of the forcing time history via Fourier analysis. The present hypothesis is that only a single period of vortex loading is present in the force coefficient time history, so it cannot be adequately resolved. One factor that may diminish the clarity of the Fourier transform is that when the exact frequency vortex loading frequency  $f_v = T_v^{-1}$  is not considered in the Fourier analysis, the amplitude associated with  $f_v$  is distributed to adjacent frequencies. Additionally, the vortex shedding frequency from the structure changes continuously as the

impinging vortex approaches the structure, so the non-stationary nature of the forcing time history may also reduce clarity of the Fourier transform. Regardless of the source of the discrepancy, analysis of structures' response has been shown to be a superior means to frequency decomposition to identify forcing frequencies within a forcing time history produced by an impacting vortex. Subsequent studies are needed to assess why DLF analysis provides superior resolution to Fourier analysis.

The present study analyzes a structure impacted by a vortex where the critical radius  $r_c$  of the impinging vortex is one to one-hundred times the structure's diameter  $D$ . However, rotor- and wing-tip vortices may be substantially smaller than the rotors and airframe components that they load. Subsequent study is needed to evaluate structural loading when the impinging vortex is smaller than that of the loaded structure. It is expected that the vortex impact time with respect to the vortex shedding cycle from the structure will increasingly influence structural loading amplitude as the vortex size increases, and interaction between impacting vortices and rotor- and wing-tip vortices may become increasingly significant. However, studies using smaller impinging vortices will introduce computational time issues because extensive grid refinement will be required to accurately transport vortices. It is likely that a parallel computer program will be required to conduct the necessary number of simulations required to study the variation in structural loading with vortex impact time.

Force coefficient time histories from computer-simulated, two-dimensional vortex impact with slender structures are utilized as forcing to assess the possible dynamic amplification of tornado-vortex wind loads. This methodology is invaluable because it develops a conceptual framework for the analysis procedure. However, the next step in the analysis is to utilize forcing produced by a three-dimensional computer model applied to a multi-degree of freedom structure.

The three dimensional simulation and multi degree of freedom structure can account for variation in tangential velocity along the height of the tornado vortex, thereby providing a more realistic representation of structural loading by a tornado-like vortex.

All simulations reported in the present study use a maximum vortex tangential velocity to translational velocity ratio  $V_{\theta, \max} / U_{\infty}$  of unity. This ratio is realistic for some aerospace applications, but the vortex velocity ratio varies widely, from 3.11 to 14.19, for documented tornados and hurricanes. The vortex velocity ratio greatly influences the forcing time history produced by the vortex. Therefore, the cylinder loading trend with vortex size needs to be re-evaluated to determine its influence on the phenomenon controlling the cylinder loading trend with increasing vortex size.

The final suggested study is to reexamine the trend in the maximum amplitude of  $Cl'(-)$  for  $r_c / D \leq 5$ . Figures 5.10b, 5.11b, and 5.11d show that the trend in the maximum amplitude of  $Cl'(-)$  exhibits a progressively-pronounced dip as the vortex tangential velocity profile progresses from sharp (RCVM) to flat (S-K). The reason for this dip has not been investigated, and the underlying phenomenon are not known.

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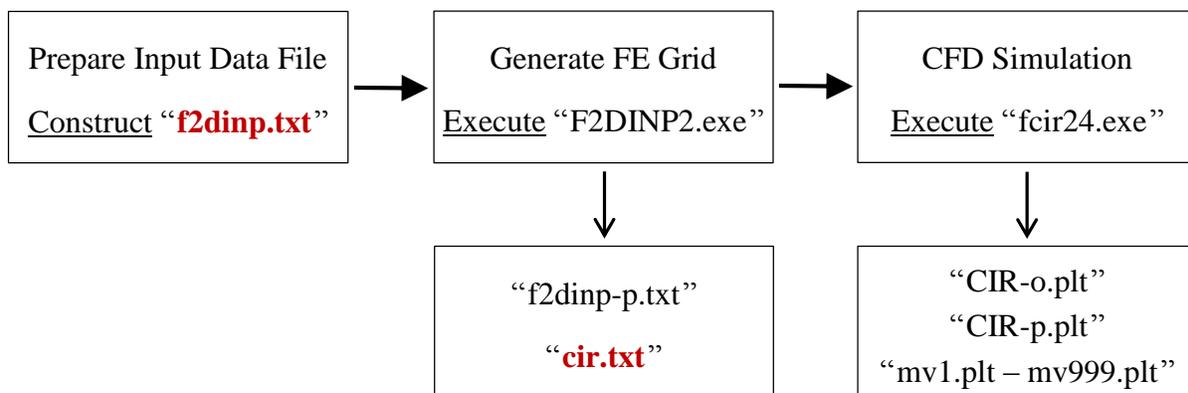
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## APPENDIX A: USE OF CFD CODE “FCIR24.EXE”

### A.1 Introduction and Overview

The present study utilizes the 2D, finite element code developed by Dr. R. Panneer Selvam to directly simulate impact of a vortex with a slender, cylindrical structure. The author modified the original program fcir23.exe by replacing the Rankine Combined vortex model with Vatisas’ vortex model. Vatisas’ model is selected because it replicates numerous vortex profiles as reviews in Chapter 3. This modified program is now referred to as fcir24.exe.

Figure A.1 is a flow chart that illustrates the three step process used to simulate vortex loading of a structure. The user first prepares f2dinp.txt which is the input data file for F2DINP2.exe and contains the simulation parameters and necessary information for constructing the grid. F2DINP2.exe reads in simulation and grid parameters from f2dinp.txt and assigns the grid points and elements for the finite element mesh. The output data file f2dinp-p.txt is formatted to plot the grid in Tecplot® if the user desires to view the grid prior to executing the CFD simulation. The other output data file cir.txt is then used as the input data file for the CFD simulation program fcir24.exe; cir.txt contains the simulation parameters as well as the grid information in the required finite element format.



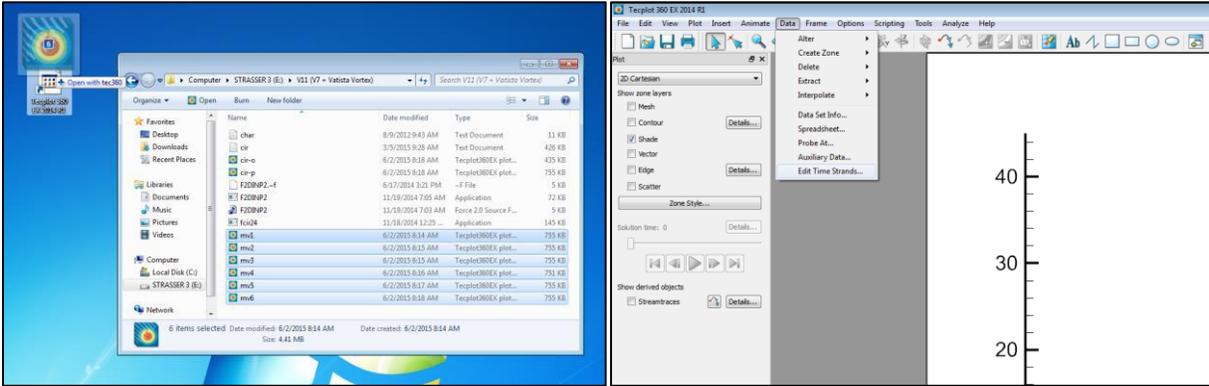
**Figure A.1:** Flow chart illustrating the simulation process using fcir24.

After running the CFD simulation a variable number of output data files are produced as indicated in the second box in the second row of Figure A.1. CIR-p.plt is used to plot contours of the values of the field variables (velocity, pressure, and vorticity) at the last time step of the CFD simulation using Tecplot®. Movie data files mv1.plt - mv999.plt contain the same information as CIR-p.plt but are plotted at the interval specified by the program's user. The text file char.txt contains the list of movie file names and must be kept in the same folder as fcir24.exe. Individual movie files can be combined in Tecplot® to create a running movie of the simulation. Procedure for creating a running movie in Tecplot® (360 EX 2014) is given below. Note that “+” implies a mouse click.

1. Select the movie files, and use drag-drop to upload them into Tecplot® (Figure A.2a).
2. Select the Data menu + Edit Time Strand (Figure A.2b) + Constant Delta + Apply (Figure A.2c).
3. Select Animate + Time... (Figure A.2d) + Destination → To File + Animate to File + Export Format → AVI + OK (Figure A.2e).

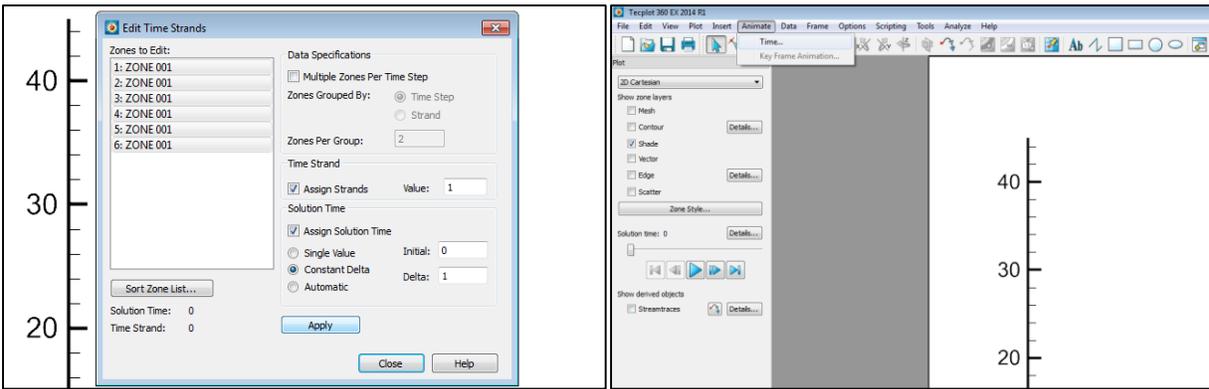
CIR-o.plt contains three tab-delineated columns of data which are dimensionless time, drag force coefficient, and lift force coefficient. Force coefficients are defined by integrating surface pressure and shear forces around the cylinder; drag and lift force coefficients are respectively defined by resolving the x- and y-direction forces and using the definition  $C = 2 \cdot F / A \cdot \rho \cdot U_{\text{ref}}^2$ . The program is written with the assumption that the reference area A, fluid density  $\rho$ , and reference velocity  $U_{\text{ref}}$  are all unity. Therefore, the actual output data in CIR-o.plt is dimensionless time, twice the horizontal force, and twice the vertical force. It is left to the user's discretion to adequately adjust the output data in CIR-o.plt to accommodate the traditional force

coefficient definitions. It is left to the user's discretion to adequately adjust the output data in CIR-o.plt to accommodate the traditional force coefficient definitions.



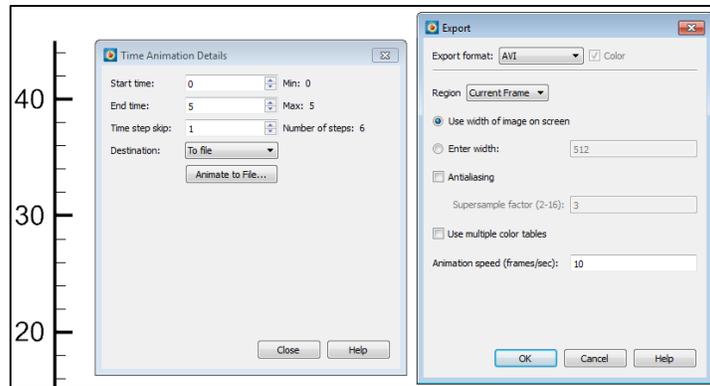
(a)

(b)



(c)

(d)



(e)

**Figure A.2:** Illustration of process for compressing individual movie (.mov) files into a running movie using Tecplot®.

It is prudent to always structure the grid around the grid so that the diameter of the cylinder is unity; consequently, the projected area per unit length is also unity and does not complicate computation of the force coefficient. As explained in Chapter 4, the fluid density is also assumed to be unity when introducing the dimensionless terms in the Navier Stokes equations. The present work utilizes different reference velocities depending upon the parameters of the system being simulated. It is left to the user to post-process CIR-o.plt if they wish to use a different reference velocity than the default value of unity.

## A.2 Preparation of Input Data File f2dinp.txt

F2DINP2.f reads data from f2dinp.txt using the command lines listed below. Descriptions of each of the variables are provided subsequently on a line-by-line basis. Note that Line 4 requires the user to input JM1 radial node points. These are defined along the shortest line from the cylinder surface to the edge of the square; it is convenient to define the first grid point, which is the cylinder radius, because this corresponds to a cylinder diameter of unity and simplifies force coefficient calculations. Subsequently, Line 5 requires that the user input IM2 horizontal grid point locations for the rectangular downstream region of the grid. The radial point provided in Line 4 and the first horizontal point provided in Line 5 must be the same.

```

READ(5,*)IM1,JM1,IM2,JM2,ANGLE
READ(5,*)TLAG,TTIME,DT,IFB,IFE,IFL2
READ(5,*)VISC,RAMAX,ROTC,VTRAN,PN,YSHIFT
READ(5,*)(R(I),I=1,JM1)
READ(5,*)(RX2(I),I=1,IM2)

```

Line 1: READ(5,\*)IM1,JM1,IM2,JM2,ANGLE

**IM1** number of tangential nodes (around the circumference) in Region I.

**JM1** number of radial nodes in Region I.

**IM2** number of nodes along the I-axis (horizontal axis) in region II.

**JM2** number of nodes along the J-axis (vertical axis) in region II (**JM2** = **IM1**/4 + 1).

**ANGLE** angle between nodes in Region I (**ANGLE** = 360/**IM1**).

Line 2: READ(5,\*)TLAG,TTIME,DT,IFB,IFE,IFL2

**TLAG** number of time units before vortex and cylinder centers coincide.  
**TTIME** total number of time units in simulation.  
**DT** time step length.  
**TW1** is the time at which the first movie file is written.  
**TW2** is the time at which the last movie file is written.  
**IFL2** number of time steps between movie files being written.  
 \*Note that a maximum of 999 movie files may be written.

Line 3: READ(5,\*)VISC,RAMAX,ROTC,VTRAN,YSHIFT

**VISC** kinematic viscosity of the fluid.  
**RAMAX** radius of forced vortex.  
**ROTC** rotational constant.  
**VTRAN** rate at which center of vortex translates (left to right).  
**PN** exponent for Vatistas vortex model  
**YSHIFT** is the vertical distance between horizontal axes of vortex and cylinder.  
 \*"-YSHIFT" moves the vortex up and "+YSHIFT" moves the vortex down.

Line 4: READ(5,\*)(R(I),I=1,JM1)

**R(I)** radial node points for Region I.  
 \*Maximum of 120 radial node points may be used.

Line 5: READ(5,\*)(RX2(I),I=1,IM2)

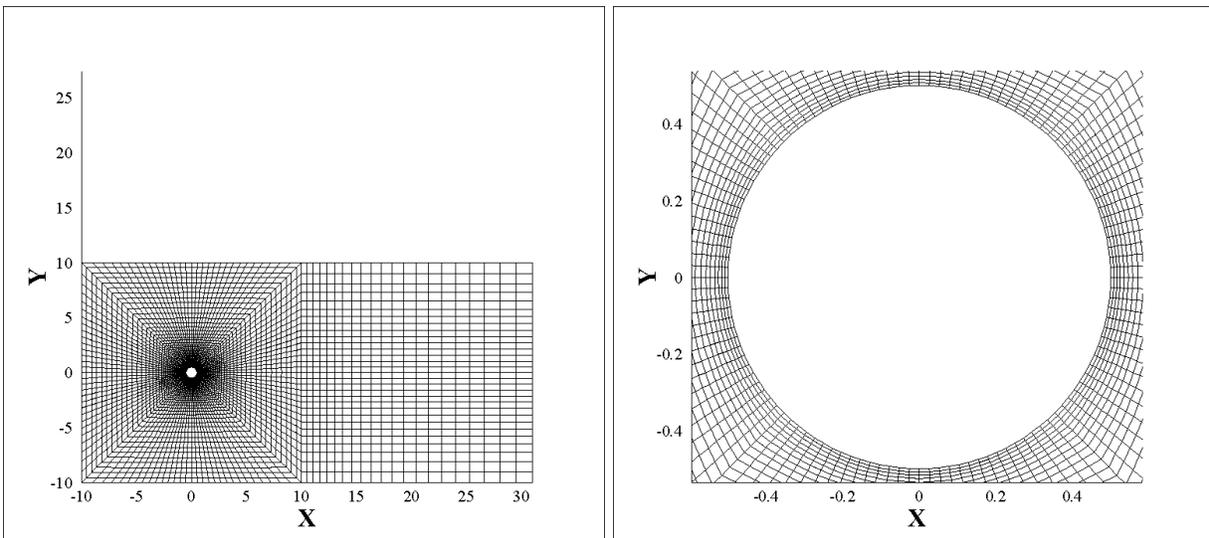
**RX2(I)** node points along I-axis (x-axis) of Region II.  
 \*Maximum of 60 horizontal node points may be used.  
 \*Note that **RX2(1) = R(JM1)** – The last radial node and first horizontal node coincide.

### A.2.1 Sample Input Data File

An example of the input data file f2dinp.txt is provided in three columns below. **Figure A.3** illustrates the grid produced, which is f2dinp-p.txt, when F2DINP2.f is executed. Note that **JM1** is marked in red text along with the corresponding radial grid points, and IM2 is marked in blue text along with the corresponding horizontal grid points in the downstream region of the rectangular region of the grid. Finally, note that the last radial node and first horizontal node, designated by bold text and yellow highlight, both have values of 10.

120 <b>55</b> <b>22</b> 31 3.	0.51575	0.571153782
250. 300. 0.01 1 300 10	0.524825	0.585769161
0.006667 5. 0.2 1. 0.	0.5348075	0.601846077
0.5	0.54578825	0.619530685
0.5075	0.557867075	0.638983753

0.660382128	2.34107524	10.5
0.683920341	2.53268276	11.05
0.709812375	2.74345104	11.65
0.738293613	2.97529614	12.3
0.769622974	3.23032576	13.
0.804085271	3.51085833	13.75
0.841993799	3.81944417	14.55
0.883693178	4.15888858	15.4
0.929562496	4.53227744	16.3
0.980018746	4.94300519	17.25
1.03552062	5.39480571	18.25
1.09657268	5.89178628	19.3
1.16372995	6.34825447	20.4000001
1.23760295	6.80472266	21.5500001
1.31886324	7.26119085	22.7500001
1.40824956	7.71765904	24.0000001
1.50657452	8.17412723	25.3000001
1.61473197	8.63059543	26.6500001
1.73370517	9.08706362	28.0500001
1.86457569	9.54353181	29.5000001
2.00853326	<b>10.</b>	31.00000
2.16688658	<b>10.</b>	



**Figure A.3:** Grid produced using the sample f2dinp.txt data file.

#### A.2.2 Use of GW.f to Produce f2dinp.txt

Manual construction of f2dinp.txt is time consuming as the user must define the positions of all radial nodes in Region I of the grid and the horizontal positions of all columns of nodes in

Region II of the grid; furthermore, manual construction and entry of the grid points is another potential source of error. Therefore, a program titled GW.f has been developed to construct f2dinp.txt from user-specified grid dimensions and spacing. The required input data file input.txt is structured as listed below. The only new terms that have not been previously explained are listed in Row 3, hence these are the only new variables that shall be explained. GC.f is currently configured to use constant horizontal node spacing in Region II. However, it can easily be modified to stretch horizontal node spacing, as is used in the Grid illustrated in Figure A.3, by employing a growth coefficient as is done for the radial node spacing.

```

READ(2,*)VISC,PN,VTRAN,RAMAX,ROTC,YSHIFT
READ(2,*)TLAG,TTIME,DT,IFB,IFE,IFL2
READ(2,*)RO,RF,DRO,DRM,GC,ANGLE,X

```

Row 3: READ(2,\*)RO,RF,DRO,DRM,GC,ANGLE,X

- RO** is the location of the first radial node (should be taken as 0.5 for simplicity).
- RF** is the location of the last radial node (side length of Region I).
- DRO** is the first radial node spacing.
- DRM** is the maximum radial node spacing.
- GC** is the growth coefficient used to stretch radial node spacing (1.10 is used herein).
- ANGLE** has the same definition as given above.
- X** is the length of Region II.

### A.2.3 Source Code for GW.f

```

C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%%*****----- GW.f -----*****%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%% Developed by: Matthew N. Strasser      %%%
C      %%% November 11, 2014                      %%%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%%*****----- Description -----*****%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%% This program is used to expedite construction of f2dinp      %%%
C      %%% which is the input data file for F2DINP2. The input file      %%%
C      %%% Contains all information for the CFD simulation as well as %%%
C      %%% the necessary information to construct the grid. This saves%%
C      %%% substantial time as opposed to having to manually generate %%%
C      %%% the radial node locations in Region I and the horizontal %%%
C      %%% node locations in Region II of the grid.      %%%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%%*****----- Disclaimer -----*****%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%% It is the responsibility of the user to understand the      %%%

```

```

C      %% interworking of this program prior to use.                                %%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
PARAMETER (NR=1000)
IMPLICIT REAL *8 (a-h,o-z)
DIMENSION R(NR),X2(NR)
OPEN(2,FILE='input.txt')
OPEN(3,FILE='f2dinp.txt')

READ(2,*)VISC,PN,VTRAN,RAMAX,ROTC,YSHIFT
READ(2,*)TLAG,TTIME,DT,IFB,IFE,IFL2
READ(2,*)RO,RF,DRO,DRM,GC,ANGLE,X

DO I=1,NR
  R(I)=0.
  X2(I)=0.
END DO

C.....C
C.....Generate Radial Nodes in RI.....C
C.....C
C      Radial nodes are defined as  $R_i = [(R_{i-1}) - (R_{i-2})] * GC$  until spacing
C      exceeds the maximum given in the input. Then, the remaining radial
C      distance to be spanned is computed, and nodes are equally spaced
C      to cover it. The actual maximum radial node spacing is slightly
C      less than that given.
C.....C
R(1)=RO
R(2)=R(1)+DRO
DO I=3,NR
  R(I)=R(I-1)+(R(I-1)-R(I-2))*GC
  IF(R(I).GT.RF)THEN
    R(I)=RAMAX
    JM1=I
    GOTO 1000
  END IF
  CHECK=(R(I-1)-R(I-2))*GC
  IF(CHECK.GT.DRM)THEN
    DRR=RF-R(I-1)
    NMAX=DRR/DRM
    DREQ=DRR/(NMAX+1)
    DO J=I,I+NMAX
      R(J)=R(J-1)+DREQ
    END DO
    JM1=J-1
    GOTO 1000
  END IF
END DO

C.....C
C.....Compute Horizontal Node Spacing in RII .....C
C.....C
C      Horizontal node spacing in Region II is taken as the average of
C      the vertical node spacing.
C.....C
1000 IM1=360./ANGLE      ! Nodes Around the Cylinder (Region I)
    JM2=IM1/4+1        ! Vertical Nodes (Region II)
    NYO=(JM2-1)/2      ! Number of Vertical Spaces
    DYAVG=RF/NYO       ! Average Vertical Spacing
    NDX=X/DYAVG

```

```
DX=X/(NDX+1)
X2(1)=R(JM1)
DO I=2,NDX+2
  X2(I)=X2(I-1)+DX
END DO
IM2=NDX+2
```

C.....C

```
WRITE(3,*)IM1,JM1,IM2,JM2,ANGLE
WRITE(3,*)TLAG,TTIME,DT,IFB,IFE,IFL2
WRITE(3,*)VISC,RAMAX,ROTC,VTRAN,PN,YSHIFT
DO I=1,JM1
  WRITE(3,*)R(I)
END DO
DO I=1,IM2
  WRITE(3,*)X2(I)
END DO
STOP
END
```

## **APPENDIX B: DATA EXTRACTION PROGRAM “GET\_DAT.M”**

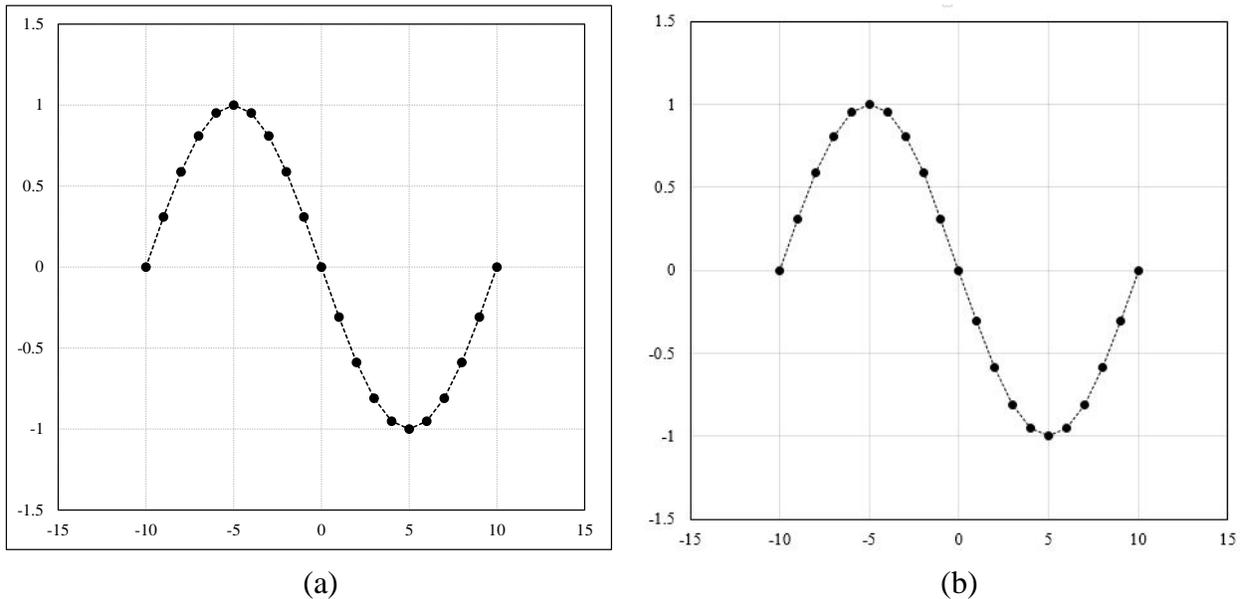
### **B.1 Overview and Description**

Technical documents typically report experimental findings graphically rather than in tabular form so significant trends can be easily identified. The primary problem with graphical data presentation is that the numerical values of the plotted data typically are not reported in the literature. Consequently, it is difficult to locate numerical data values for validation and comparison of data values attained in subsequent studies. The authors of the technical documents can, in some cases, be contacted with requests for the numerical values of the plotted data. However, this is a tedious, time-consuming process, and original data are frequently lost in the years following publication of the technical documents. In short, methodology is needed to read discrete data points from images of plot files.

“Get\_Dat.m” is a program developed using the Matlab<sup>®</sup> programming environment. The program allows the user to extract discrete data points from a .jpg image file of a plot having linear axis. The user is prompted to click-select the plot’s origin, right x-axis bound, and top y-axis bound as well as to specify the x- and y-values at each location. The user then is prompted to click-select any number of discrete data points which are then converted to the user-assigned coordinate system and output as a .txt file. The accuracy with which the user-defined data points represent the original data points in the .jpg image of the plot depends upon the user’s accuracy in click-selecting the values. However, discrete points are selected with sufficient accuracy for applications such as the present study. It is the user’s responsibility to establish if the extracted points have sufficient resolution for their particular study. The subsequent subsection illustrates the use of “Get\_Dat.m” and effectively serves as a user’s manual.

## B.2 Demonstration of Use

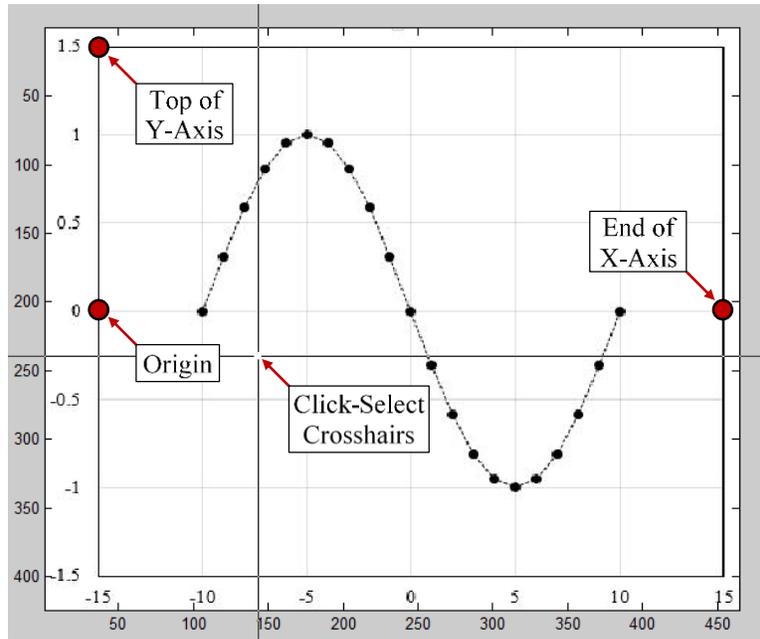
After compiling “Get\_Dat.m” in a folder, the user should then save the selected plot image as a “Capture.jpg” in the same folder (The author suggests using the “Snip” tool to select the desired region of the plot for data extraction). Figures B.1a and B.1b illustrate the original data plot from which points are to be extracted and the .jpg file produced using the snip tool respectively.



**Figure B.1:** (a) Original data plot and (b) .jpg image of the original data plot.

When “Get\_Dat.m” is executed, the program reads in “Capture.jpg” and assigns axis in the top-left corner of the image (This is a default setting because the command “imread(image name)” command is used). The user is prompted to (1) click-select and (2) define values at the locations of the origin, top of the y-axis, and end of the x-axis, the locations of which are illustrated in Figure B.2. The click-selected axis locations are important because they establish the origin locations with respect to the matlab-defined axis system (which is defined with respect to the top-left corner of the image). The user-defined axis values are important because they,

along with the click-selected axis values, provide a means to transform the data points from the matlab-defined axis to the user-defined axis.

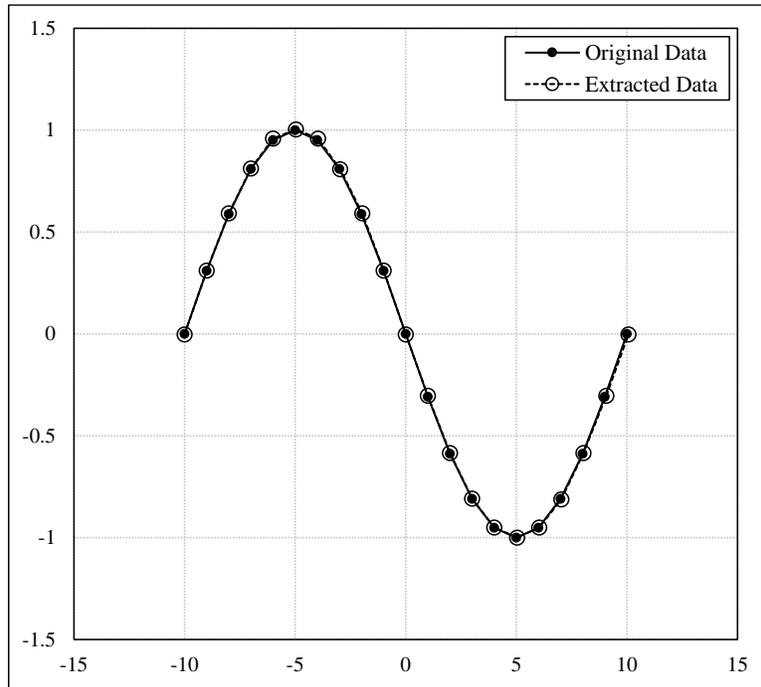


**Figure B.2:** Illustration of “Capture.jpg” in the Matlab® environment and the three axis that the user must click-select and assign values of.

After establishing and defining the locations of the origin, top of y-axis, and end of x-axis, the user is then prompted to select any number of discrete data points from the curve. Discrete points are click-selected and recorded until the user terminates the data-selection phase by striking the “Enter” key. The extracted data points are output in a tab-delineated .txt file titled “DataPoints.txt” and displayed via a pop-up Matlab® plot.

Figure B.3 compares the extracted data points with the original data points. The extracted data points are excellent representation of the the original data points. There are small variations between the original and extracted values in some cases, as there is user-error in manually click-selecting the locations of the data points within the .jpg image. However, for applications such as extracting measurements of vortex tangential velocity profiles or curves of

force coefficient and Strouhal number trends for qualitative comparison (Which are the specific uses in the present study), “Get\_Dat.m” provides sufficient accuracy.



**Figure B.3:** Comparison of original data points with extracted data points.

### B.3 Source Code

“Get\_Dat.m” has been an invaluable resource that has allowed the author to rapidly compile large quantities of data from physical and computer simulations reported in the literature that would otherwise have been unattainable or at least would have required extensive time to compile. This program can be utilized for the same purpose in other studies, hence the author wishes to disseminate it for use by other authors as they pursue their studies. However, the author requests that subsequent users give credit where due and retain the header file (Green text).

```

%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%*****----- Get_Dat.m -----*****%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%% Developed by: Matthew N. Strasser & Cyrus Garner          %%
%% June 30, 2015                                           %%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%*****----- Description -----*****%%
%% This program allows the reading of discrete data points from a .jpg %%
%% image file of a data plot having LINEAR axis. The user click-assigns %%
%% the locations of the axis and origin and provides values for each. %%
%% The user then click-selects points on the plot image, which are %%
%% then converted to discrete points based upon the user-defined axis. %%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%*****----- Disclaimer -----*****%%
%% To the best of the author's knowledge, this program is usable for %%
%% any plot image having linear axis. The user should be cautious %%
%% and verify that GET_DAT.m provides sufficient accuracy for their %%
%% application. The outputted discrete values are approximate, with %%
%% their resolution being determined by the accuracy of the user's %%
%% click-selection of data points.                          %%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
close all;
clear all;
im_mat = imread('Capture.JPG'); % Read input .jpg file
fName = 'DataPoints.txt'; % Write output .txt file
imagesc(im_mat) % Scale and assign .jpg file
% Define plot area from which data will be extracted and assign axis and
% dimensions for extracted data.
check_flag = 0;
while check_flag == 0
    disp('Select Origin')
    Oc = ginput(1);
    Ox = input('Input Origin "X-Value"');
    Oy = input('Input Origin "Y-Value"');
    check_flag = input('Is Origin Correct? (1 yes, 0 no)');
end
check_flag = 0;
while check_flag == 0
    disp('Select Top End of Y-Axis')
    Yc = ginput(1);
    Yy = input('Input "Y-Value"');
    check_flag = input('Is End Y-Axis Value Correct? (1 yes, 0 no)');
end
check_flag = 0;
while check_flag == 0
    disp('Select End of X-Axis')
    Xc = ginput(1);
    Xx = input('Input "End of X-Axis Value"');
    check_flag = input('Is End X-Axis Value Correct? (1 yes, 0 no)');
end
% Length Scales for Matlab-Assigned Axis
XLc = abs(abs(Oc(1,1))-abs(Xc(1,1)));
YLc = abs(abs(Oc(1,2))-abs(Yc(1,2)));
% Length Scales for User-Defined Axis
XLu = abs(Xx-Ox);

```

```

YLu = abs(Yy-Oy);
% Scalars for Conversion from Matlab-to-User Axis
XS = XLu/XLc;
YS = YLu/YLc;
% Read Click-Selected Data Points
disp('HIT RETURN TO EXIT');
[Px,Py] = ginput;
% Flip Y-Axis (Matlab Assigns at Top-Left Image Corner)
Pyf = - Py + Oc(1,2);
% Convert Matlab-Axis Data Points to User-Defined Axis
for i=1:length(Px)
    DXc = Px(i)-Oc(1,1);
    DYc = Pyf(i);
    TEMP(i,1) = Ox + DXc*XS;
    TEMP(i,2) = Oy + DYc*YS;
end

dlmwrite(fName,TEMP,'\t');

figure(1)
plot(TEMP(:,1),TEMP(:,2),'k--o','linewidth',2)
title('Extracted Points','fontsize',20,'fontweight',...
      'b','fontname','Times New Roman')
grid on

```

## APPENDIX C: DLF ANALYSIS PROGRAM “DLF.M”

### C.1 Overview and Description

One aspect of this body of work is the assessment of the capability of lateral vortex loading to be dynamically amplified. Chapter 6 explains extensively how DLF analysis is an effective way to determine whether or not the manner in which a transient load is applied will produce dynamic amplification of the loading. The present program “DLF.m” is developed in the Matlab® environment. Both dimensional (Section C.3) and dimensionless (Section C.4) versions have been developed following the discussion in Section 6.3.1. The model’s response is resolved at each time step via fourth order Runge Kutta. For each structure frequency, the maximum DLF value is defined; DLF(fn) values are then compiled, thereby defining the DLF curve for loading by both force coefficients.

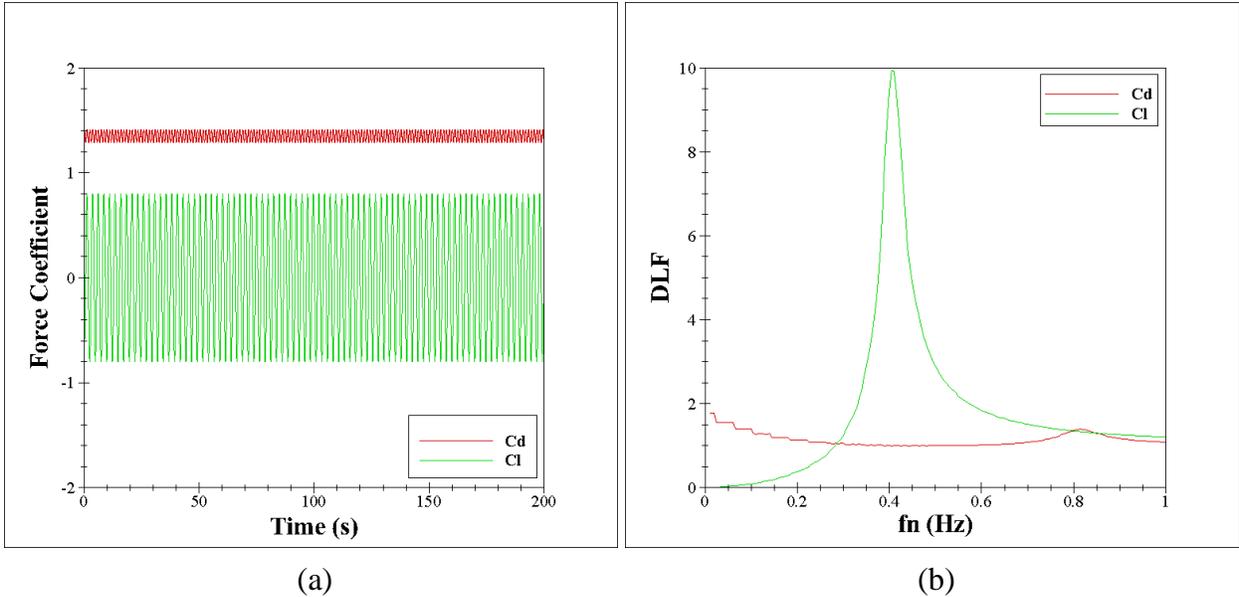
The user defines the SDOF system’s “mass” and “damping ratio” inside the program, as well as the range and increment of the “fundamental structure frequency” inside the program. The nondimensional model requires extra parameters such as fluid density, reference area, etc. The program then computes the structure “stiffness” for each fundamental frequency prior to analyzing the SDOF system’s response to the applied forcing. The program outputs the DLF(fn) curves for application of the drag and lift force coefficients in the .txt file “DLF\_out.txt”. The author validates “DLF.m” by comparison of a computed DLF curve from a sine wave forcing profile with the analytical definition for the same forcing profile. However, it is the responsibility of the user to verify the accuracy and usability of “DLF.m” in their individual application. The subsequent subsection illustrates the use of “DLF.m” and effectively serves as a user’s manual.

“DLF.m” is a valuable program that may aid other authors in their studies and serve as a starting point for response models developed in their individual studies. The author wishes to

dissiminate it for use by other authors but requests that subsequent users give credit where due and retain the header if the program is not significantly modified (Green text).

### C.2 Demonstration of Use

After typing and saving “DLF.m” in a folder, the user should also save the x- and y-direction forcing time histories as a .plt file titled “FC.plt” in the same folder. The forcing considered for this example is dimensionless drag and lift force coefficient time histories which are illustrated in Figure 6.1, hence the dimensionless version of the program is used. The fundamental structure frequency range is declared in Line 31 as “fn = (0.01:0.005:1.0)”. This reads that the lowest frequency is  $0.01 \text{ s}^{-1}$ , the frequency increment is  $0.005 \text{ s}^{-1}$ , and the highest frequency is  $1.0 \text{ s}^{-1}$ . The structure’s mass “m” and damping ratio “z” are respectively defined in Line 32 and 39. The reference velocity “Ur”, reference dimension “D”, and reference length “L” which are required to introduce the dimensionless force coefficient are respectively defined in Lines 34-36. Upon execution of DLF.m, the DLF curve produced for both force coefficients is shown in Figure C.1.



**Figure C.1:** (a) Force coefficient time histories and (b) corresponding DLF curves.

### C.3 Source Code (Dimensional)

```

%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%*****----- DLF.m -----*****%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%% Developed by: Matthew N. Strasser          %
%%% March 10, 2015                            %
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%*****----- Description -----*****%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%% This program analyzes the response of a single degree of freedom %
%%% (SDOF) structure to decoupled drag and lift forces and defines the %
%%% dynamic load factor (DLF) curve for the applied forcing as function %
%%% of the fundamental structure frequency (fn). %
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%*****----- Disclaimer -----*****%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%% It is the responsibility of the user to verify the applicability of %
%%% this program for their study. %
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
clear all;
close all;
load FC.plt;          % Forcing Time Histories.
fName='DLF_out.txt'; % Output data file.
TIMEi = FC(:,1);     % Time Vector.
Fdi = FC(:,2);       % Drag Force Vector.
Fli = FC(:,3);       % Lift Force Vector.
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%% Response Spectrum %
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
fn = (0.01:0.005:1.0); % Structure frequency (Hz)
m = 200000;            % Structure mass (kg)
z = 0.05;              % Damping ratio (dim)
tic;
for j=1:length(fn)
    wn = 2*pi*fn(j); % Structure frequency (rad/s)
    k = wn^2*m;      % Structure stiffness (N/m)
    Kt(j) = k;
    A = [0,1;-wn^2,-2*z*wn]; % State matrix
    B = @(tt, qq, FF) (A*qq + [0; FF/m]); % System of Eqs
    q0 = [0; 0]; % Initial conditions
% Initialize dummy solution vectors.
    S1 = zeros(length(q0), length(TIMEi));
    S1(:,1) = q0;
    S2 = zeros(length(q0), length(TIMEi));
    S2(:,1) = q0;
% Analyze Fl
    for i = 1:length(TIMEi)-1
        if(i==1)
            dt = TIMEi(2)-TIMEi(1);
        else
            dt = TIMEi(i)-TIMEi(i-1);
        end
        k1 = B(TIMEi(i), S1(:,i), Fli(i));
        qmid = S1(:,i) + dt/2*k1;
        k2 = B(TIMEi(i) + 1/2*dt, qmid, (Fli(i) + Fli(i+1))/2);

```

```

        qmid = S1(:,i) + dt/2*k2;
        k3 = B(TIMEi(i) + 1/2*dt, qmid, (Fli(i) + Fli(i+1))/2);
        qend = S1(:,i) + dt*k3;
        k4 = B(TIMEi(i) + dt, qend, Fli(i));
        S1(:,i+1) = S1(:,i) + dt/6*(k1 + 2*k2 + 2*k3 + k4);
    end
% Analyze Fd
    for i = 1:length(TIMEi)-1
        if(i==1)
            dt = TIMEi(2)-TIMEi(1);
        else
            dt = TIMEi(i)-TIMEi(i-1);
        end
        k1 = B(TIMEi(i), S2(:,i), Fdi(i));
        qmid = S2(:,i) + dt/2*k1;
        k2 = B(TIMEi(i) + 1/2*dt, qmid, (Fdi(i) + Fdi(i+1))/2);
        qmid = S2(:,i) + dt/2*k2;
        k3 = B(TIMEi(i) + 1/2*dt, qmid, (Fdi(i) + Fdi(i+1))/2);
        qend = S2(:,i) + dt*k3;
        k4 = B(TIMEi(i) + dt, qend, Fdi(i));
        S2(:,i+1) = S2(:,i) + dt/6*(k1 + 2*k2 + 2*k3 + k4);
    end
% Take Steady Response (Avoid unsteadiness in initial RK4 solution)
    XF1 = S1(1,round(length(TIMEi)/8):length(TIMEi));
    XFd = S2(1,round(length(TIMEi)/8):length(TIMEi));
% Compute Maximum Response Amplitude
    X1(1) = max(XF1);
    X1(2) = abs(min(XF1));
    X2(1) = max(XFd);
    X2(2) = abs(min(XFd));
    AF1(j) = max(X1);
    AFd(j) = max(X2);
    clear XF1 XFd RMS X1 X2;
% Re-Set dummy solution vector
    S1 = zeros(length(q0), length(TIMEi));
    S1(:,1) = q0;
    S2 = zeros(length(q0), length(TIMEi));
    S2(:,1) = q0;
toc;
J=j
W=length(fn)
end
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%% Convert maximum response to DLF %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
% Compute maximum static displacements.
Fdt(1) = abs(max(Fdi));
Fdt(2) = abs(min(Fdi));
Fdm = max(Fdt); % Absolute maximum drag force
Flt(1) = abs(max(Fli));
Flt(2) = abs(min(Fli));
Flm = max(Flt); % Absolute maximum lift force
for j=1:length(fn)
    DLFFd(j) = AFd(j)/(Fdm/(Kt(j)));
    DLFFl(j) = AF1(j)/(Flm/(Kt(j)));
end
% Write response data.

```

```

for i=1:length(fn)
    TEMP(i,1)=fn(i);           % Structure frequencies (Hz)
    TEMP(i,2)=DLFFd(i);       % Drag Force DLF
    TEMP(i,3)=DLFFl(i);       % Lift Force DLF
end
dlmwrite(fName,TEMP,'\t');

y1 = 1.10*max(max(DLFFd),max(DLFFl));
figure (1)
plot(fn,DLFFl,':og',fn,DLFFd,':or','linewidth',2)
xlabel('Structure Frequency (Hz)','fontsize',14,'fontweight','b',...
    'fontname','Times New Roman')
ylabel('DLF','fontsize',14,'fontweight','b','fontname',...
    'Times New Roman')
set(gca,'xlim',[0 max(fn)],'ylim',[0 y1],'fontsize',12,'fontname',...
    'Times New Roman')
legend('Fl','Fd',2)
grid on

```

## C.4 Source Code (Dimensionless)

```

%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%*****----- DLF.m -----*****%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%% Developed by: Matthew N. Strasser                                %%
%% October 15, 2015                                              %%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%*****----- Description -----*****%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%% This program analyzes the response of a single degree of freedom %%
%% (SDOF) structure to decoupled drag and lift force coefficients and %%
%% defines the dynamic load factor (DLF) curve for the forcing as a %%
%% function of the fundamental structure frequency (fn).          %%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%*****----- Disclaimer -----*****%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%% It is the responsibility of the user to verify the applicability of %%
%% this program for their study.                                  %%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
clear all;
load FC.plt;                % Force Coefficient Time Histories.
fName1 = 'RSP_out.txt';    % Response Spectrum.
fName2 = 'DLF_out.txt';    % DLF Spectrum.
tFC = FC(:,1);             % Time Vector.
CdFC = FC(:,2);            % Drag Coefficient Vector.
ClFC = FC(:,3);            % Lift Coefficient Vector.
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%% Interpolate to Constant-Time-Step %%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
TIMEi = linspace(min(tFC),max(tFC),length(tFC));                % Time Vector.
dt = TIMEi(2)-TIMEi(1);                                         % Constant Time Step.
Cli = interp1(tFC,ClFC,TIMEi);                                   % Interpolated Cl.
Cdi = interp1(tFC,CdFC,TIMEi);                                   % Interpolated Cd.
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%% Response Spectrum %%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
fn = (0.001:0.001:0.5);                                         % Structure frequency (Hz)
m = 200000;                                                       % Structure mass (kg)
RHO = 1.0;                                                         % Fluid density (kg/m3)
Ur = 1.0;                                                          % Reference velocity (m/s)
D = 1.0;                                                           % Structure diameter (m)
L = 1.0;                                                           % Structure unit length (m)
Ar = D*L;                                                         % Reference area (m2)
Rm = RHO*Ar*D/m;                                                  % Nondimensionalize mass (dim)
z = 0.05;                                                         % Damping ratio (dim)
tic;
for j=1:length(fn)
    wn = 2*pi*fn(j);                                             % Structure frequency (rad/s)
    Us = Ur/(wn*D);                                              % Nondim. reference vel. (dim)
    % Stiffness associated with structured wn(j) is still needed to compute
    % maximum static deflection.
    k = wn^2*m;                                                  % Structure stiffness (N/m)
    Kt(j) = k;
    A = [0, 1; -(1/Us)^2, -2*z/Us];                               % State matrix
    B = @(tt, qq, FF)(A*qq + [0; FF*Rm/2]);                       % System of Eqs
end

```

```

    q0 = [0; 0]; % Initial conditions
% Initialize dummy solution vectors.
    S1 = zeros(length(q0), length(TIMEi));
    S1(:,1) = q0;
    S2 = zeros(length(q0), length(TIMEi));
    S2(:,1) = q0;
% Analyze Cl
    for i = 1:length(TIMEi)-1
        k1 = B(TIMEi(i), S1(:,i), Cli(i));
        qmid = S1(:,i) + dt/2*k1;
        k2 = B(TIMEi(i) + 1/2*dt, qmid, (Cli(i) + Cli(i+1))/2);
        qmid = S1(:,i) + dt/2*k2;
        k3 = B(TIMEi(i) + 1/2*dt, qmid, (Cli(i) + Cli(i+1))/2);
        qend = S1(:,i) + dt*k3;
        k4 = B(TIMEi(i) + dt, qend, Cli(i));
        S1(:,i+1) = S1(:,i) + dt/6*(k1 + 2*k2 + 2*k3 + k4);
    end
% Analyze Cd
    for i = 1:length(TIMEi)-1
        k1 = B(TIMEi(i), S2(:,i), Cdi(i));
        qmid = S2(:,i) + dt/2*k1;
        k2 = B(TIMEi(i) + 1/2*dt, qmid, (Cdi(i) + Cdi(i+1))/2);
        qmid = S2(:,i) + dt/2*k2;
        k3 = B(TIMEi(i) + 1/2*dt, qmid, (Cdi(i) + Cdi(i+1))/2);
        qend = S2(:,i) + dt*k3;
        k4 = B(TIMEi(i) + dt, qend, Cdi(i));
        S2(:,i+1) = S2(:,i) + dt/6*(k1 + 2*k2 + 2*k3 + k4);
    end
% Take Steady Response (Avoid unsteadiness in initial RK4 solution)
    XC1 = S1(1,round(length(TIMEi)/8):length(TIMEi));
    XCd = S2(1,round(length(TIMEi)/8):length(TIMEi));
% Compute Maximum Response Amplitude
    X1(1) = max(XC1);
    X1(2) = abs(min(XC1));
    X2(1) = max(XCd);
    X2(2) = abs(min(XCd));
    AC1(j) = max(X1);
    ACd(j) = max(X2);
    clear XC1 XCd RMS X1 X2;
% Re-Set dummy solution vector
    S1 = zeros(length(q0), length(TIMEi));
    S1(:,1) = q0;
    S2 = zeros(length(q0), length(TIMEi));
    S2(:,1) = q0;
toc;
J=j
W=length(fn)
end
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%% Convert maximum response to DLF %%%%%%%%%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
% Compute maximum static displacements.
Cdt(1) = abs(max(Cdi));
Cdt(2) = abs(min(Cdi));
Cdm = max(Cdt);
Cs = RHO*Ar*Ur^2/2; % Absolute maximum drag coefficient value
Fdm = Cdm*Cs;

```

```

Clt(1) = abs(max(Cli));
Clt(2) = abs(min(Cli));
Cln = max(Clt); % Absolute maximum lift coefficient value
Flm = Cln*Cs;
for j=1:length(fn)
    % Convert dimensionless amplitudes (ACd & AC1) to dimensioned
    % amplitudes so they can be directly compared with static
    % displacements.
    DLFCd(j) = ACd(j)*D/(Fdm/(Kt(j)));
    DLFC1(j) = AC1(j)*D/(Flm/(Kt(j)));
end
% Write response data.
for i=1:length(fn)
    TEMP1(i,1)=fn(i);
    TEMP1(i,2)=ACd(i);
    TEMP1(i,3)=AC1(i);
    TEMP2(i,1)=fn(i); % Corresponding structure frequencies (Hz)
    TEMP2(i,2)=DLFCd(i); % Drag coefficient DLF
    TEMP2(i,3)=DLFC1(i); % Lift coefficient DLF
end
dlmwrite(fName1,TEMP1,'\t');
dlmwrite(fName2,TEMP2,'\t');

yl = 10;
figure (1)
plot(fn,DLFCd,':og',fn,DLFC1,':or','linewidth',2)
xlabel('Structure Frequency (Hz)','fontsize',14,'fontweight','b',...
'fontname','Times New Roman')
ylabel('Xmax/Xstatic','fontsize',14,'fontweight','b','fontname',...
'Times New Roman')
title('DLF','fontsize',20,'fontweight','b','fontname','Times New Roman')
set(gca,'xlim',[0 max(fn)],'ylim',[0 yl],'fontsize',12,'fontname',...
'Times New Roman')
legend('Signal 1','Signal 2',2)

```

## APPENDIX D: EMPIRICAL CYLINDER LOADING BY VORTEX “EFC.F”

### D.1 Overview and Description

“EFC.f” is a program developed in the Fortran programming language to duplicate the loading of a rigid, circular cylinder by an impinging vortex at low Reynolds numbers ( $Re \leq 300$ ). The program uses the equations and logic defined in Chapter 4 (for the transient velocity boundary condition) to compute the time history of the resultant velocity ( $V_R$ ) at the prescribed location of the cylinder center. Additional studies reported in Chapter 6 simulate loading of a rigid, circular cylinder immersed in a free stream at  $100 \leq Re \leq 300$  (while holding cylinder diameter “D” and fluid viscosity “ $\nu$ ” constant) allowing the mean ( $C_M$ ) and harmonic ( $C_H$ ) components of the cylinder loading to be defined as functions of  $V_R$  (See lines 60 and 61).

EFC.f reads the tornado parameters from the input data file “input.txt”, which must be located in the same folder. The outputs are the mean drag and lift force coefficient time histories “FC(Mean).plt” and the envelopes that bound the amplitude of both force coefficient time histories “FC(Envelopes).plt”. All force coefficients are computed using the reference velocity of  $U_{ref} = 2$ . Underlying assumptions of the application of EFC.f are that (1)  $V_R$  incident on the cylinder is  $V_R$  that would be at the location of the cylinder center if the cylinder were not present to disrupt the flow and (2) the vortex is sufficiently large so that  $V_R$  is incident across the entire cylinder.

The user should be cautious when using EFC.f and understand what the limitations of the program are. Validation studies in Chapter 6 show that (1) EFC.f accurately reproduces force coefficient time histories from cylinder loading by a large vortex ( $r_c/D = 50$ ) and (2) mean force coefficient time histories defined by EFC.f dynamically excite the same fundamental structure frequencies as force coefficient time histories from simulated structure loading by small vortices

( $2 \leq r_c/D \leq 10$ ). That being said, EFC.f is not able to replicate the aerodynamic phenomenon (vortex shedding, interaction between attached and impinging vortices, etc.) which may also influence the cylinder loading. This means that the same force coefficient time history will always be produced for a given set of vortex parameters, and the variation in cylinder due to interaction between attached and impinging vortices (See Chapter 5) will not be captured. EFC.f provides a methodology to assess the character of low-Re vortex loading of a rigid circular cylinder; mean- and force coefficient envelopes are defined in a matter of minutes for simulations that require several days to execute via 2D direct simulation. This program can be used to assess how vortex parameters influence the cylinder loading, and then 2D CFD modelling can be used to simulate the loading cases deemed critical. The user must remain cautious, however, in that the empirical equations currently used to define the mean and harmonic components of the cylinder loading are only valid for  $Re \leq 300$ . The subsequent subsection illustrates the use of EFC.f and effectively serves as a user's manual.

## D.2 Demonstration of Use

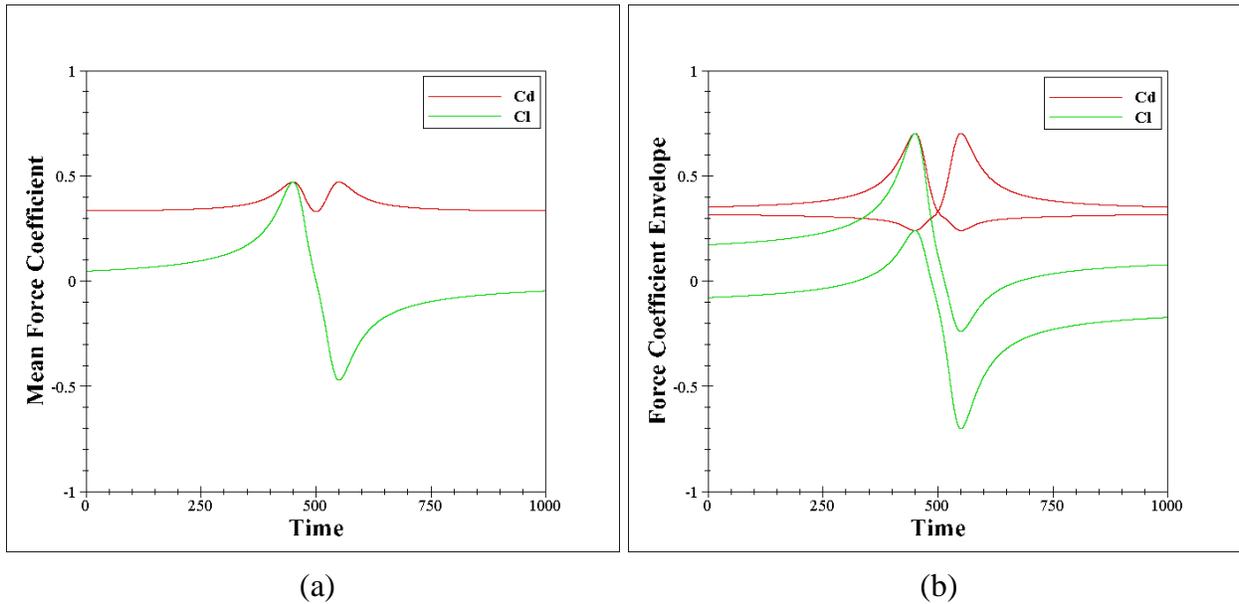
EFC.f must be typed, saved, and compiled in a folder. Subsequently, the input data file input.txt must be constructed and saved in the same folder. The input data consists of ten parameters listed in three rows. The sample data file is listed below, followed by the variable names, and finally by descriptions of each of the variables. The same equations used to define the transient velocity boundary condition are used to define the resultant velocity at the location of the cylinder center (See *Figure 4.4*), consequently,  $X_p$  and  $Y_p$  should remain fixed at zero.

```
1., 50., 0.02, 2
0, 0, 0
500, 1000, 0.01
```

```
UFS, Rc, Alpha, Sn
Del, Xp, Yp
Tlag, TTIME, DT
```

UFS - Free stream velocity  
 Rc - Vortex critical radius  
 Alpha - Vortex rotational constant  
 Sn - Vatistas vortex coefficient  
     (1) -> Normalized Sculley  
     (2) -> Normalized Lamb-Oseen/Burgers-Rott  
     (100) -> Rankine Combined  
  
 Del - Vertical shift in vortex path  
 Xp - Horizontal location of point W/R cylinder center  
 Yp - Vertical ...  
  
 Tlag - Time when vortex/cylinder centers align  
 TTIME - Total considered interaction time  
 DT - Time Step

The output data files containing the mean force coefficient time histories and the envelopes bounding the force coefficient time histories are plotted in Figures C.1a and C.1b respectively. Chapter 6 illustrates the combination of these two data sets and shows them to be excellent replication of the force coefficient time history produced by 2D computer simulation.



**Figure D.1:** (a) Mean force coefficient time histories from “FC(Mean).plt” and (b) force coefficient envelop time histories from “FC(Envelopes).plt”.

### D.3 Source Code

“EFC.m” is a valuable program that may aid others in their studies of low-Re vortex loading of structures, hence the author would like to disseminate the code. However, the author implores any that use the program to make sure that they first understand the applicability of the program. Also, any duplication of the program without significant alteration should retain the header (Grey text).

```
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%%%%%%%%*****----- EFC.f -----*****%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%% Developed by: Matthew N. Strasser          %%%
C      %%% May 6, 2015                               %%%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%%%%%%%%*****----- Description -----*****%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%% This program approximates the drag (Cd) and lift (Cl) force%%
C      %%% coefficient time histories on a cylindrical cross section  %%%
C      %%% located at specified point (Xp,Yp) defined relative to the %%%
C      %%% cylinder center location (See Dissertation Figure 4.4).    %%%
C      %%%                                                    %%%
C      %%% The emperical force coefficient equations are assigned for %%%
C      %%% using free stream simulations (D = 1) and (v = 1/150) for  %%%
C      %%% 100 < Re < 300, therefore, the maximum velocity that may  %%%
C      %%% be used in the domain (translational + tangential) is 2.0. %%%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%%%%%%%%*****----- Disclaimer -----*****%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
C      %%% The force coefficient time histories defined by this      %%%
C      %%% program reproduce the time histories produced by computer %%%
C      %%% simulated vortex loading of a cylindrical structure in the %%%
C      %%% range 100 < Re < 300. The user is responsible to evaluate  %%%
C      %%% the usability of "EFC.f" for their specific application.  %%%
C      %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION FC(3,1000000)
OPEN(2,FILE='input.txt')
OPEN(3,FILE='FC(Mean).plt')
OPEN(5,FILE='FC(Envelopes).plt')
READ(2,*)UFS,Rc,Alpha,Sn
READ(2,*)Del,Xp,Yp
READ(2,*)Tlag,TTIME,DT
PI=3.14159
NTS=TTIME/DT
TO = -Tlag
TSTAR = TO
TIME = 0.
DO I=1,NTS+1
    Xpp = Xp - UFS*TSTAR
    Ypp = Yp - Del
```

```

Rpp = (Xpp**2.+Ypp**2.)**0.5
Vtheta = Alpha*Rpp*(2./((Rpp/Rc)**(2.*Sn)+1))**(1./Sn)
! The denominator of Vtheta becomes too large for the RCVM
! approximation when Rpp << 0 or Rpp >> 0, so the RCVM vortex
! definition is used instead.
  IF(Sn.EQ.100) THEN
  IF(Rpp.LE.Rc) THEN
    Vtheta = Alpha*Rpp
  Else
    Vtheta = Alpha*Rc**2./Rpp
  END IF
END IF
U = UFS - Vtheta * (Ypp/Rpp)
V = Vtheta * (Xpp/Rpp)
Vres = (U**2.+V**2.)**0.5
CH = 0.3111*Vres**2.-0.2594*Vres+0.0712
CM = 0.3569*Vres**2.-0.0525*Vres+0.0261
Ang = atan(V/U)
CdH = CH*sin(Ang)
CdM = CM*cos(Ang)
ClH = CH*cos(Ang)
ClM = CM*sin(Ang)
Cdplus = CdM + CdH
Cdminus = CdM - CdH
Clplus = ClM + ClH
Clminus = ClM - ClH
WRITE(3,*)TIME,CdM,ClM
WRITE(5,*)TIME,Cdplus,Cdminus,Clplus,Clminus
PRINT*,TIME
TSTAR = TSTAR + DT
TIME = TIME + DT
END DO
STOP
END

```

## CURRICULUM VITAE

### EDUCATION

**Doctor of Philosophy, Civil Engineering (Structural)** December 2015  
University of Arkansas  
GPA: 3.88/4.00  
Dissertation: The Aerodynamic and Dynamic Loading of a Slender Structure by an  
                  Impacting Tornado-Like Vortex: The Influence of Relative Vortex-to-  
                  Structure Size on Structural Loading  
Committee: R. Panneer Selvam, Micah Hale, Ernest Heymsfield, and Rick Couvillion

**Master of Science, Civil Engineering (Structural)** December 2012  
University of Arkansas  
GPA: 3.88/4.00  
Thesis: Performance and Cost Analysis of a Structured Concrete Thermocline Thermal  
                  Energy Storage System  
Committee: R. Panneer Selvam, Micah Hale, and Ernest Heymsfield

**Bachelor of Science, Mechanical Engineering** May 2011  
Harding University  
GPA: 3.20/4.00

### LICENSURE AND CERTIFICATION

Concrete Field Technician Grade I (ACI-01242091) April 2012 - 2017  
Engineer in Training (Arkansas EIT-7797) June 2011

### QUALIFICATIONS

- A multi-disciplinary engineer who pursues leadership roles.
- Delivers work on time in high-pressure settings in individual and team environments.
- An experienced writer and presenter of technical documents and reports.
- An experienced programmer in Fortran and Matlab languages with experience in C++.
- An experienced user of Microsoft Office software: Word, Excel, and PowerPoint.

### HONORS AND AWARDS

1<sup>st</sup> Place Poster Presentation – Arkansas Academy of Science 2014  
1<sup>st</sup> Place Poster Presentation – U of A Abstract to Contract 2013  
University of Arkansas Doctoral Academy Fellowship 2012 - 2015

### RESEARCH EXPERIENCE

Location: University of Arkansas, Computational Mechanics Laboratory  
Research Director: Dr. R. Panneer Selvam

## Doctoral Research

November 2013 – Present

- Applied a previously-developed, two-dimensional, fluid dynamics model to study phenomenon influencing structural loading by intense vortices.
- Identified new phenomenon: Interaction between an impinging vortex and vortices produced on the loaded body produces large variation in the structure loading.
- Developed a generalized methodology to assess dynamic amplification of structures' responses to a vortex's tangential wind field using the dynamic load factor concept.

## Subsequent Research

January – October 2013

- Applied numerical heat transfer models and cost analysis to assess cost savings by integrating thermal energy storage for residential climate control and industrial water heating.
- Rigorously surveyed parameters controlling pressure drop for flow through packed beds and demonstrated that utility-scale packed-bed thermocline thermal energy storage systems are not viable when gas is used as heat transfer fluid.
- Supervised five undergraduate students' research projects developing a wood gasifier.

## Masters Research

August 2011 – December 2012

- Evaluated performance and viability of a structured-concrete thermocline thermal energy storage system.
- Developed and applied one- and two-dimensional numerical heat transfer models to optimize solid media configuration for thermal performance.
- Conducted complete cost analysis of utility-scale 50 MW central receiver solar thermal power plant with packed-bed and structured-concrete thermal energy storage systems.
- Conducted thermal cycling tests to evaluate the chemical compatibility of propriety concrete mix designs in a 565°C molten salt environment.

## PUBLICATIONS

### Journal Papers

Strasser, M.N., Yousef, M.A., and Selvam, R.P., 2015. Defining the Vortex Loading Period and Application to Assess Dynamic Amplification of Tornado-Like Wind Loading, **In Review with** *J. Fluids and Structures*, Manuscript Number: YJFLS-D-15-00381.

Strasser, M.N. and Selvam, R.P., 2015. The Influence of Vortex Size on the Maximum Loading of a Slender, Cylindrical Structure Impacted by Aerospace to Tornado-Scale Vortices, **In Review with** *J. Fluids and Structures*, Manuscript Number: YJFLS\_2015\_126.

Strasser, M.N. and Selvam, R.P., 2015. Selection of a Realistic Viscous Vortex Tangential Velocity Profile for Computer Simulation of Vortex-Structure Interaction, *J. Arkansas Academy of Science* **69**, Manuscript Number: JAAS MS69-12.

Strasser, M.N. and Selvam, R.P., 2015. The variation in the maximum loading of a circular cylinder impacted by a 2D vortex with time of impact, *J. Fluids and Structures* **58**, 66-78.

- Strasser, M.N. and Selvam, R.P., 2014. A cost and performance comparison of packed bed and structured thermocline thermal energy storage systems, *J. Solar Energy* **108**, 390-402.
- Skinner, J.E., Strasser, M.N., Brown, B.M., and Selvam, R.P., 2014. Testing of high-performance concrete as thermal energy storage medium at high temperatures, *J. Solar Energy Engineering* **136**(2), 021004.

### Conference Proceedings

- Strasser, M.N. and Selvam, R.P., 2015. Evaluation of Pressure Drop in Air- and Salt-Based Thermocline Thermal Energy Storage Systems. Proc. XIX CONGRESO NACIONAL DE INGENIERIA CIVIL, Ancash-Huarez, Peru, November 11-14.
- Selvam, R.P., Strasser, M.N., Ahmed, N., Yousef, M., & Ragan, Q.S., 2015. Observations of the Influence of Hilly Terrain on Tornado path and Intensity from Damage Investigation of the 2014 Tornado in Mayflower, Arkansas. Proc. ASCE Structures Congress, Portland, OR, pp. 2711-2721.
- Strasser, M.N. and Selvam, R.P., 2014. Influence of Phase of Vortex Shedding at Vortex Impact on Peak Loading. Proc. ICAFD, India.
- Strasser, M.N. and Selvam, R.P., 2013. A Comparative Cost and Performance Analysis of Structured and Packed-Bed Thermocline Thermal Energy Storage Systems. Proc. 7<sup>th</sup> Int. Conference on Energy Sustainability, Minneapolis, MN, DOI:10.1115/HT2013-17169.
- Brown, B.M., Strasser, M.N., & Selvam, R.P., 2012. Development of a Structured Thermocline Thermal Energy Storage System. Proc. ASES, Denver, CO, Paper Number 0074.

### Technical Reports

- Selvam, R.P., Ahmed, N., Strasser, M.N., Yousef, M., Ragan, Q.S., and Costa, A., 2015. RAPID: Documentation of Tornado Track of Mayflower Tornado in Hilly Terrain. US National Science Foundation. Available online: <http://compmech.cveg.uark.edu>.
- Selvam, R.P., Hale, W.M., and Strasser, M.N., 2013. Development and Performance Evaluation of High Temperature Concrete for Thermal Energy Storage for Solar Power Generation. US Department of Energy. DOI:10.2172/1072014.

### POSTER PRESENTATIONS

- Strasser, M.N. and Selvam, R.P., 2015. Selection of a Viscous Vortex Tangential Velocity Profile for CFD Application, Presented at: Meeting of the Arkansas Academy of Science, Henderson St. University, Arkadelphia, AR.
- Strasser, M.N. and Selvam, R.P., 2014. The Influence of Vortex Size and Path on Structural Loading, Presented at: Abstract to Contract Research Contest, University of Arkansas, Fayetteville, AR.
- Strasser, M.N. and Selvam, R.P., 2014. Influence of the Phasing of Vortex Shedding and Vortex Impact on Structural Loading, Presented at: Meeting of the Arkansas Academy of Science, Harding University, Searcy, AR.

- Strasser, M.N. and Selvam, R.P., 2013. Cost and Performance Analysis of Thermal Energy Storage Systems for Concentrating Solar Power Plants, Presented at: Abstract to Contract Research Contest, University of Arkansas, Fayetteville, AR.
- Strasser, M.N. and Selvam, R.P., 2012. Performance and Cost Analysis of Packed Bed and Structured Thermocline Thermal Energy Storage Systems, Presented at: High Performance Computing Center Seminar, University of Arkansas, Fayetteville, AR.
- Selvam, R.P., Hale, W.M., John, E., & Strasser, M.N., 2012. Development and Performance Evaluation of High Temperature Concrete for Thermal Energy Storage for Solar Power Generation, Presented at: Sunshot Grand Technology Forum, Denver, CO.

## **PATENT FILINGS AND INTELLECTUAL PROPERTY DISCLOSURES**

- R.P. Selvam, R. Foster, and M. Strasser, 2013. Thermal energy storage for electricity production using “Center-fed” packed bed and air or optional liquids as heat transfer fluid, IP disclosed to University of Arkansas, May 8, 2013.
- R.P. Selvam, M. Strasser, and M. Hale, 2013. Thermal energy storage using packed bed using air for Heating and electricity production, IP disclosed to University of Arkansas, February 11, 2013.

## **MEMBERSHIPS**

Arkansas Academy of Science	2014 – 2015
American Society of Mechanical Engineers	2009 – 2011, 2013 – 2014
American Solar Energy Society	2012 – 2013
Society of Tribologists and Lubrication Engineers	2011 – 2012

## **UNDERGRADUATE STUDENTS MENTORED**

### **University of Arkansas Freshman Engineering Research Program**

Nicholas Chavis & Christopher Maestri Project: Construction of combustion unit for a wood gasifier	August 2013 – April 2014
William Chu & Alexander Raney Project: Construction of filtration unit for a wood gasifier * Received 1 <sup>st</sup> place award on poster presentation	August 2013 – April 2014

### **George Washington Carver Summer Research Program**

Kory Pough Project: Design of a wood gasifier	Summer 2012
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## **REVIEWER**

J. Heat Transfer	Since 2013
J. Renewable and Sustainable Energy	Since 2013
Physics of Fluids	Since 2014
J. Solar Energy Engineering	Since 2014